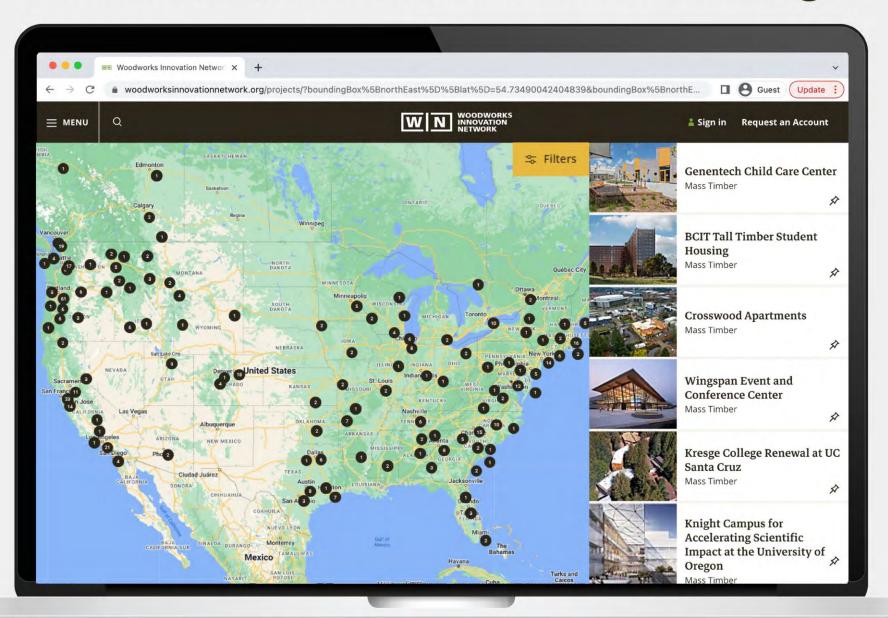


June 21 and June 29, 2023

Presented by Anthony Harvey, PE WoodWorks

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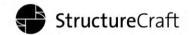














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Questions related to specific materials, methods, and services will be addressed at the conclusion of this presentation.



## **Course Description**

Mass timber is a unique, non-commodity building material and, to lay the groundwork for success, certain critical decisions must be made as early as possible. These decisions can have a big impact on cost and can either increase or limit opportunities later in design. There are many cases of project teams that want to realize the full benefits of mass timber, but, because they base their designs on traditional building practices instead of optimizing them for mass timber, end up with avoidable price premiums. This presentation will walk through early project decisions and design steps, focusing on how to optimize projects for mass timber and how one early decision can influence others. Topics will include construction types, fire ratings, column grids and beam/panel spans, acoustics and MEP integration. Completed mass timber projects will be used to illustrate the variety of viable options when navigating these key decisions.

# Learning Objectives

- 1. Identify construction types within the International Building Code where a mass timber structure is permitted.
- 2. Discuss the impacts of construction type on required fire-resistance ratings of structural elements, noting the impacts that these ratings have on effective member spans and resulting grids.
- 3. Review code-compliance requirements for acoustics and primary frame connections, and provide solutions for meetings these requirements with tested mass timber assemblies.
- 4. Highlight effective methods of integrating MEP services in a mass timber building and discuss the relative impacts of each on cost, aesthetics, occupant comfort and future tenant renovations.

What is the Single Most Important Early Design Decision on a Mass Timber Project? Is it:

Construction Type
Fire-Resistance Ratings
Member Sizes
Grids & Spans
Exposed Timber (where & how much)

MEP Layout
Acoustics
Concealed Spaces
Connections
Penetrations

The Answer is...They All Need to Be Weighed (Plus Others)

Significant Emphasis Placed on the Word Early

#### **Early Because:**

Avoids placing limitations due to construction norms or traditions that may not be efficient with mass timber

Allows greater integration of all building elements in 3D models, ultimately used throughout design, manufacturing and install



#### Early = Efficient

#### Realize Efficiency in:

- Cost reduction
- Material use (optimize fiber use, minimize waste)
- Construction speed
- Trade coordination
- Minimize RFIs

Commit to a mass timber design from the start



There are a number of project-specific factors that influence how these early decisions are made, and in some cases, the order in which the decisions are made:

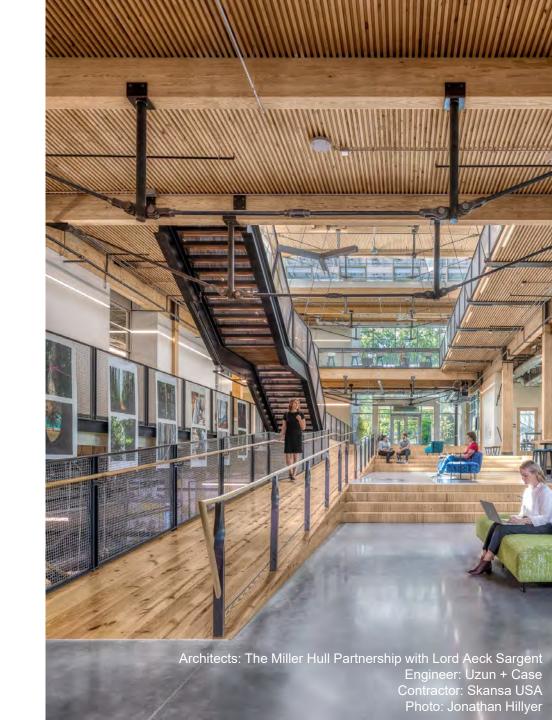
- Site (size, orientation, zoning, cost)
- Building needs (size, occupancy(ies), layout, floor to floor, aesthetics, sustainability goals)
- Resulting code options & design implications



#### One potential design route:

- 1. Building size & occupancy informs construction type & grid
- 2. Construction type informs fire resistance ratings
- 3. Grid & fire resistance ratings inform timber member sizes & MEP layout

But that's not all...



#### Other impactful decisions:

- Acoustics informs member sizes (and vice versa)
- Fire-resistance ratings inform connections & penetrations
- MEP layout informs use of concealed spaces

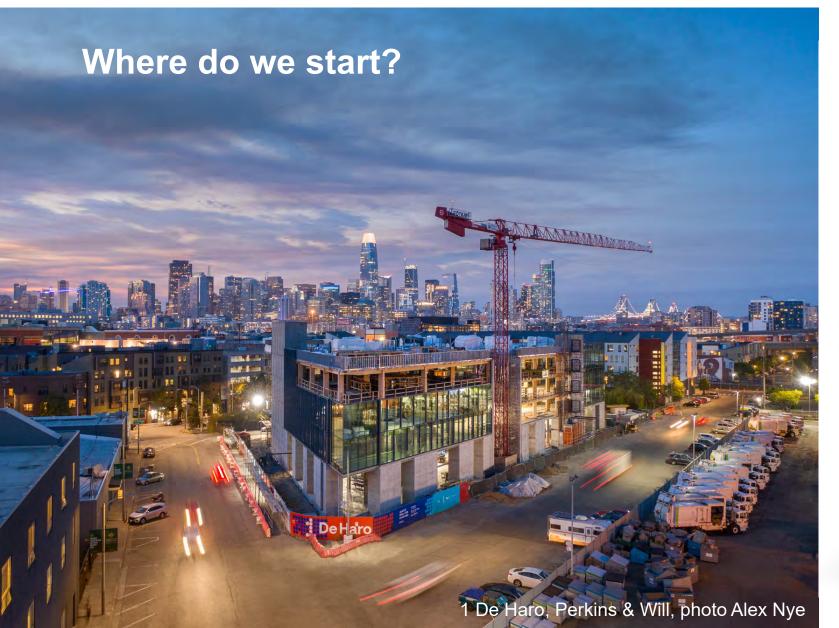


#### Other impactful decisions:

- Grid informs efficient spans, MEP layout
- Manufacturer capabilities inform member sizes, grids & connections
- Lateral system informs connections, construction sequencing

And more...







#### **Construction Type** – Primarily based on building size & occupancy

	Construction Type (All Sprinklered Values)									
	IV-A	IV-B	IV-C	IV-HT	III-A	III-B	V-A	V-B		
Occupancies	Allowable Building Height above Grade Plane, Feet (IBC Table 504.3)									
A, B, R	270	180	85	85	85	85	70	60		
	Allowable Number of Stories above Grade Plane (IBC Table 505.4)									
A-2, A-3, A-4	18	12	6	4	4	3	3	2		
В	18	12	9	6	6	4	4	3		
R-2	18	12	8	5	5	5	4	3		
	Allowable Area Factor (At) for SM, Feet <sup>2</sup> (IBC Table 506.2)									
A-2, A-3, A-4	135,000	90,000	56,250	45,000	42,000	28,500	34,500	18,000		
В	324,000	216,000	135,000	108,000	85,500	57,000	54,000	27,000		
R-2	184,500	123,000	76,875	61,500	72,000	48,000	36,000	21,000		

#### **Construction Type** – Primarily based on building size & occupancy

	Construction Type (All Sprinklered Values)									
	IV-A	IV-B	IV-C	IV-HT	III-A	III-B	V-A	V-B		
Occupancies Allowable Building Height above Grade Plane, Feet (IBC Table 504.3)										
A, B, R	270	180	85	85	85	85	70	60		
For lo	For low- to mid-rise mass timber buildings, there may be									
<b>Amultipl</b>	e opti	ons <sup>2</sup> for	consti	ruction	type.	There a	re pros	and		
cons	of eacl	n, don't	assun	ne that	one ty	pe is al	lways k	est.		
R-2	18	12	8	5	5	5	4	3		
	Allowable Area Factor (At) for SM, Feet <sup>2</sup> (IBC Table 506.2)									
A-2, A-3, A-4	135,000	90,000	56,250	45,000	42,000	28,500	34,500	18,000		
В	324,000	216,000	135,000	108,000	85,500	57,000	54,000	27,000		
R-2	184,500	123,000	76,875	61,500	72,000	48,000	36,000	21,000		

#### **Fire-Resistance Ratings**

- Driven primarily by construction type
- Rating achieved through timber alone or non-com protection required?

TABLE 601
FIRE-RESISTANCE RATING REQUIREMENTS FOR BUILDING ELEMENTS (HOURS)

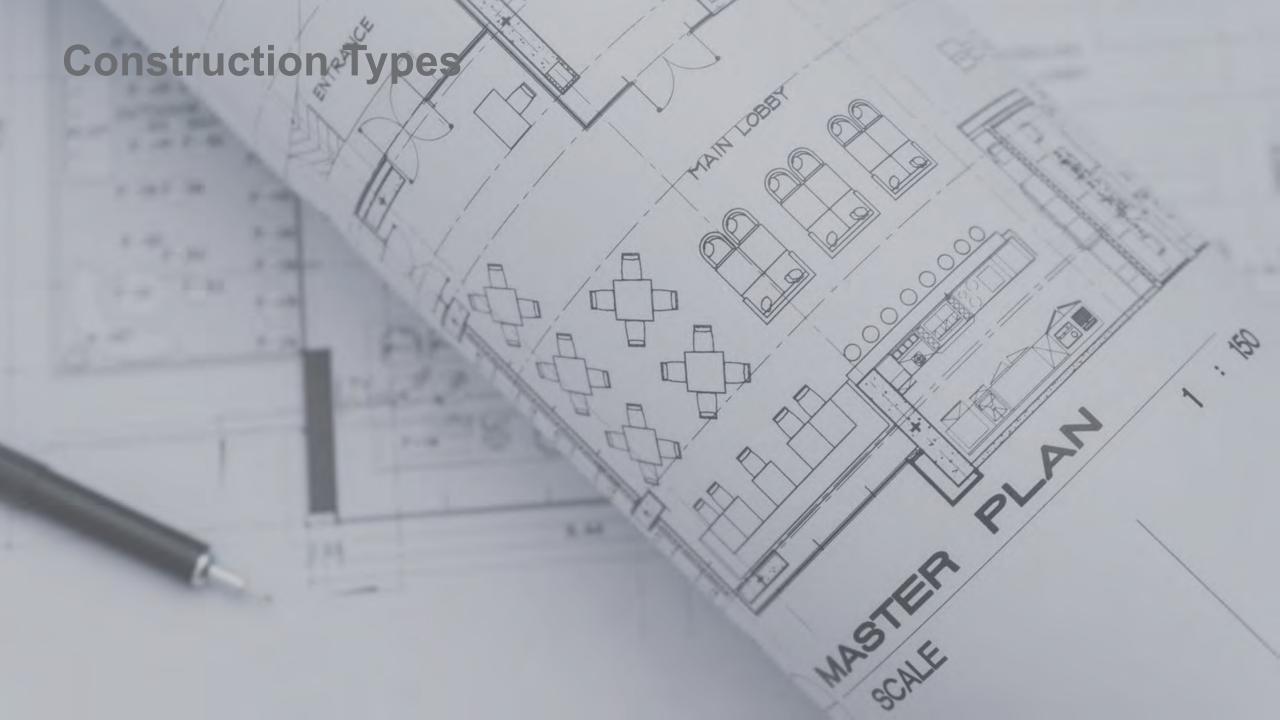
BUILDING ELEMENT		TYPEI		TYPE II		TYPE III		TYPE IV			TYPE V	
		В	Α	В	Α	В	Α	В	С	HT	Α	В
Primary structural frame <sup>f</sup> (see Section 202)	3a, b	2a, b, c	1 <sup>b, c</sup>	0°	1 <sup>b, c</sup>	0	3ª	2ª	2ª	HT	1 <sup>b, c</sup>	0
Bearing walls												
Exterior <sup>a, f</sup>	3	2	1	0	2	2	3	2	2	2	1	0
Interior	3ª	2ª	1	0	1	0	3	2	2	1/HT <sup>g</sup>	1	0
Nonbearing walls and partitions Exterior	See Table 705.5											
Nonbearing walls and partitions Interior <sup>d</sup>	0	0	0	0	0	0	0	0	0	See Section 2304.11.2	0	0
Floor construction and associated secondary structural members (see Section 202)	2	2	1	0	1	0	2	2	2	HT	1	0
Roof construction and associated secondary structural members (see Section 202)	11/2b	1 <sup>b,c</sup>	1 <sup>b,c</sup>	0°	1 <sup>b,c</sup>	0	11/2	1	1	HT	1 <sup>b,c</sup>	0

#### Fire-Resistance Ratings (FRR)

- Thinner panels (i.e. 3-ply) generally difficult to achieve a 1+ hour FRR
- 5-ply CLT / 2x6 NLT & DLT panels can usually achieve a 1- or 2hour FRR
- Construction Type | FRR | Member Size | Grid (or re-arrange that process but follow how one impacts the others)

Panel	Example Floor Span Ranges
3-ply CLT (4-1/8" thick)	Up to 12 ft
5-ply CLT (6-7/8" thick)	14 to 17 ft
7-ply CLT (9-5/8")	17 to 21 ft
2x4 NLT	Up to 12 ft
2x6 NLT	10 to 17 ft
2x8 NLT	14 to 21 ft
5" MPP	10 to 15 ft

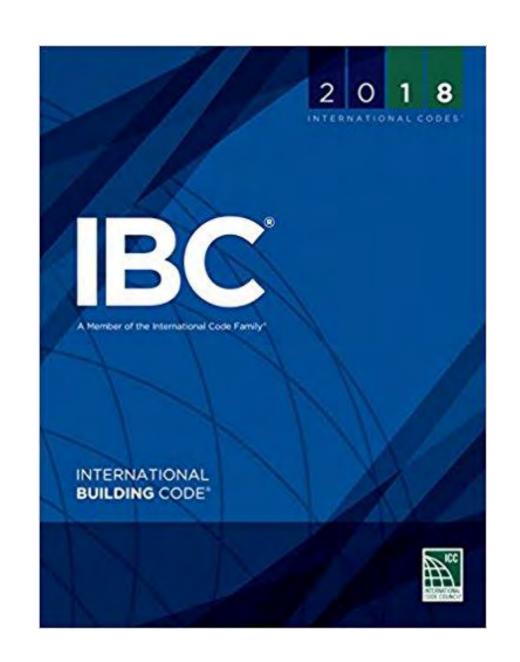




# When does the code allow mass timber to be used?

IBC defines mass timber systems in IBC Chapter 2 and notes their acceptance and manufacturing standards in IBC Chapter 23

Permitted anywhere that combustible materials and heavy timber are allowed, plus more



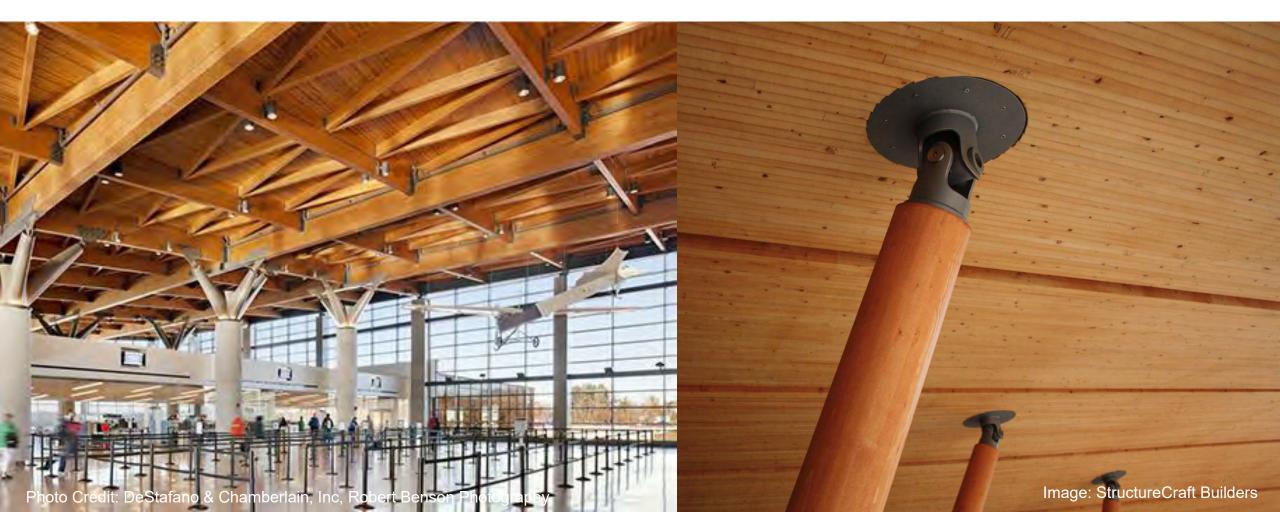
IBC defines 5 construction types: I, II, III, IV, V A building must be classified as one of these

Construction Types I & II: All elements required to be non-combustible materials

However, there are exceptions including several for mass timber

#### Where does the code allow MT to be used?

Type IB & II: Roof Decking



All wood framed building options:

#### Type III

Exterior walls non-combustible (may be FRTW)
Interior elements any allowed by code, including mass timber

#### Type V

All building elements are any allowed by code, including mass timber

Types III and V are subdivided to A (protected) and B (unprotected)

#### **Type IV (Heavy Timber)**

Exterior walls non-combustible (may be FRTW OR CLT)
Interior elements qualify as Heavy Timber (min. sizes, no concealed spaces except in 2021 IBC)

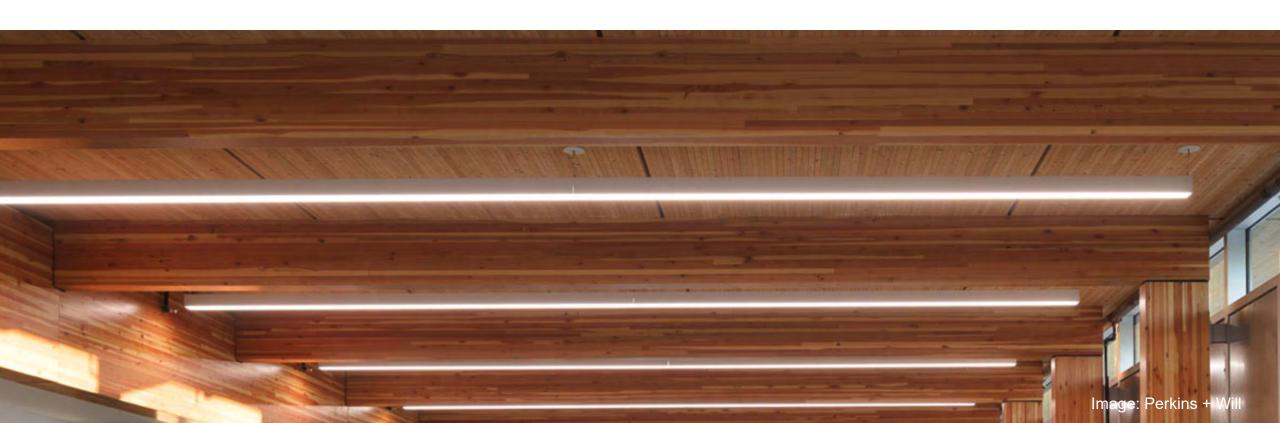
# Where does the code allow MT to be used?

 <u>Type III</u>: Interior elements (floors, roofs, partitions/shafts) and exterior walls if FRT



#### Where does the code allow MT to be used?

 <u>Type IV</u>: Any exposed interior elements & roofs, must meet min. sizes; exterior walls if CLT or FRT.
 Concealed space limitations (varies by code version)

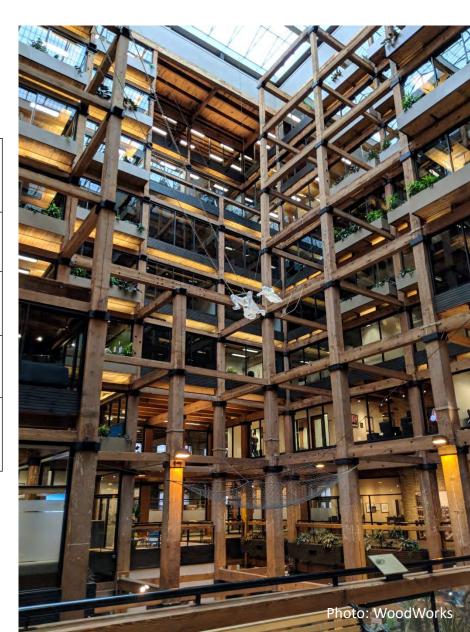


# Type IV construction permits exposed heavy/mass timber elements of min. sizes.

Framing		Solid Sawn (nominal)	Glulam (actual)	SCL (actual)		
Floor	Columns	8 x 8	$6^3/_4 \times 8\frac{1}{4}$	7 x 7½		
Flo	Beams	6 x 10	5 x 10½	5¼ x 9½		
of	Columns	6 x 8	5 x 8¼	5¼ x 7½		
Roof	Beams*	4 x 6	$3 \times 6^7/_8$	3½ X 5½		

Minimum Width by Depth in Inches See IBC 2018 2304.11 or IBC 2015 602.4 for Details

\*3" nominal width allowed where sprinklered



#### Type IV min. sizes:

#### Floor Panels/Decking:

- 4" thick CLT (actual thickness)
- 4" NLT/DLT/GLT (nominal thickness)
- 3" thick (nominal) decking covered with: 1" decking or 15/32" WSP or ½" particleboard







#### Type IV min. sizes:

#### **Interior Walls:**

- Laminated construction 4" thick
- Solid wood construction min. 2 layers of 1" matched boards
- Wood stud wall (1 hr min)
- Non-combustible (1 hr min)

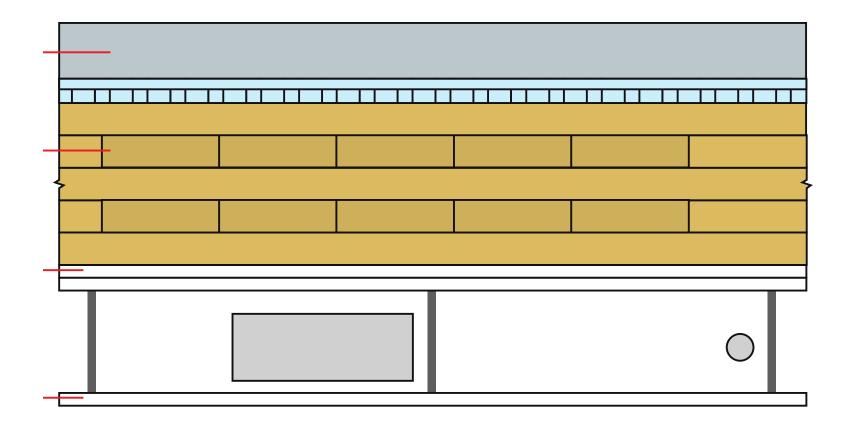
Verify other code requirements for FRR (eg. interior bearing wall; occupancy separation)





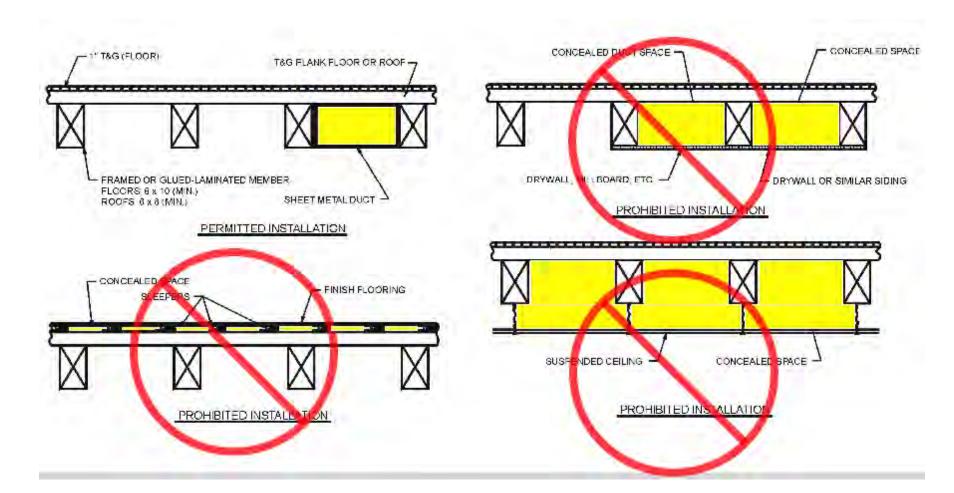
#### Type IV concealed spaces

Can I have a dropped ceiling? Raised access floor?



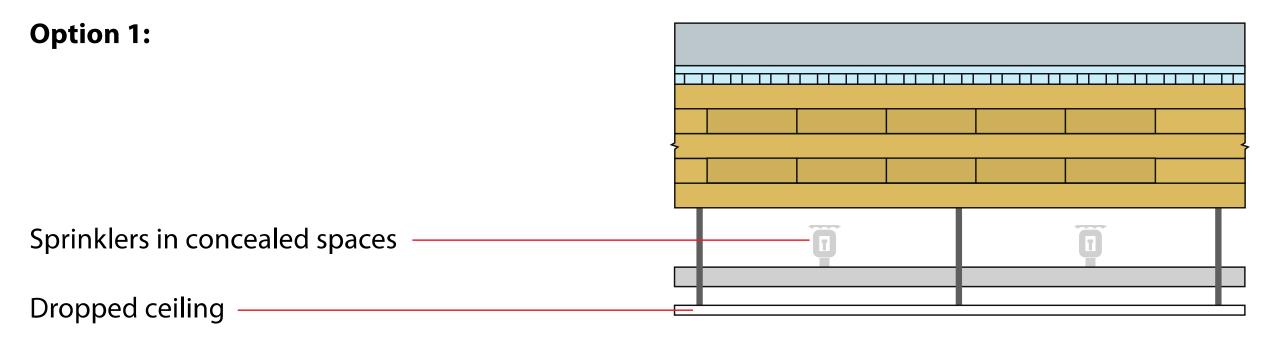
#### Type IV concealed spaces

Until 2021 IBC, Type IV-HT provisions prohibited concealed spaces



Credit: IBC

#### Type IV concealed space options within 2021 IBC

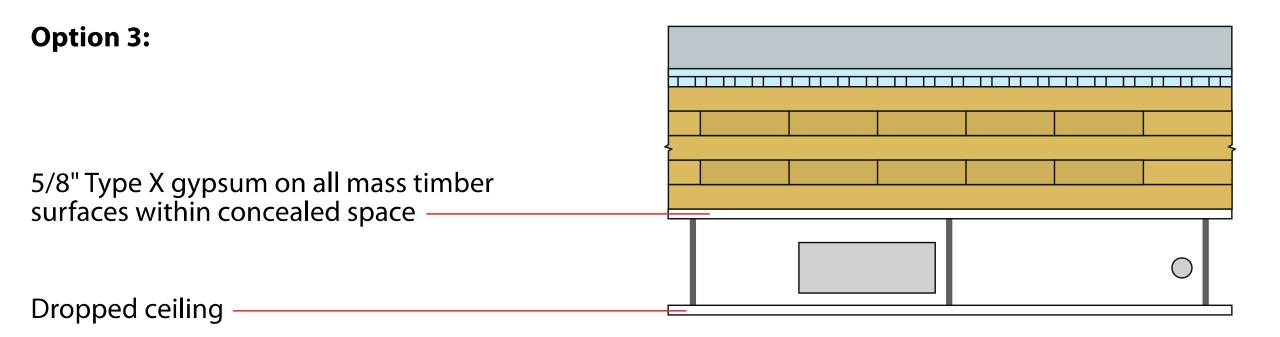


#### Type IV concealed space options within 2021 IBC

Noncombustible insulation

Dropped ceiling

#### Type IV concealed space options within 2021 IBC



#### Concealed spaces solutions paper



# Concealed Spaces in Mass Timber and Heavy Timber Structures

Concealed spaces, such as those created by a dropped ceiling in a floor/ceiling assembly or by a stud wall assembly, have unique requirements in the International Building Code (IBC) to address the potential of fire spread in non-visible areas of a building. Section 718 of the 2018 IBC includes prescriptive requirements for protection and/or compartmentalization of concealed spaces through the use of draft stopping, fire blocking, sprinklers and other means. For information on these requirements, see the WoodWorks Q&A, Are sprinklers required in concealed spaces such as floor and roof cavities in multi-family wood-frame buildings?

For mass timber building elements, the choice of construction type can have a significant impact on concealed space requirements. Because mass timber products such as cross-laminated timber (CLT) are prescriptively recognized for Type IV construction, there is a common misperception that exposed mass timber building elements cannot be used or exposed in other construction types. This is not the case.

In addition to Type IV buildings, structural mass timber elements—including CLT, glue-laminated timber (glulam), nail-laminated timber (NLT), structural composite lumber (SCL), and tongue-and-groove (T&G) decking—can be utilized and exposed in the following construction types, whether or not a fire-resistance rating is required:

- Type III Floors, roofs and interior walls may be any material permitted by code, including mass timber; exterior walls are required to be noncombustible or fire retardant-treated wood.
- Type V Floors, roofs, interior walls and exterior walls (i.e., the entire structure) may be constructed of mass timber.
- Types I and II Mass timber may be used in select circumstances such as roof construction—including the primary frame in the 2021 IBC—in Types I-B, II-A or II-B; exterior columns and arches when 20 feet or more of horizontal separation is provided; and balconies, canopies and similar projections.



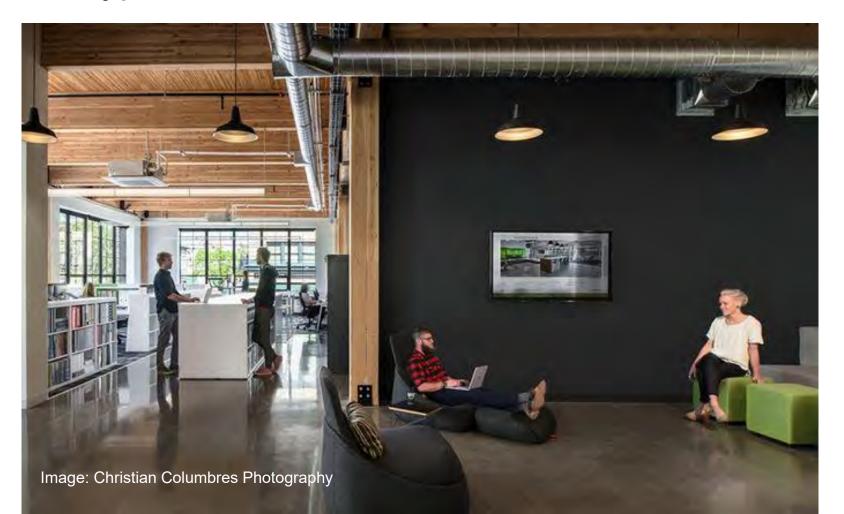


https://www.woodworks.org/wp-content/uploads/wood\_solution\_paper-Concealed Spaces Timber Structures.pdf

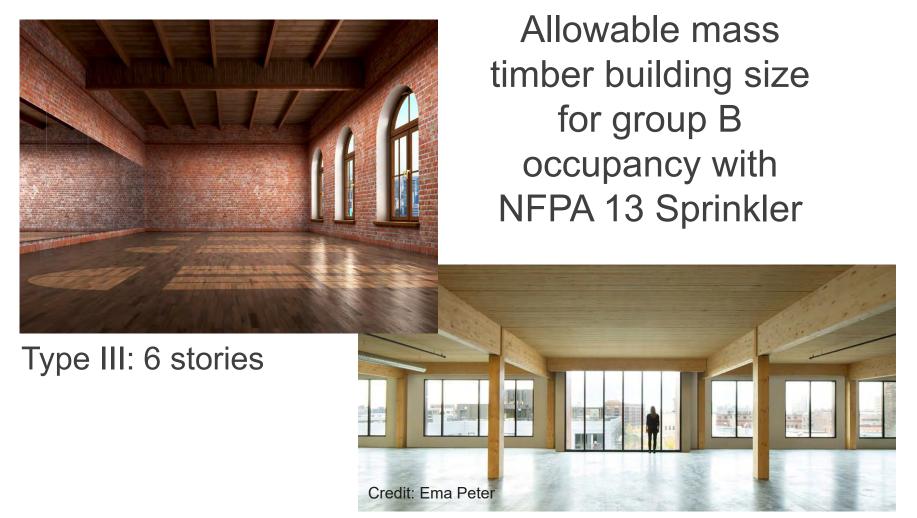
### **Construction Types**

#### Where does the code allow MT to be used?

• Type V: All interior elements, roofs & exterior walls



### **Construction Types**

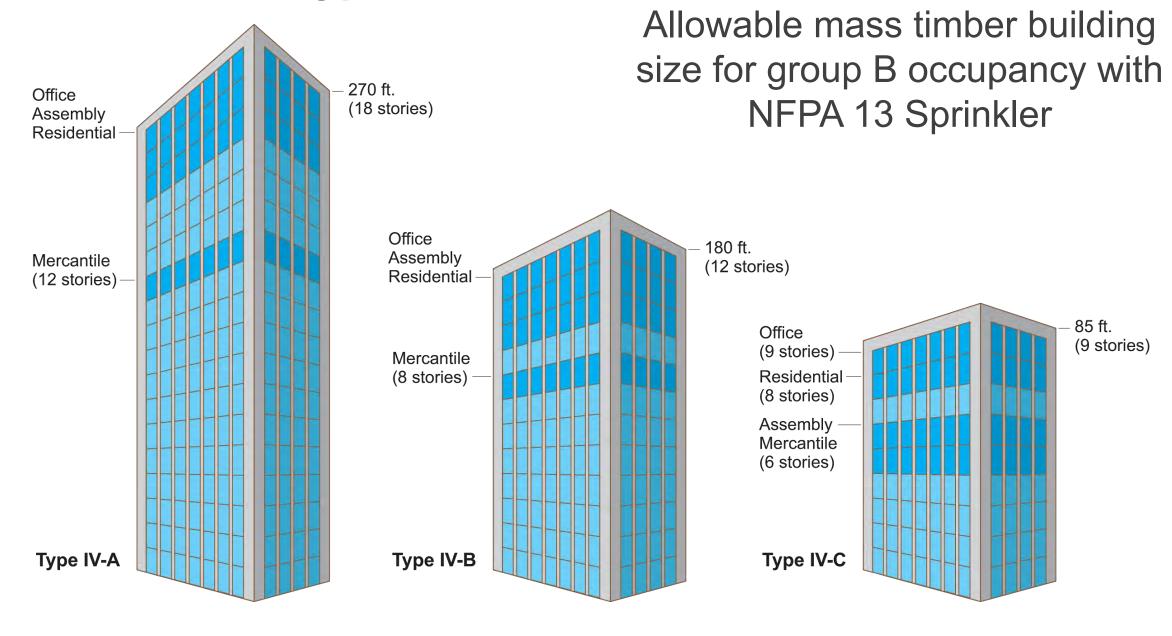


Credit: Christian Columb

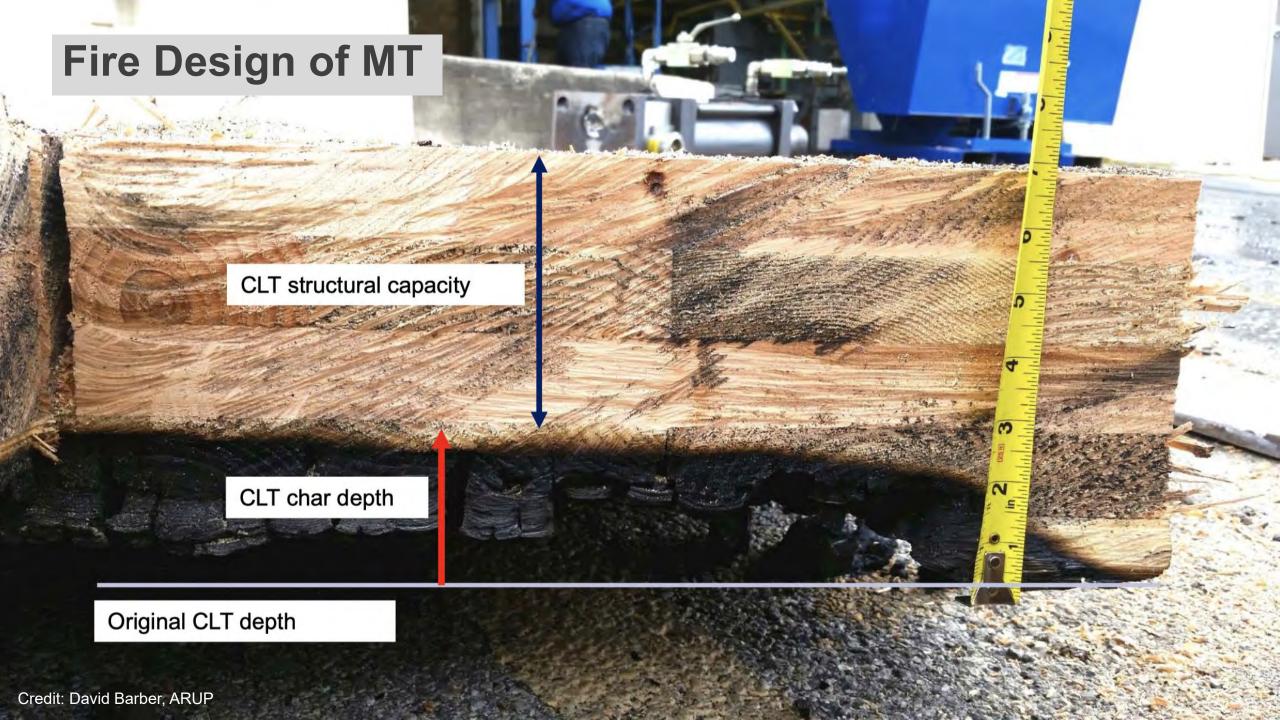
Type V: 4 stories

Type IV: 6 stories

### **Construction Types**



New Options in 2021 IBC



### Construction type influences FRR

TABLE 601
FIRE-RESISTANCE RATING REQUIREMENTS FOR BUILDING ELEMENTS (HOURS)

BUILDING ELEMENT	TYI	PEI	TYF	E II	TYPE III		TYPE IV	TYF	PE V
BOILDING ELEMENT	Α	В	Α	В	Α	В	HT	Α	В
Primary structural frame <sup>f</sup> (see Section 202)	3ª	2ª	1	0	1	0	HT	1	0
Bearing walls Exterior <sup>e, f</sup> Interior	3 3ª	2 2ª	1 1	0	2 1	2 0	2 1/HT	1	0
Nonbearing walls and partitions Exterior				See	Table 6	Table 602			
Nonbearing walls and partitions Interior <sup>d</sup>	0	0	0	0	0	0	See Section 602.4.6	0	0
Floor construction and associated secondary members (see Section 202)	2	2	1	0	1	0	НТ	1	0
Roof construction and associated secondary members (see Section 202)	1 <sup>1</sup> / <sub>2</sub> <sup>b</sup>	1 <sup>b,c</sup>	1 <sup>b,c</sup>	0°	1 <sup>b,c</sup>	0	НТ	1 <sup>b,c</sup>	0

Source: 2018 IBC

### Construction type influences FRR

FIRE-RESISTANCE RATING REQUIREMENTS FOR BUILDING ELEMENTS (HOURS)

BUILDING ELEMENT	TY	PEI	TYF	EII	TYP	EIII		TYPE IV			TYPE	
BUILDING ELEMENT	Α	В	Α	В	Α	В	А	В	С	HT	A	В
Primary structural frame <sup>f</sup> (see Section 202)	3ª, 6	2ª, b, c	1 <sup>b, c</sup>	0°	1 <sup>h, c</sup>	0	3*	2*	2ª	HT	1 <sup>b, c</sup>	0
Bearing walls												
Exterior e.f	3	2	1	0	2	2	3	2	2	2	1	0
Interior	3ª	2°	1	0	1	0	3	2	2	1/HT <sup>g</sup>	1	0
Nonbearing walls and partitions Exterior						See 7	Table 70	5.5				
Nonbearing walls and partitions Interior <sup>d</sup>	0	0	0	0	0	0	0	0	0	See Section 2304.11.2	0	0
Floor construction and associated secondary structural members (see Section 202)	2	2	1	0	1	0	2	2	2	HT	1	0
Roof construction and associated secondary structural members (see Section 202)	1 <sup>1</sup> / <sub>2</sub> <sup>b</sup>	1 <sup>b,c</sup>	1 <sup>b,c</sup>	0°	$1^{b,c}$	0	11/2	1.	1.	HT	1 <sup>b,c</sup>	0

Source: 2021 IBC

Construction type influences FRR

- Type IV-HT Construction (minimum sizes)
- Other than type IV-HT: Demonstrated fire resistance

Method of demonstrating FRR (calculations or testing) can impact member sizing





#### **Member Sizes**

- Impact of FRR on sizing
- Impact of sizing on efficient spans
- Consider connections can drive member sizing





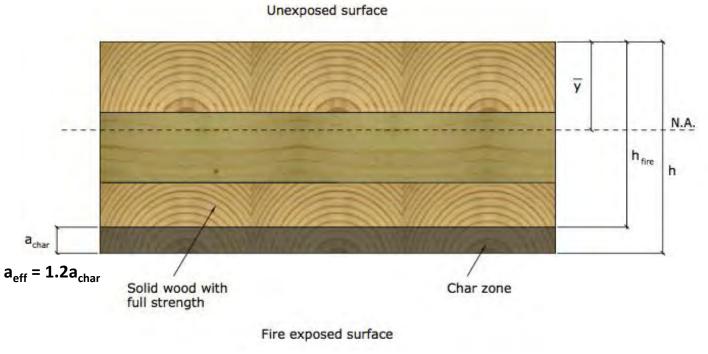




### Which Method of Demonstrating FRR of MT is Being Used?

- 1. Calculations in Accordance with IBC 722 → NDS Chapter 16
- 2. Tests in Accordance with ASTM E119





# Calculated FRR of Exposed MT: IBC to NDS code compliance path



Code Path for Exposed Wood Fire-Resistance Calculations

#### IBC 703.3

#### Methods for determining fire resistance

- Prescriptive designs per IBC 721.1
- Calculations in accordance with IBC 722
- · Fire-resistance designs documented in sources
- · Engineering analysis based on a comparison
- · Alternate protection methods as allowed by 104.11



#### **IBC 722**

#### Calculated Fire Resistance

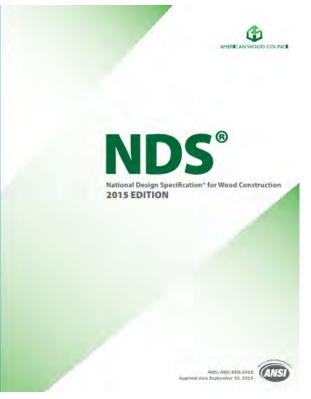
"The calculated fire resistance of exposed wood members and wood decking shall be permitted in accordance with Chapter 16 of ANSI/AWC National Design Specification for Wood Construction (NDS)



#### **NDS Chapter 16**

#### Fire Design of Wood Members

- · Limited to calculating fire resistance up to 2 hours
- Char depth varies based on exposure time (i.e., fire-resistance rating), product type and lamination thickness. Equations and tables are provided.
- TR 10 and NDS commentary are helpful in implementing permitted calculations.







NDS Chapter 16 includes calculation of fire resistance of NLT, CLT, Glulam, Solid Sawn and SCL wood products

# Table 16.2.1B Effective Char Depths (for CLT with $\beta_n$ =1.5in./hr.)

Required Fire Endurance					(in.)	Depths esses, h			
(hr.)	5/8	3/4	7/8	1	1-1/4	1-3/8	1-1/2	1-3/4	2
1-Hour	2.2	2.2	2.1	2.0	2.0	1.9	1.8	1.8	1.8
1½-Hour	3.4	3.2	3.1	3.0	2.9	2.8	2.8	2.8	2.6
2-Hour	4,4	4.3	4.1	4.0	3.9	3.8	3.6	3.6	3.6

Nominal char rate of 1.5"/HR is recognized in NDS. Effective char depth calculated to account for duration, structural reduction in heat-affected zone



Table 16.2.1A Char Depth and Effective Char Depth (for  $\beta_n = 1.5$  in./hr.)

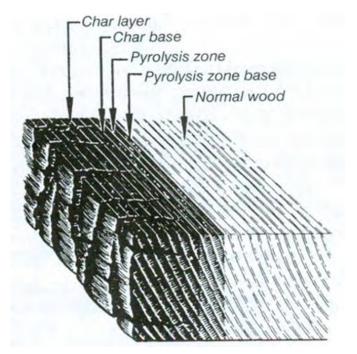
Required Fire Resistance (hr.)	Char Depth, a <sub>char</sub> (in.)	Effective Char Depth, a <sub>eff</sub> (in.)
1-Hour	1.5	1.8
1½-Hour	2.1	2.5
2-Hour	2.6	3.2

Table 16.2.1B Effective Char Depths (for CLT with  $\beta_n$ =1.5in./hr.)

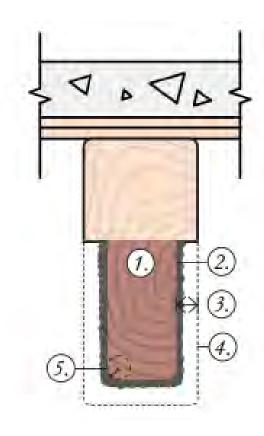
Required Fire Endurance (hr.)	Effective Char Depths, a <sub>char</sub> (in.) lamination thicknesses, h <sub>lam</sub> (in.)										
	5/8	3/4	7/8	1	1-1/4	1-3/8	1-1/2	1-3/4	2		
1-Hour	2.2	2.2	2.1	2.0	2.0	1.9	1.8	1.8	1.8		
1½-Hour	3.4	3.2	3.1	3.0	2.9	2.8	2.8	2.8	2.6		
2-Hour	4,4	4.3	4.1	4.0	3.9	3.8	3.6	3.6	3.6		

Two structural capacity checks performed:

- 1. On entire cross section neglecting fire effects
- 2. On post-fire remaining section, with stress increases







$$a_{char} = \beta_t t^{0.813}$$

Solid Sawn, Glulam, SCL

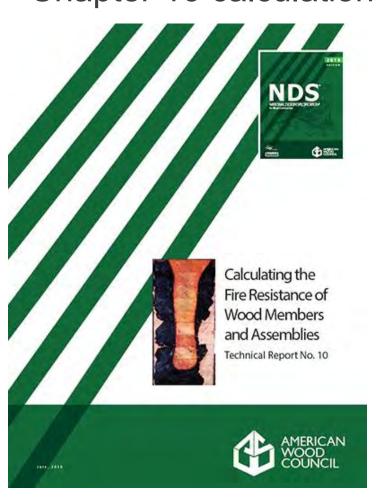
$$a_{char} = n_{lam} h_{lam} + \beta_t \left( t - \left( n_{lam} t_{gi} \right) \right)^{0.813}$$

$$a_{eff} = 1.2a_{char}$$

**Effective Char Depth** 

CLT

# AWC's TR10 is a technical design guide, aids in the use of NDS Chapter 16 calculations



#### **Example 5: Exposed CLT Floor** - Allowable Stress Design

Simply-supported cross-laminated timber (CLT) floor spanning L=18 ft in the strong-axis direction. The design loads are q<sub>live</sub>=80 psf and q<sub>dead</sub>=30 psf including estimated self-weight of the CLT panel. Floor decking, nailed to the unexposed face of CLT panel, is spaced to restrict hot gases from venting through half-lap joints at edges of CLT panel sections. Calculate the required section dimensions for a 1-hour structural fire resistance time when subjected to an ASTM E119 fire exposure.

For the structural design of the CLT panel, calculate the maximum induced moment.

Calculate panel load (per foot of width):

 $W_{load} = (q_{dead} + q_{live}) = (30 \text{ psf} + 80 \text{ psf})(1\text{ft width}) = 110 \text{ plf/ft of width}$ 

Calculate maximum induced moment (per foot of width):

 $M_{\text{max}} = w_{\text{load}} L^2 / 8 = (110)(18^2)/8 = 4,455 \text{ ft-lb/ft} \text{ of width}$ 

From PRG 320, select a 5-ply CLT floor panel made from 1-3/8 in x 3-1/2 in. lumber boards (CLT thickness of 6-7/8 inches). For CLT grade V2, tabulated properties are:

Bending moment,  $F_bS_{eff,0} = 4,675$  ft-lb/ft of width

(PRG 320 Annex A, Table A2)

Calculate the allowable design moment (assuming  $C_D=1.0$ :  $C_M=1.0$ :  $C_t=1.0$ :  $C_L=1.0$ )

 $M_{s}' = F_b(S_{eff})(C_D)(C_M)(C_t)(C_L) = 4,675 (1.0)(1.0)(1.0) = 4,675 \text{ ft-lb/ft of width}$ 

(NDS 10.3.1)

Structural Check:

 $M_s' \ge M_{max}$ 

4,675 ft-lb/ft > 4,455 ft-lb/ft

V

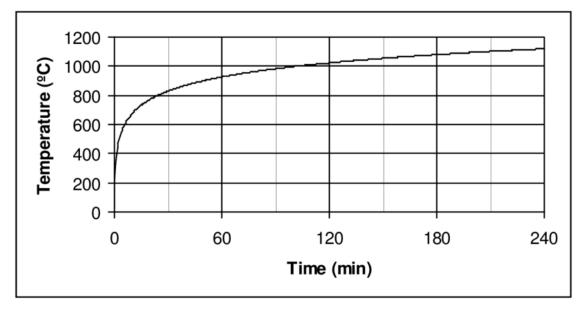
(note: serviceability check is not performed to simplify the design example, but should be done in typical structural design).

Source: AWC's TR10

#### **Tested FRR of Exposed MT:**

 IBC 703.2 notes the acceptance of FRR demonstration via testing in accordance with ASTM E119

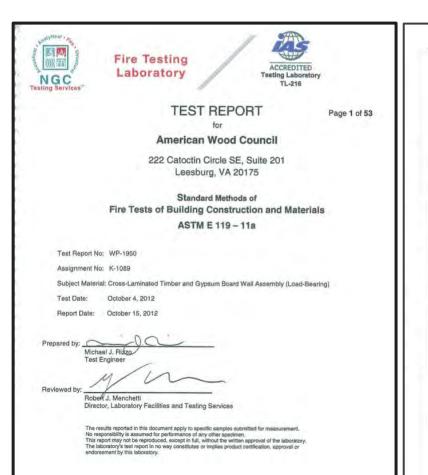
703.2 Fire-resistance ratings. The fire-resistance rating of building elements, components or assemblies shall be determined in accordance with the test procedures set forth in ASTM E119 or UL 263 or in accordance with Section 703.3. The fire-resistance rating of penetrations and fire-resistant joint systems shall be determined in accordance Sections 714 and 715, respectively.

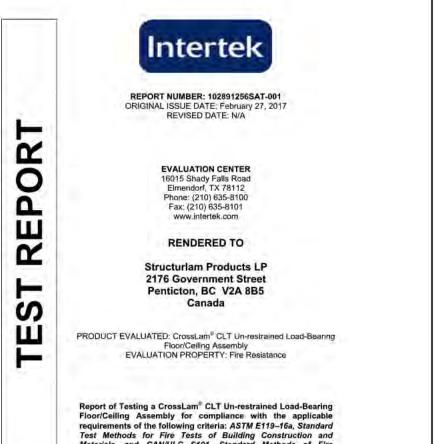


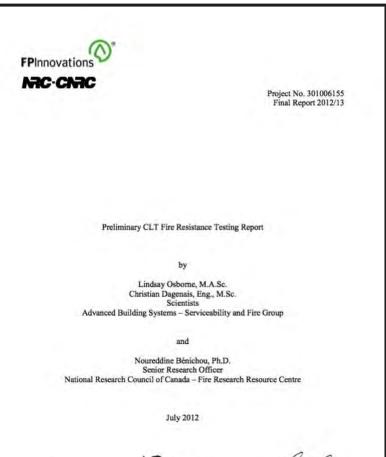
Standard ASTM E119 test timetemperature curve

#### **Tested FRR of Exposed MT:**

 Many successful Mass Timber ASTM E119 fire tests have been completed by industry & manufacturers







### **WoodWorks Inventory of Fire Tested MT Assemblies**





CLT Panel	Manufacturer	CLT Grade or Major x Minor Grade	Ceiling Protection	Panel Connection in Test	Floor Topping	Load Rating	Fire Resistance Achieved (Hours)	Source	Testing Lab
3-pty CLT (114-mm + 488 in)	Nordic	SPF 1650 Fb 1.3 EMSR 3, SPF #3	2 layers 1/2" Type X gypsum	Half-Lap	None	Reduced 56% Moment Capacity	- u - 1	J.(Test I)	NRC Fire Laboratory
3-ply CLT (105 mm + .133 in)	Structurlam	SPF #1/#2 # SPF #1/#2	1 layer 5/8" Type Xgypsum	fluff-Lap	None	Reduced 7.5% Mannent Capacity	-1	1 (Test 1)	NRC Fire Laboratory
5-ply CLT (173mm#.875*)	Nordir	Ei	None	Topside Spline	2 staggered layers of 1/2" coment boards	Londed. See Manufacturer	2	2	NRC Fire Laboratory March 2016
5-ply CLT (175mm6.875*)	Nordic	El	1 layer of 5 %" Type X gyp sum under 2- change is and furing strips with 3 5 %"	Tops ide Spline	2 stagg ered layers of 1/2" cement bounds	Loaded. See Manufacturer	3	5	NRC Fire Laboratory Nov 2014
5-ply CLT (175mm6.875*)	Nordie	El	None	Topside Spline	3/4 in proprietary gyperere over Maxxon amoustical mat	Reduced 50% Mannest Capacity	(.5	8	UL
5-ply CLT (175mm6.875*)	Nordic	Ei	l layer 5/8" normal gypsum	Topside Spline	3/4 (a. proprietary gyperete o ver Maxion acoustical mat or proprietary sound board	Rod acod 50% Mament Capacity	2	-40	ÜL
5-ply-CLT (175mm6-875*)	Nordie	El	Likyer 5%* Type X Gyp ander Resilien Channel ander 7.7%* Literat with \$ 1/2* Mineral Wood feween hists	Half-Lap	None	Loaded. See Mamifacturer	2	21	Interték 8/24/2012
S-ply CLT (175mm6-875*)	Simetorlan	EI M5 MSR 2100 A SPF #2	None	Tops ide Spline	1-1/2" Maxxon Cyp-Gree 2000 over Maxxon Bourforcing Mesh	Loaded. See Manofacturer	2.5	6	Intertek, 2/22/2016
5-ply CLT (175mm6.875*)	DR Johnson	VI	None	Half-Lap & Tops ide Spline	2° gypsumtopping	Loaded. See Manufacturer	2	4	SwRI (May 2016)
5-ply CLT (175mm6.875*)	Nordic	SPF 1950 Fb MSR a SPF #3	None	Half-Lap	None	Reduced 5 9% Moment Capacity	(3	(Test-1)	NRC Fire Laboratory
5-ply CLT (175mm6.875°)	Structurfam	SPF #1/#2 # SPF #1/#2	I layer 5/K" Type Xgypsum	Half-Lap	Nesz	Unreduced 101% Moment Capacity	2	1 (Test 6)	NRC Fire Laboratory
7-ply CLT (245mm 9.65°)	Structurlam	SPF #1/#2 # SPF #1/#2	None	fliiffLap	None	Unroduced 101% Moment Capacity	2.5	1 (Fest 7)	NRC Fire Laboratory
5-ply CLT (175mm#.875*)	SmartLam	SL-V4	None	Half-Lap	nominal 1/2*ply wood with 8d nails.	Londed. See Manufacturer	7	12 (Test 4)	Western Fire Center 10/26/2016
5-ply CLT (175mm6.875°)	SmartLani	Vi	None	Half-Lap	nominal 1/2* ply wood with #d nails.	Loaded. See Manufacturer	2.	12 (Test 5)	Western Fire Center 10/28/2016
5-ply CLT (175mm6.875°)	DR Johnson	VI.	None	Half-Lag	nominal 1/2" plywoodwith #danils.	Lorded See Mamifacturer	2	12 (Test 6)	Western Fire Center 11/01/2016
5-ply C17	KIH	CV3M1	Noue	Half-Lap &	None	Lizaded.	1	18	SwRI

Method of demonstrating FRR (calculations or testing) can impact member sizing

Each has unique benefits:

- Testing:
  - Can result in higher FRR for some assemblies when compared to calculations (i.e. 2-hr FRR with 5-ply CLT panel).
  - Seen as more acceptable by some building officials
- Calculations:
  - Can provide more design flexibility
  - Allows for project span and loading specific analysis



## Fire-Resistive Design of Mass Timber Members

Code Applications, Construction Types and Fire Ratings

Richard McLain, PE, SE - Senior Technical Director - WoodWorks Scott Breneman, PhD, PE, SE - Senior Technical Director - WoodWorks

For many years, exposed heavy timber framing elements have been permitted in U.S. buildings due to their inherent fire-resistance properties. The predictability of wood's char rate has been well-established for decades and has long been recognized in building codes and standards.

Today, one of the exciting trends in building design is the growing use of mass timber—i.e., large solid wood panel products such as cross-laminated timber (CLT) and naillaminated timber (NLT)—for floor, wall and roof construction. Like heavy timber, mass timber products have inherent fire resistance that allows them to be left exposed and still achieve a fire-resistance rating. Because of their strength and dimensional stability, these products also offer a low-carbon alternative to steel, concrete, and masonry for many applications. It is this combination of exposed structure and strength that developers and designers across the country

are leveraging to create innovative designs with a warm yet modern aesthetic, often for projects that go beyond traditional norms of wood design.

This paper has been written to support architects and engineers exploring the use of mass timber for commercial and multi-family construction. It focuses on how to meet fire-resistance requirements in the International Building Code (IBC), including calculation and testing-based methods. Unless otherwise noted, references refer to the 2018 IBC.

#### Mass Timber & Construction Type

Before demonstrating fire-resistance ratings of exposed mass timber elements, it's important to understand under what circumstances the code currently allows the use of mass timber in commercial and multi-family construction.

A building's assigned construction type is the main indicator of where and when all wood systems can be used. IBC Saction 602 defines five main options (Type I through V) with all but Type IV having subcategories A and B. Types III and V permit the use of wood framing throughout much of the structure and both are used extensively for modern mass timber buildings.

Type III (IBC 602.3) – Timber elements can be used in floors, roofs and interior walls. Fire-retardant-treated wood (FRTW) framing is permitted in exterior walls with a fireresistance rating of 2 hours or less.

Type V (IBC 602.5) – Timber elements can be used throughout the structure, including floors, roofs and both interior and exterior

Type IV (IBC 602.4) – Commonly referred to as 'Heavy Timber' construction, this option



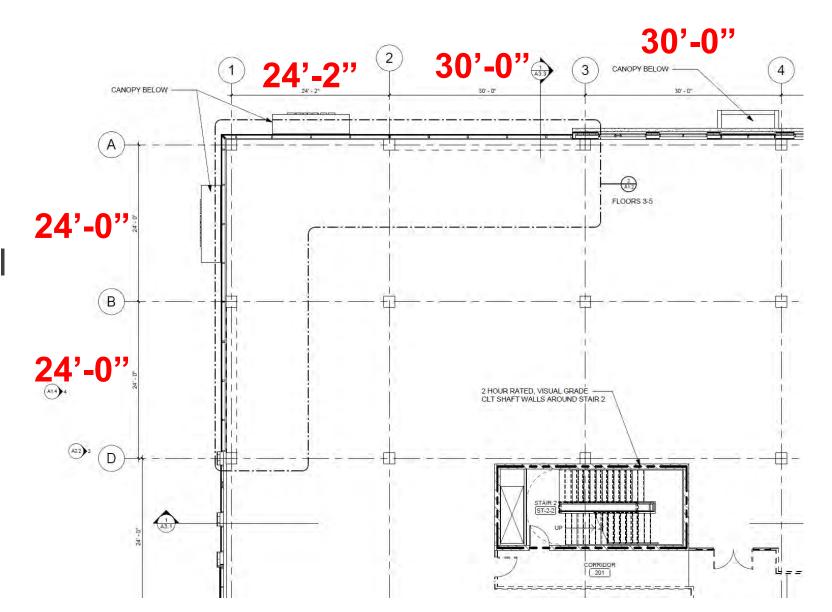
### Mass Timber Fire Design Resource

- Code compliance options for demonstrating FRR
- Free download at woodworks.org



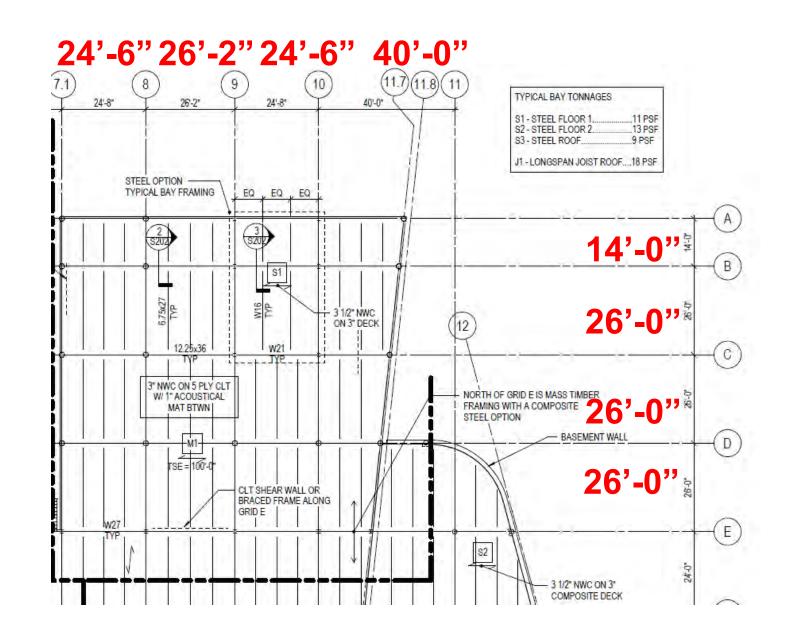
### **Grids & Spans**

- Consider Efficient Layouts
- Repetition & Scale
- Manufacturer Panel Sizing
- Transportation



### **Grids & Spans**

- Consider Efficient Layouts
- Repetition & Scale
- Manufacturer Panel Sizing
- Transportation



#### **Member Sizes**

- Impact of FRR on Sizing
- Impact of Sizing on Efficient Spans
- Consider connections can drive member sizing

### 0 HR FRR: Consider 3-ply Panel

- Efficient Spans of 10-12 ft
- Grids of 20x20 (1 purlin) to 30x30
   (2 purlins) may be efficient

Albina Yard, Portland, OR 20x20 Grid, 1 purlin per bay 3-ply CLT Image: Lever Architecture



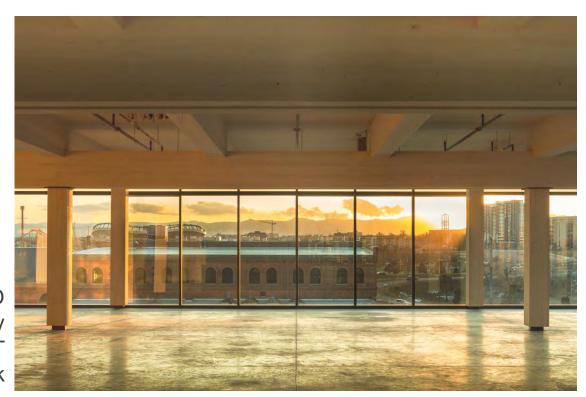
#### **Member Sizes**

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### 0 HR FRR: Consider 3-ply Panel

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- Grids of 20x20 (1 purlin) to 30x30
   (2 purlins) may be efficient

Platte Fifteen, Denver, CO 30x30 Grid, 2 purlins per bay 3-ply CLT Image: JC Buck



#### **Member Sizes**

- Impact of FRR on Sizing
- Impact of Sizing on Efficient Spans
- Consider connections can drive member sizing

### 1 or 2 HR FRR: Likely 5-ply Panel

- Efficient spans of 14-17 ft
- Grids of 15x30 (no purlins) to 30x30 (1 purlin) may be efficient

First Tech Credit Union, Hillsboro, OR 12x32 Grid, One-Way Beams 5-ply (5.5") CLT Image: Swinerton

#### **Member Sizes**

- Impact of FRR on Sizing
- Impact of Sizing on Efficient Spans
- Consider connections can drive member sizing

### 1 or 2 HR FRR: Likely 5-ply Panel

- Efficient spans of 14-17 ft
- Grids of 15x30 (no purlins) to 30x30 (1 purlin) may be efficient

Clay Creative, Portland, OR 30x30 Grid, 1 purlin per bay 2x6 NLT

Image: Mackenzie

### **Construction Type Early Decision Example**



### 7-story building on health campus

- Group B occupancy, NFPA 13 sprinklers throughout
- Floor plate = 22,300 SF
- Total Building Area = 156,100 SF

#### **MT Construction Type Options:**

- If Building is < 85 ft</li>
  - 7 stories of IV-C
  - 6 stories of IIIA or IV-HT over 1 story IA podium
- If Building is > 85 ft
  - 7 stories of IV-B

### **Construction Type Early Decision Example**

#### MT Construction Type Options:

- If Building is < 85 ft</li>
  - 7 stories of IV-C
  - 6 stories of IIIA or IV-HT over 1 story IA
- If Building is > 85 ft
  - 7 stories of IV-B

### Implications of construction type choice in this example:

- FRR (2 hr vs 1 hr vs min sizes)
- Efficient spans & grid
- Exposed timber limitations
- Concealed spaces
- Cost
- And more...



### **Construction Type Early Decision Example**

#### MT Construction Type Options:

- If Building is < 85 ft</li>
  - 7 stories of IV-C
  - 6 stories of IIIA or IV-HT over 1 story IA
- If Building is > 85 ft
  - 7 stories of IV-B

#### **Implications of Type IV-C:**

- 2 hr FRR, all exposed floor panels, beams, columns
- Likely will need at least 5-ply CLT / 2x6 NLT/DLT
- Efficient spans in the 14-17 ft range
- Efficient grids of that or multiples of that (i.e. 30x25, etc)
- No podium required



### **Construction Type Early Decision Example**

#### MT Construction Type Options:

- If Building is < 85 ft</li>
  - 7 stories of IV-C
  - 6 stories of IIIA or IV-HT over 1 story IA
- If Building is > 85 ft
  - 7 stories of IV-B

#### Implications of Type IIIA or IV-HT:

- 1 hr FRR or min. sizes
- Potential to use 3-ply or thin 5-ply CLT
- Efficient spans in the 10-12 ft range
- Efficient grids of that or multiples of that (i.e. 20x25, etc)
- 1 story Type IA podium required



### **Construction Type Early Decision Example**

#### MT Construction Type Options:

- If Building is < 85 ft</li>
  - 7 stories of IV-C
  - 6 stories of IIIA or IV-HT over 1 story IA
- If Building is > 85 ft
  - 7 stories of IV-B

#### Implications of Type IV-B:

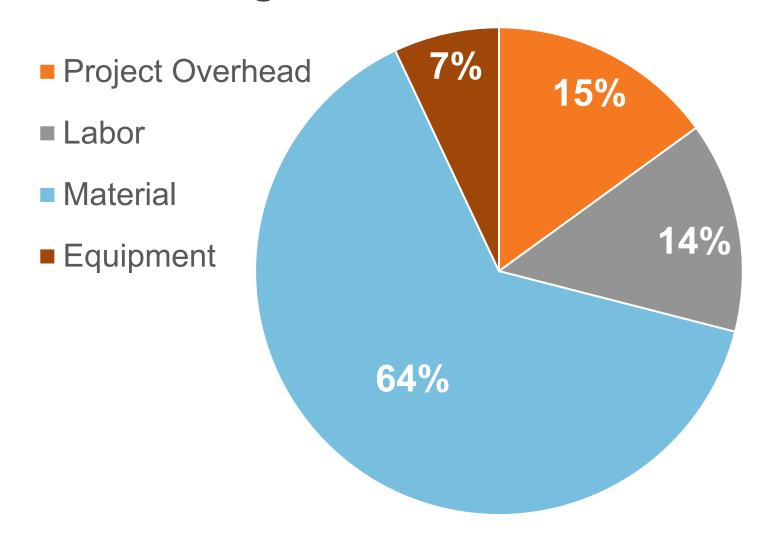
- 2 hr FRR, mostly protected floor panels, beams, columns
- Exposed areas: likely 5-ply / 2x6 NLT/DLT
- Protected areas: potential for thinner panels
- Choose 1 system throughout or multiple systems?
- Does grid vary or consistent throughout?
- No podium required

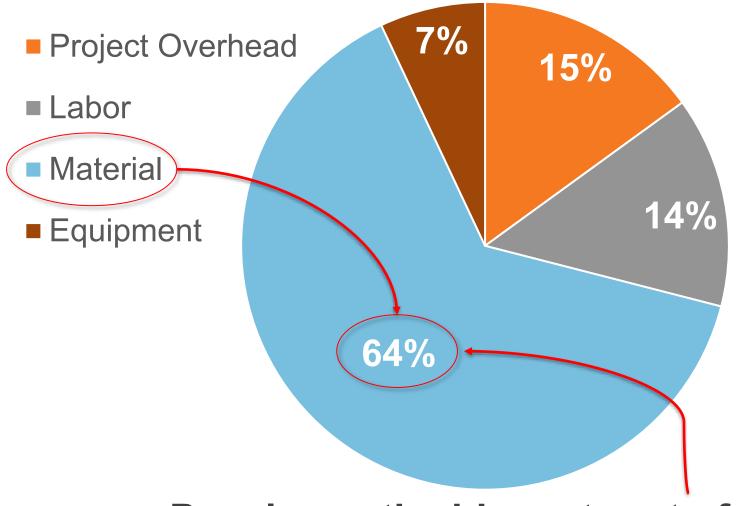


Why so much focus on panel thickness?



### **Typical MT Package Costs**

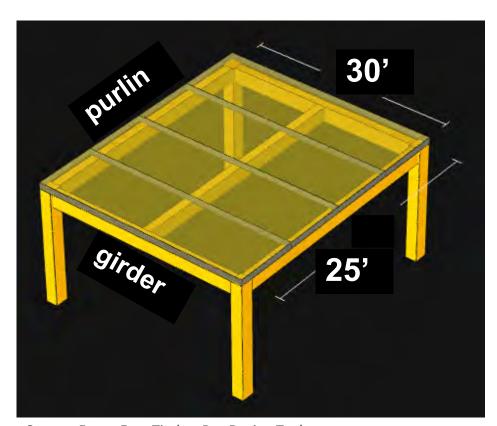




Panels are the biggest part of the biggest piece of the cost pie

Source: Swinerton

### Panel volume usually 65-80% of MT package volume



Source: Fast + Epp, Timber Bay Design Tool

### Type IIIA option 1

1-hr FRR

Purlin: 5.5"x28.5"

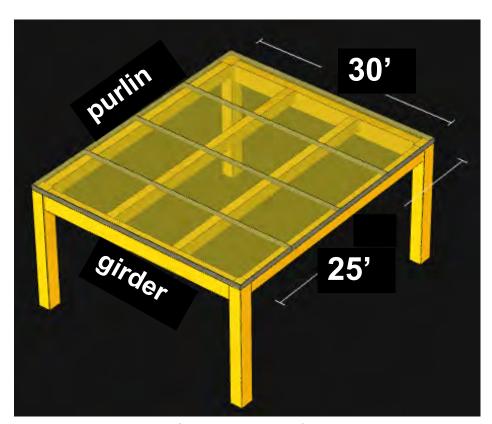
Girder: 8.75"x33"

Column: 10.5"x10.75"

Floor panel: 5-ply

Glulam volume = 118 CF (22% of MT) CLT volume = 430 CF (78% of MT) Total volume = 0.73 CF / SF

### Panel volume usually 65-80% of MT package volume



Source: Fast + Epp, Timber Bay Design Tool

### Type IIIA option 2

1-hr FRR

Purlin: 5.5"x24"

Girder: 8.75"x33"

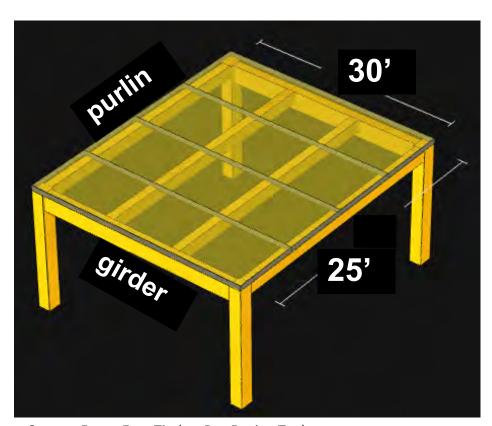
Column: 10.5"x10.75"

Floor panel: 5-ply

Glulam volume = 123 CF (22% of MT) CLT volume = 430 CF (78% of MT) Total volume = 0.74 CF / SF

Cost considerations: One additional beam (one additional erection pick), 2 more connections

#### Panel volume usually 65-80% of MT package volume



Source: Fast + Epp, Timber Bay Design Tool

#### **Type IV-HT**

0-hr FRR (min sizes per IBC)

Purlin: 5.5"x24" (IBC min = 5"x10.5")

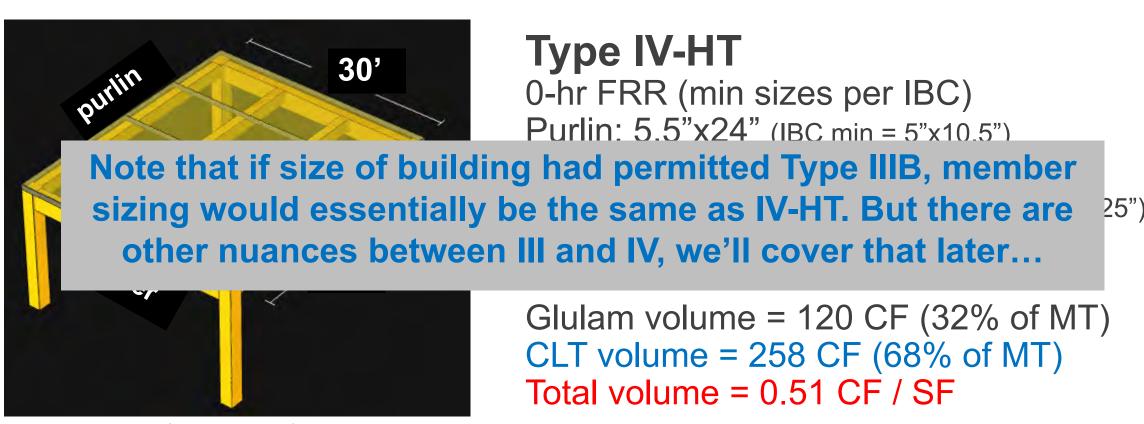
Girder: 8.75" $\times 33$ " (IBC min = 5" $\times 10.5$ ")

Column: 10.5"x10.75" (IBC min = 6.75"x8.25")

Floor panel: 3-ply (IBC min = 4" CLT)

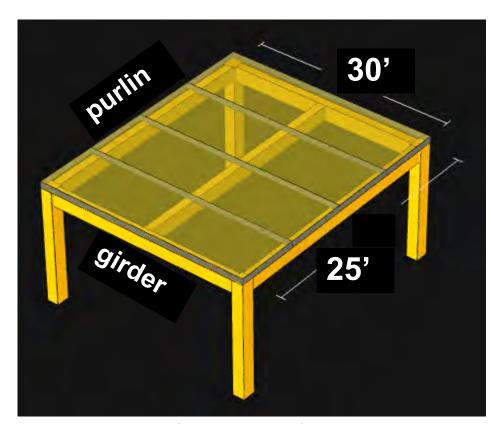
Glulam volume = 120 CF (32% of MT) CLT volume = 258 CF (68% of MT) Total volume = 0.51 CF / SF

#### Panel volume usually 65-80% of MT package volume



Source: Fast + Epp, Timber Bay Design Tool

#### Panel volume usually 65-80% of MT package volume



Source: Fast + Epp, Timber Bay Design Tool

#### Type IV-C

2-hr FRR

Purlin: 8.75"x28.5"

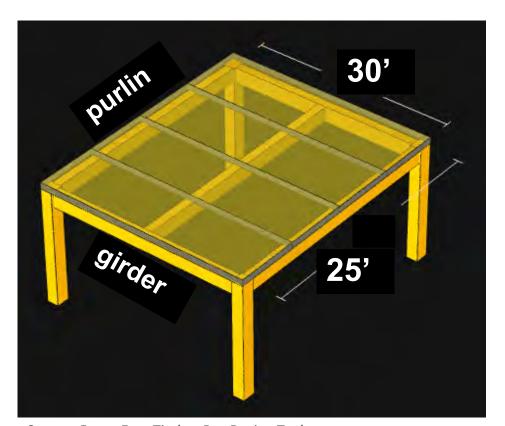
Girder: 10.75"x33"

Column: 13.5"x21.5"

Floor panel: 5-ply

Glulam volume = 183 CF (30% of MT) CLT volume = 430 CF (70% of MT) Total volume = 0.82 CF / SF

#### Which is the most efficient option?

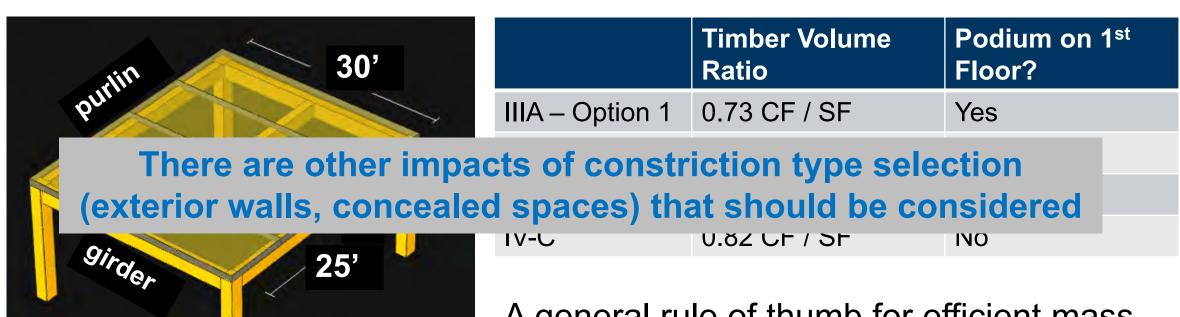


Source: Fast + Epp, Timber Bay Design Tool

	Timber Volume Ratio	Podium on 1 <sup>st</sup> Floor?
IIIA – Option 1	0.73 CF / SF	Yes
IIIA – Option 2	0.74 CF / SF	Yes
IV-HT	0.51 CF / SF	Yes
IV-C	0.82 CF / SF	No

A general rule of thumb for efficient mass timber fiber volume is no higher than 0.75 CF per SF. Ratios in the 0.85 to 1.0 CF / SF range tend to become cost prohibitive

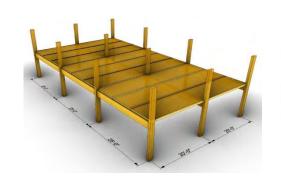
#### Which is the most efficient option?

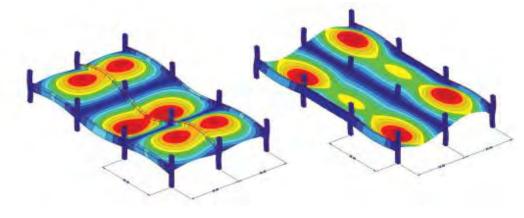


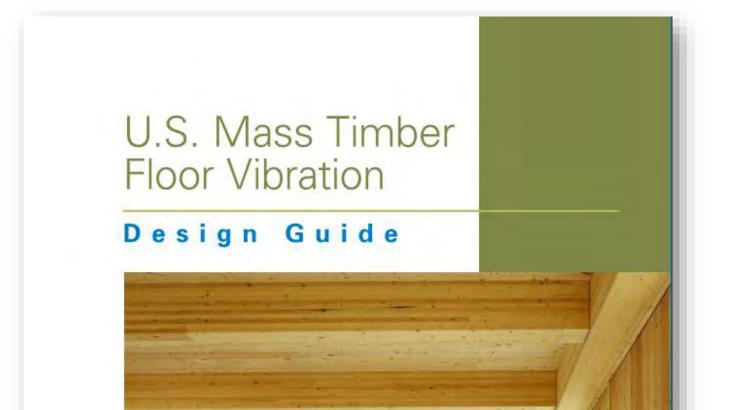
Source: Fast + Epp, Timber Bay Design Tool

A general rule of thumb for efficient mass timber fiber volume is no higher than 0.75 CF per SF. Ratios in the 0.85 to 1.0 CF / SF range tend to become cost prohibitive

# NEW MASS TIMBER FLOOR VIBRATION DESIGN GUIDE







Worked office, lab and residential Examples

Covers simple and complex methods for bearing wall and frame supported floor systems



Events



Award Gallery Why Wood?

About

< Back To Award Gallery

# MSU STEM Teaching and Learning Facility

Location:

East Lansing, Michigan

Architect:

Integrated Design Solutions (AOR & power plant), Ellenzweig (STEM addition)

Structural Engineer:

SDI Structures

Contractor:

**Granger Construction** 

**Award Category:** 

Wood in Schools

Photos:

Christofer Lark, Kevin Marshall/Integrated Design Solutions

View Project on WIN



Free Project Support from WoodWorks - Learn more

Save ☆

#### **Michigan State University** STEM Teaching and **Learning Facility**

**BLDG SYSTEM** Mass Timber NO. OF STORIES

SQ. FOOTAGE 160,000

PEast Lansing, MI









COMPANIES INVOLVED:

Ellenzweig

Architect

IDS - Integrated Design

Project Architect

**Granger Construction** 

General Contractor

Solutions

**Chistman Constructors** 

Erector

IDS - Integrated Design

Solutions

Engineer

Rothoblaas

Tools

SDI Structures

Structural Engineer

Sansin

Timber finishes

INDIVIDUALS INVOLVED:

Jeffrey Johnson

Architect

Peter Heeringa

Structural Engineer



Many ways to demonstrate connection fire protection: calculations, prescriptive NC, test results, others as approved by AHJ

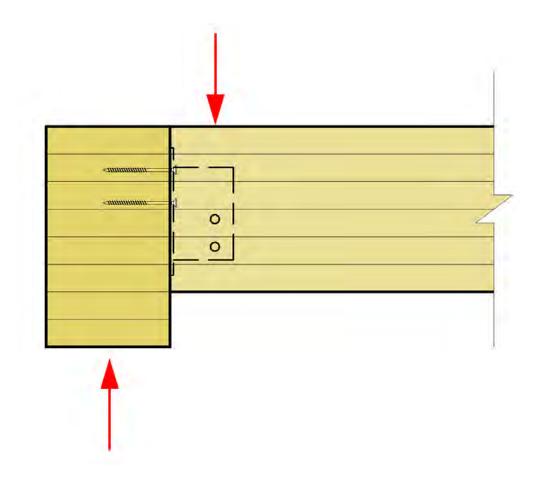






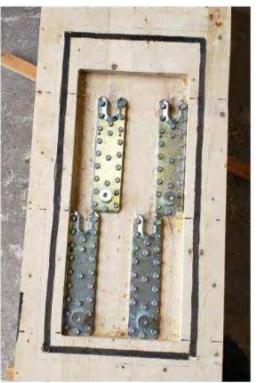


Steel hangers/hardware fully concealed within a timber-to-timber connection is a common method of fire protection

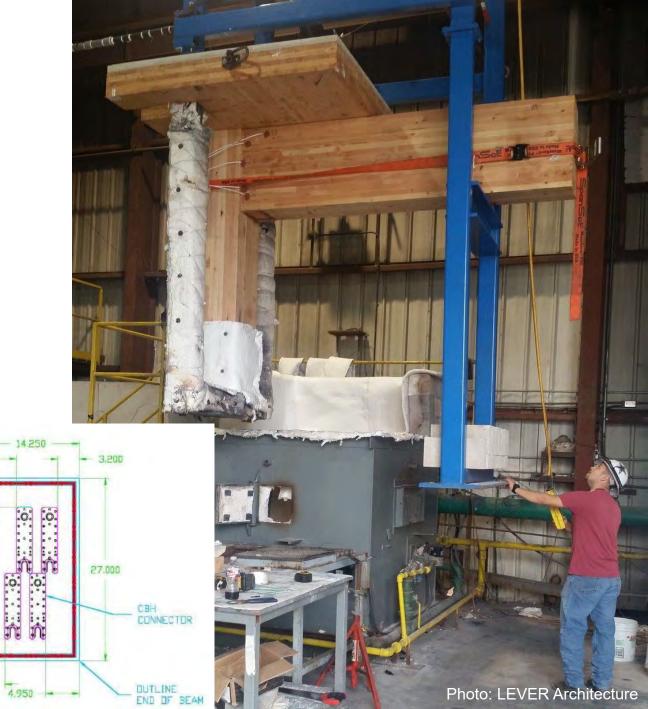




Connection FRR and beam reactions could impact required beam/column sizes







Photos: Simpson Strong-Tie

2017 Glulam Beam to Column Connection Fire Tests under standard ASTM E119 time-temperature exposure







#### **Fire Test Results**

Test	Beam	Connector	Applied Load	FRR
1	8.75" x 18" (222mm x 457mm)	1 x Ricon S VS 290x80	3,905lbs (17.4kN)	1hr
2	10.75" x 24" (273mm x 610mm)	Staggered double Ricon S VS 200x80	16,620lbs (73.9kN)	1.5hrs
3	10.75" x 24" (273mm x 610mm)	1 x Megant 430	16,620lbs (73.9kN)	1.5hrs

#### Softwood Lumber Board

# Glulam Connection Fire Test Summary Report

Issue | June 5, 2017

#### SOUTHWEST RESEARCH INSTITUTE

6220 CULEBRA ROAD 78238-5166 . PO DRAWER 28510 78228 0510 . SAN ANTONIO, TEXAS, USA . (210) 684-5111 . WWW.SWRI ORG

CHEMISTRY AND CHEMICAL ENGINEERING DIVISION

IRE TECHNOLOGY DEPARTMENT WWW.FIRE.SWRI.ORG FAX (210) 522-3377



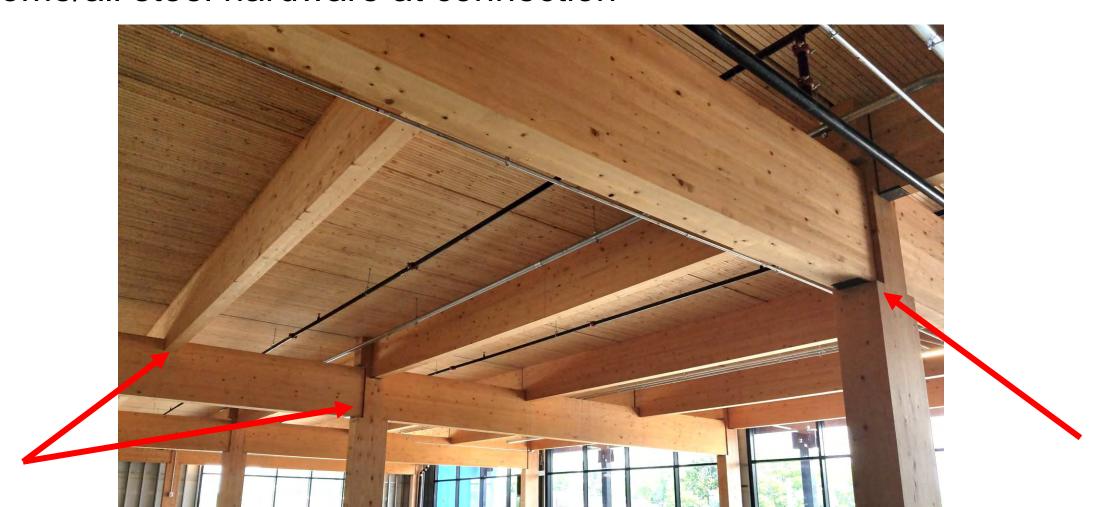
FIRE PERFORMANCE EVALUATION OF A LOAD BEARING GLULAM BEAM TO COLUMN CONNECTION, INCLUDING A CLT PANEL, TESTED IN GENERAL ACCORDANCE WITH ASTM E119-16a, STANDARD TEST METHODS FOR FIRE TESTS OF BUILDING CONSTRUCTION AND MATERIALS

FINAL REPORT Consisting of 32 Pages

#### Full Report Available at:

https://www.thinkwood.com/wp-content/uploads/2018/01/reThink-Wood-Arup-SLB-Connection-Fire-Testing-Summary-web.pdf

Member to member bearing also commonly used, can avoid some/all steel hardware at connection



Member to member bearing also commonly used, can avoid some/all steel hardware at connection



Style of connection also impacts and is impacted by grid layout and MEP integration









ARCHITECTURE
URBAN DESIGN
INTERIOR DESIGN

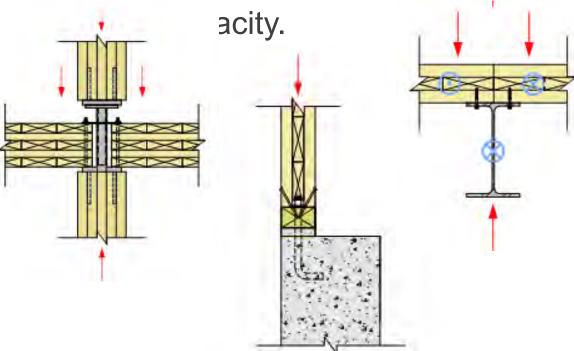


## WoodWorks Index of Mass Timber Connections



# MASS TIMBER CONNECTIONS INDEX

A library of commonly used mass timber connections with designer notes and information on fire resistance, relative cost and load-



#### **Connections**

# Other connection design considerations:

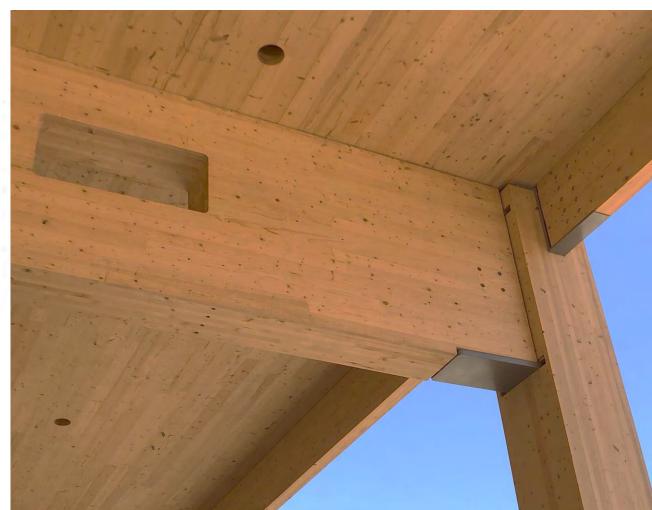
- Structural capacity
- Shrinkage
- Constructability
- Aesthetics
- Cost





Construction Type Impacts FRR | FRR impacts penetration firestopping requirements

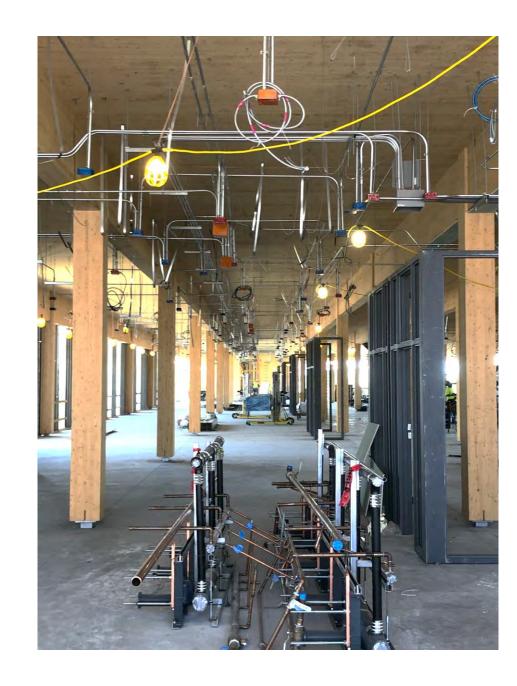
714.1.1 Ducts and air transfer openings. Penetrations of fire-resistance-rated walls by ducts that are not protected with dampers shall comply with Sections 714.3 through 714.4.3. Penetrations of horizontal assemblies not protected with a shaft as permitted by Section 717.6, and not required to be protected with fire dampers by other sections of this code, shall comply with Sections 714.5 through 714.6.2. Ducts and air transfer openings that are protected with dampers shall comply with Section 717.



Code options for firestopping through penetrations

714.4.1.1 Fire-resistance-rated assemblies. Through penetrations shall be protected using systems installed as tested in the approved fire-resistance-rated assembly.

714.4.1.2 Through-penetration firestop system. Through penetrations shall be protected by an approved penetration firestop system installed as tested in accordance with ASTM E814 or UL 1479, with a minimum positive pressure differential of 0.01 inch (2.49 Pa) of water and shall have an F rating of not less than the required fire-resistance rating of the wall penetrated.

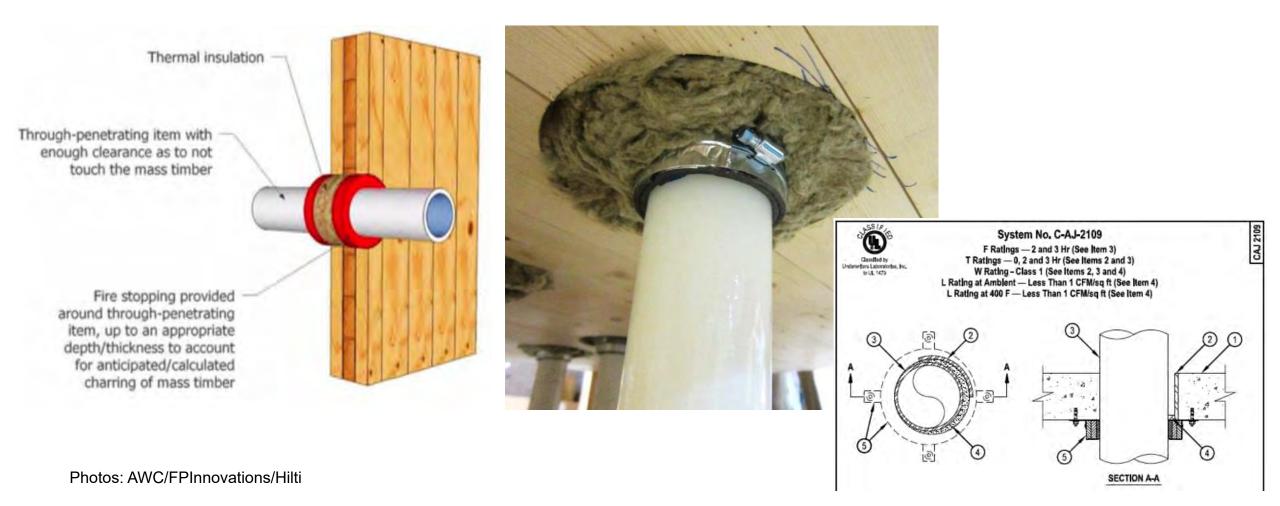


Option 1: MT penetration firestopping via tested products





Most firestopping systems include combination of fire safing (eg. noncombustible materials such as mineral wool insulation) plus fire caulk



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CHEMISTRY AND CHEMICAL ENGINEERING DIVISION

FIRE TECHNOLOGY DEPARTMENT WWW.FIRE.SWRI.ORG FAX (210) 522-3377



WCTE 2018

2013 Woold Conference on Timber Engineering
August 26 - 212-25rd : Booul, Republic of Norma

GHL

CONSULTANTS LTD

FIRE RESISTANCE PERFORMANCE EVALUATION OF A PENETRATION FIRESTOP SYSTEM TESTED IN ACCORDANCE WITH ASTM E814-13A, STANDARD TEST METHOD FOR FIRE TESTS OF PENETRATION FIRESTOP SYSTEMS

FINAL REPORT Consisting of 18 Pages

SwRI® Project No. 01.21428.01.001a Test Date: September 30, 2015 Report Date: October 22, 2015

Prepared for:

American Wood Council 222 Catoctin Circle SE Leesburg, VA 20175

#### FIRE PERFORMANCE OF FIRESTOPS, PENETRATIONS, AND FIRE DOORS IN MASS TIMBER ASSEMBLIES

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ABSTRACT: Integrity and continuity must be maintained for fire separations required to provide f prevent passage of hot gases or increased temperature on the unexposed side. Vulnerable locations, who are introduced into mass timber systems, are susceptible to fire spread. Service and closure penetral timber fire separation have been investigated. Many of the fire stop systems were able to achieve 1-½ accordance with CAN/ULC-\$115, which would be required for 2-hr fire resistance rated assemblies, stall wood buildings. Construction details are outlined which ensure adequate fire performance of these

**KEYWORDS:** Firestop, through-penetrations, fire rated door, mass timber, cross-laminated timbuildings, fire resistance

#### 1 INTRODUCTION

Many tall wood buildings using mass timber are planned or are currently being designed for construction around the world. A few have been built in Canada, including an 18 storey cross-laminated timber (CLT) and glulam building in British Columbia. The prescriptive requirements in the National Building Code of Canada (NBCC) [1] do not (yet) permit the construction of wood buildings taller than six stories, however an alternative solutions approach can be used to demonstrate equivalent performance to prescriptive acceptable

construction, as well as in several alte building designs.

Although the general fire performance well documented, there are still seve warrant further investigation to ensur safety levels are met and a number available for designers to use. Generatin generic assemblies will reduce the need completed on an individual constructio which will help ease the approvals proce widespread adoption of tall wood buildin

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#### FIRESTOPPING TEST WITNESS REPORT

for

NORDIC STRUCTURES

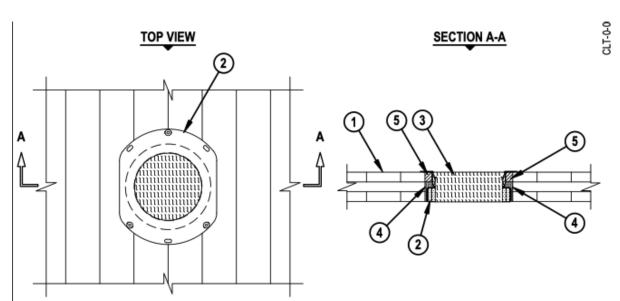
#### Inventory of Fire Tested Penetrations in MT Assemblies

#### Table 3: North American Fire Tests of Penetrations and Fire Stops in CLT Assemblies



CLT Panel	Exposed Side Protection	Penetrating Item	Penetrant Centered or Offset in Hole	Firestopping System Description	F Rating	T Rating	Stated Test Protocal	Source	Testing Lab
3-ply (78mm(3.07*)	None	1.5* dismeter data cable bunch	Centered	5.5 in diameter hole. Mineral wool was installed in the 1 in. annular space around the data cables to a total depth of approximately 2 - 5/64 in. The remaining 1 in, annular space from the top of the mineral wool to the top of the floor assembly was filled with Hilli FS-One Max can king.	1 hour	0.5 hour	CANULC SHS	26	Intertek March 30, 2016
3-ply (78mm3.075)	None	2" copporpipe	Centwed	4.375 in diameter hole. Pips wrap was installed around the copper pipe to a total depth of approximately 2 - 5/64 in. The remaining 1 inmailar space starting at the top of the mineral wood to the top of the floor as sembly was filled with Hilli FS-One Max caulking.	4 forur	NA.	CANULC SUS	26	Întortek Minch 30, 2016
3-ply (78mm3 /07*)	None	2.5" school 40 pipe	Centured	4.92 in diameter hole. Pipe was installed around the schedule 40 pipe to a total depth of approximately 2 - 5/64 in. The remaining 1 in. annular space starting at the top of the pipe was to the top of the floor assembly was filled with 1 lift FS-One Max coulding.	1. hour -	NA.	CANUIC SITS	26	Intertek Murch 30, 2016
3-ply (78mm3.82°)	None	6" cast from pipe	Centured	8.35 in diameter hole. Minoral wool was installed in the Lin annular space around the cast iron pipe to a total depth of approximately 2 - 5/64 in. The termining Lin annular space starting at the top of the pipe wrap to the top of the floor assembly was filled with Hilti FS- One Max caulking	1 hour	NA.	CANULC S113	26	Intortek March 30, 2016
3-ply (78mm 3.07*)	None	Hilli 6 in drop in device System No.: F-H-2049	Centered	9.01° diameter hole. Mineral wool was installed in the 1 – 1/4 in annular space around the drop-in device to a total depth of approximately 1 – 7/64 in and the remaining 1 in annular space from the top of the mineral wool to the top edge of the 9 – 1/64 in hole in the CLT was filled with full if FS-One Max caulking.	I hour	0.75 hour	CANULC SHIS	26	Intertek March 30, 2016
5-ply CET 131 mm 5.16*)	None	1.5* diameter data subje bunch	Contered	3.5° diameter hole. Mineral wool was installed in the fin. annular space around the data cables to a total depth of approximately 4 – 5/32 in. The remaining 1 in annular space from the top of the mineral wool to the top of the floor assembly was filled with Hilli FS-One Max casil king.	2 hours	1.5 hours	CANULE SILE	26	Interest March 50, 2016
5-ply CLT (131/mm 5.16*)	None	2* copperpipe	Centered	4.375 in diameter hole. Pipe wrap was installed around the cupper pipe to a total depth of approximately 4 = 5/32 in The remaining 1 in annular space starting at the top of the mineral wood to the top of the floor mountly was filled with Hilli FS One Max can lking.	2 hours	NA.	CANUTE STIS	26	Intertek March 30 , 20 16
5-ply CLT (13) mm 5.16*)	None	2.57 schod. 40 pipe	Contend	4,92 in diameter hole. Pipe wrap was installed around the schedule 40 pipe to a total depth of approximately 4 - 5/32 in The remaining Lin. annular space starting at the top of the pipe wrap to the top of the floor assembly was filled with Hilli FS-One Max caulking.	2 hours	0.5 hour	CANULE S113	26	Interick March 30, 2016
5-ply CLT (131 mm 5.16*)	None	6" cast from pipe	Centered	8.35 in diameter hole. Mineral woo I was installed in the Lin. annular space around the east from pipe to a total depth of approximately 4 - 5/52 in. The termining Lin. annular space starting at the top of the pipe wrap to the top of the flowr assembly was filled with Hilti FS- One Max coalking.	2 hours	NA.	CANUTE STIS	26	Intertek March 30, 2016
5-ply CLT (131 mm 5-16*)	None	Hilti 6 in drop in device System No.: F-B-2049	Centered	9.61° diameter hole. Mineral wool was installed in the 1 - 1/4 in annular space around the drop-in device to a total depth of approximately 1 - 7/6 4 in and the remaining 1 in annular space from the top of the mineral wool to the top edge of the 9 - 1/6 4 in thole in the CLT was filled with fall if FS-One Max caulking.	2 hours	1.5 hours	CANULC SHIS	26	Intertek March 30, 2016
5-ply 175mm 6 #75*)	Nano	1° nomenal PVC pipe	Contored	4.21 in diameter with a 3/4 in plywood reducer flush with the top of the slab reducing the opening to 2.28 in. Two wraps of fill if CP 648-E W45/1-3/4" Firest op wrap strip at two locations with a 30 gauge steel sleeve which extended from the top of the slab to 1 in below the slab. The first location was with the bottom of the wrap strip flush with the bottom of the steel sleeve and the second was with the bottom of the wrap strip flush with the bottom of the steel sleeve and the second was with the bottom of the wrap strip 3 in. If on the bottom of the stat. The void between the steel sleeve and the steel sleeve and pipe at the top we filled with Roxal Safe mineral wool leaving a 3/4 in deep void at the top of the assembly. Hilli PS One Max intunescent Firestop Scalant was applied to a depth of 3/4 in on the top of the assembly between the plywood and steel sleeve as well as the steel sleeve and pipe.	2 hours	2 hours	ASTM DE14	24	QALLaboratorio March 3, 2917

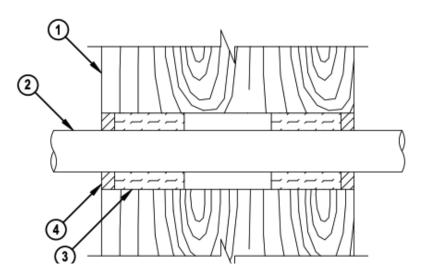
Option 2: MT penetration firestopping of penetrations via engineering judgement details (contact firestop manufacturer)



- 1. 3-PLY CROSS LAMINATED TIMBER FLOOR ASSEMBLY (MINIMUM 3" THICK) (1-HR. FIRE-RATING).
- 2. HILTI CFS-DID FIRESTOP DROP-IN DEVICE INSERTED INTO OPENING (SEE TABLE BELOW) AND SECURED TO TOP SURFACE OF CROSS LAMINATED TIMBER FLOOR ASSEMBLY WITH THREE 1/4" x 1" LONG STEEL WOOD SCREWS WITH WASHERS.
- 3. MINIMUM 3" THICKNESS MINERAL WOOL (MIN. 4 PCF DENSITY) TIGHTLY PACKED, AND FLUSH WITH TOP AND BOTTOM SURFACE OF CFS-DID FIRESTOP DROP-IN DEVICE.
- 4. MINERAL WOOL (MIN. 4 PCF DENSITY) TIGHTLY PACKED, RECESSED TO ACCOMMODATE SEALANT, AND COMPLETELY FILLING SPACE BETWEEN CFS-DID FIRESTOP DROP-IN DEVICE AND PERIPHERY OF OPENING.
- 5. MINIMUM 1" DEPTH HILTI FS-ONE MAX INTUMESCENT FIRESTOP SEALANT BETWEEN CFS-DID FIRESTOP DROP IN DEVICE AND PERIPHERY OF OPENING.

F-RATING = 1-HR. OR 2-HR. (SEE NOTE NO. 3 BELOW)

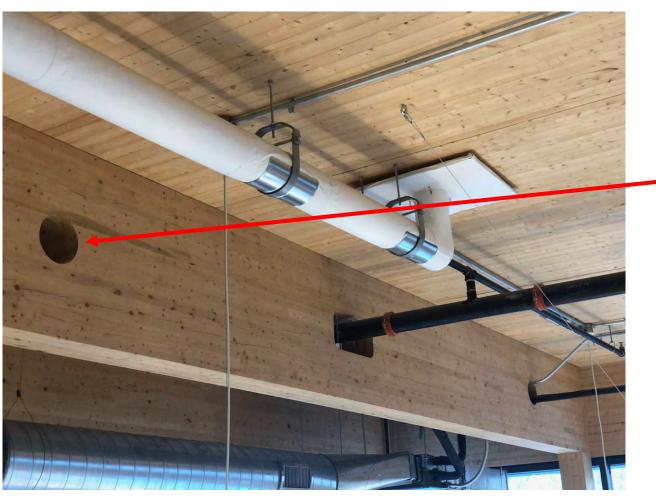
#### CROSS-SECTIONAL VIEW

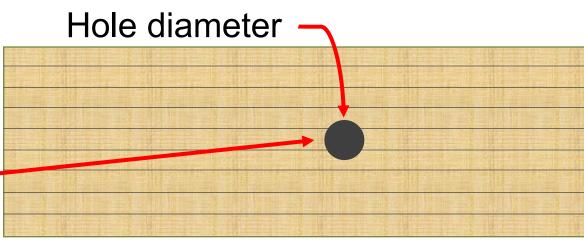


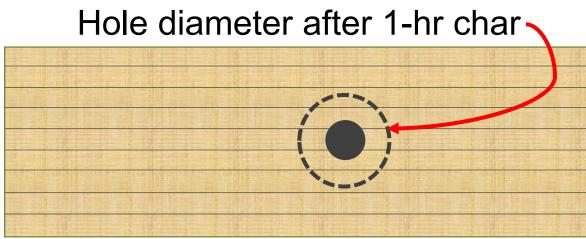
- 1. MASS TIMBER WALL ASSEMBLY (MINIMUM 12" THICK) (1-HR. OR 2-HR. FIRE-RATING).
- 2. MAXIMUM 2" NOMINAL DIAMETER PVC PLASTIC PIPE (SCH 40).
- 3. MINIMUM 4" THICKNESS MINERAL WOOL (MIN. 4 PCF DENSITY) TIGHTLY PACKED AND RECESSED TO ACCOMMODATE SEALANT.
- 4. MINIMUM 3/4" DEPTH HILTI FS-ONE MAX INTUMESCENT FIRESTOP SEALANT.

#### Beam penetrations:

- If FRR = 0-hr, analyze structural impact of hole diameter only
- If FRR > 0-hr, account for charred hole diameter or firestop penetration



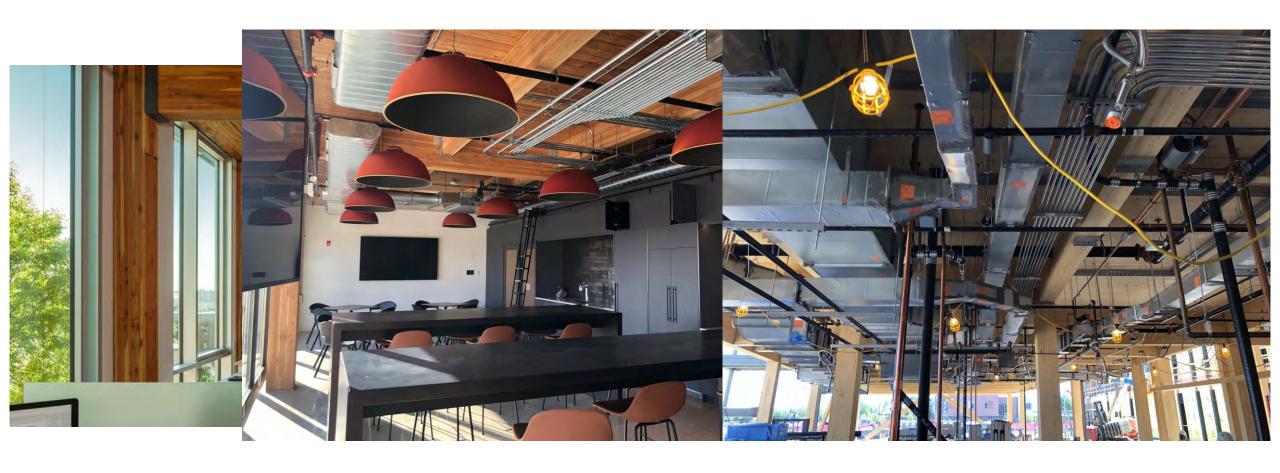






Set Realistic Owner Expectations About Aesthetics

MEP fully exposed with MT structure, or limited exposure?



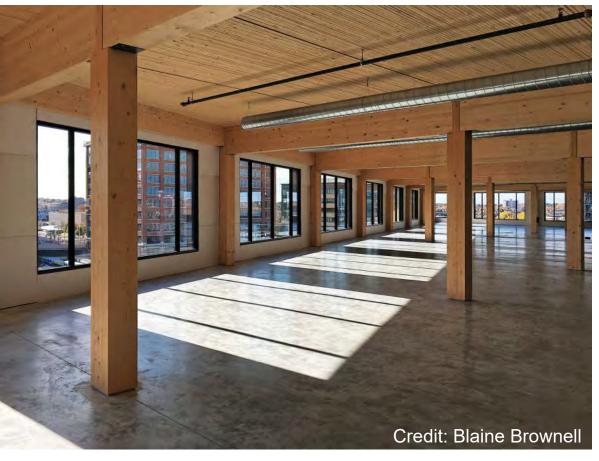
#### Key considerations:

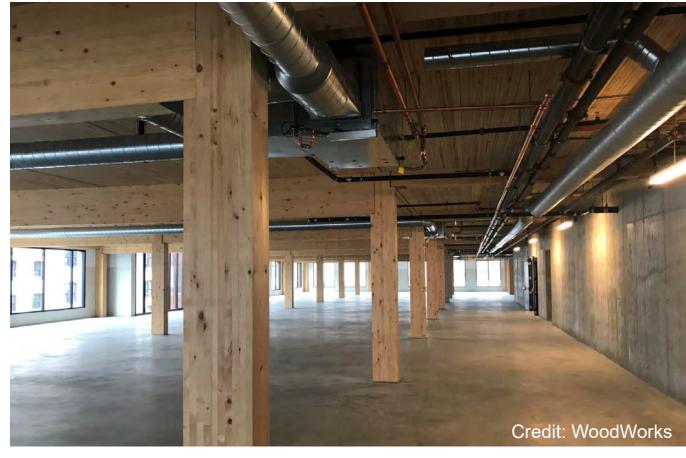
- Level of exposure desired
- Floor to floor, structure depth & desired head height
- Building occupancy and configuration (i.e. central core vs. double loaded corridor)
- Grid layout and beam orientations
- Need for future tenant reconfiguration
- Impact on fire & structural design: concealed spaces, penetrations

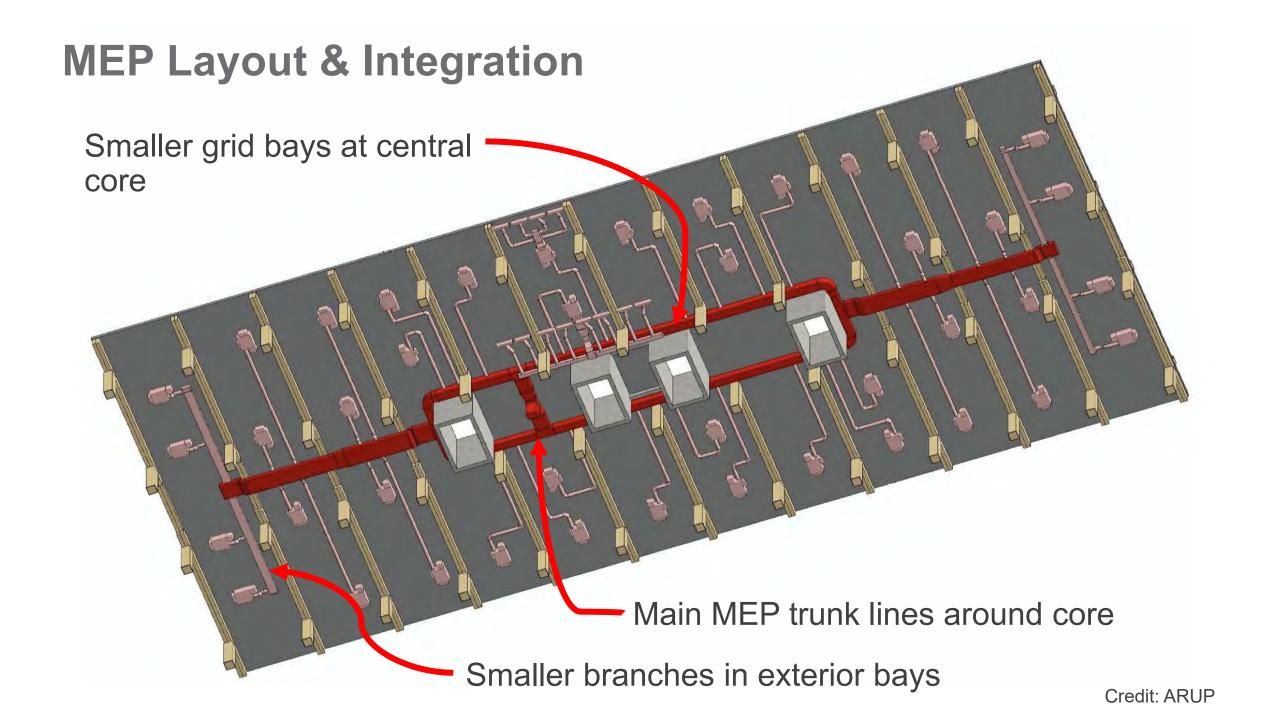


Smaller grid bays at central core (more head height)

• Main MEP trunk lines around core, smaller branches in exterior bays







Grid impact: Relies on one-way beam layout. Columns/beams spaced at panel span limits in one direction.

Beam penetrations are minimized/eliminated

Panel

2x4 NLT

2x6 NLT

2x8 NLT 5" MPP

Recall typical panel span

3-ply CLT (4-1/8" thick)

5-ply CLT (6-7/8" thick)

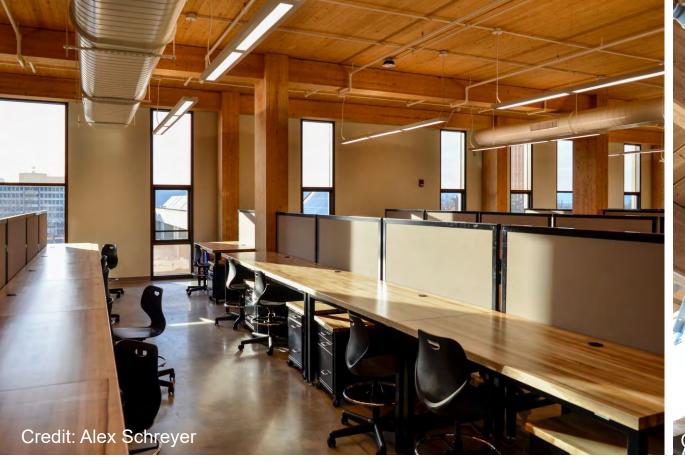
7-ply CLT (9-5/8")

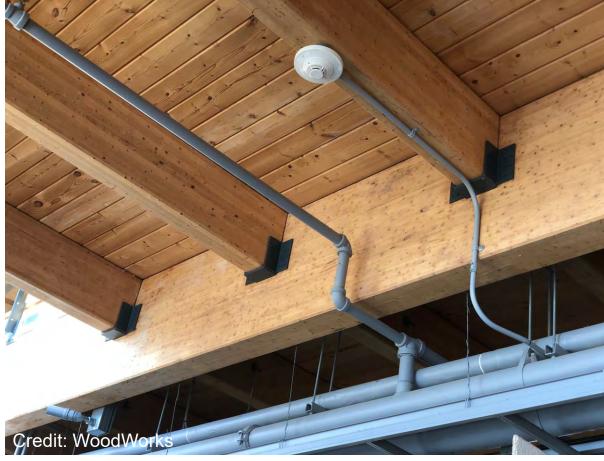
limits:

ed one		MT Panel Span Bearn	Span
e an			
Exampl	e Floor Span Ranges		
Up to 1			
14 to 1			
17 to 2	1ft		
Up to 1	2 ft		
10 to 1	7 ft		
14 to 2	1 ft		
10 to 15	5 ft		Credit: Hacker Architects

#### Dropped below MT framing

- Can simplify coordination (fewer penetrations)
- Bigger impact on head height





Grid impact: Usually more efficient when using a square-ish grid with beams in two directions





Credit: SOM Timber Tower Report

In penetrations through MT framing

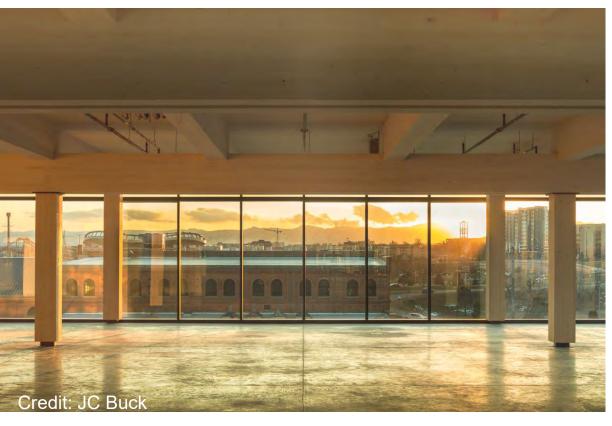
- Requires more coordination (penetrations)
- Bigger impact on structural capacity of penetrated members
- Minimal impact on head height





In chases above beams and below panels

- Fewer penetrations
- Bigger impact on head height (overall structure depth is greater)
- FRR impacts: top of beam exposure





In chases above beams and below panels at Platte 15

• 30x30 grid, purlins at 10 ft, 3-ply CLT

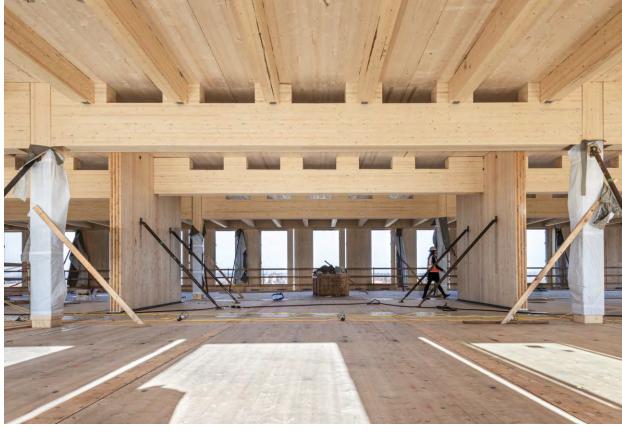




In chases above beams and below panels at Catalyst

• 30x30 grid, 5-ply CLT ribbed beam system





In gaps between MT panels

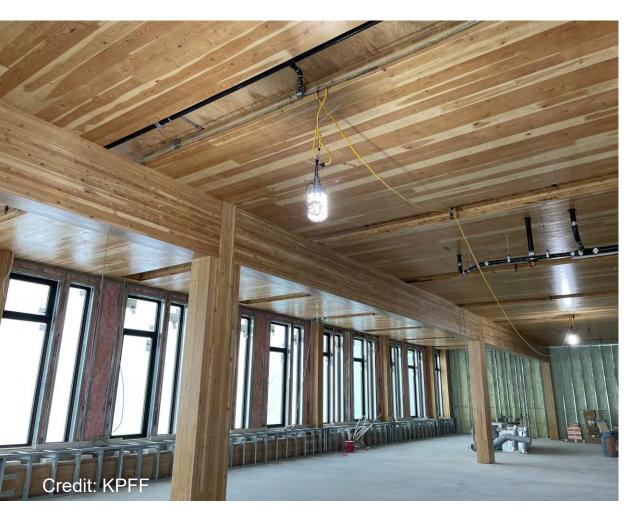
Fewer penetrations, can allow for easier modifications later

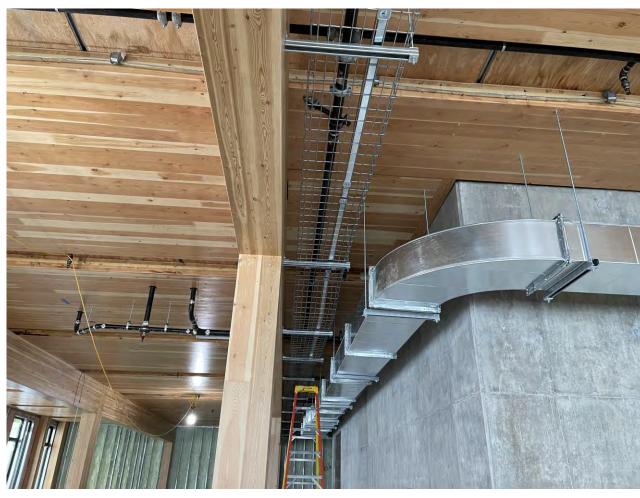




In gaps between MT panels

• FRR impacts: generally topping slab relied on for FRR





In gaps between MT panels

Impact on assembly acoustics performance





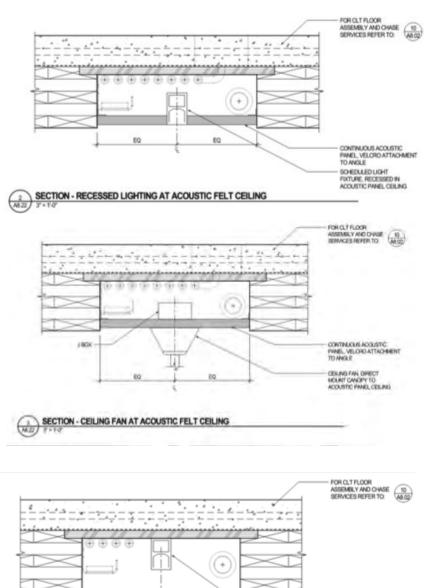
Credit: KPFF

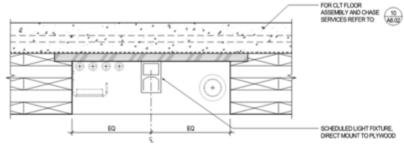
In gaps between MT panels

Greater flexibility in MEP layout









SECTION - LIGHTING IN EXPOSED CLT CHASE

Credit: PAE Consulting Engineers

In gaps between MT panels

Aesthetics: often uses ceiling panels to cover gaps



In raised access floor (RAF) above MT

Aesthetics (minimal exposed MEP)



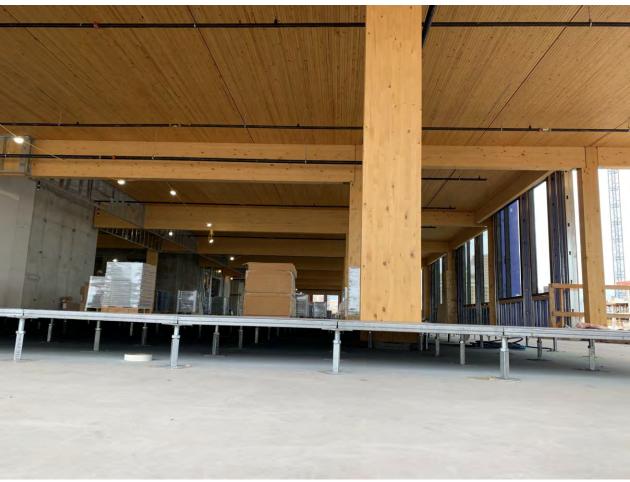




In raised access floor (RAF) above MT

- Impact on head height
- Concealed space code provisions





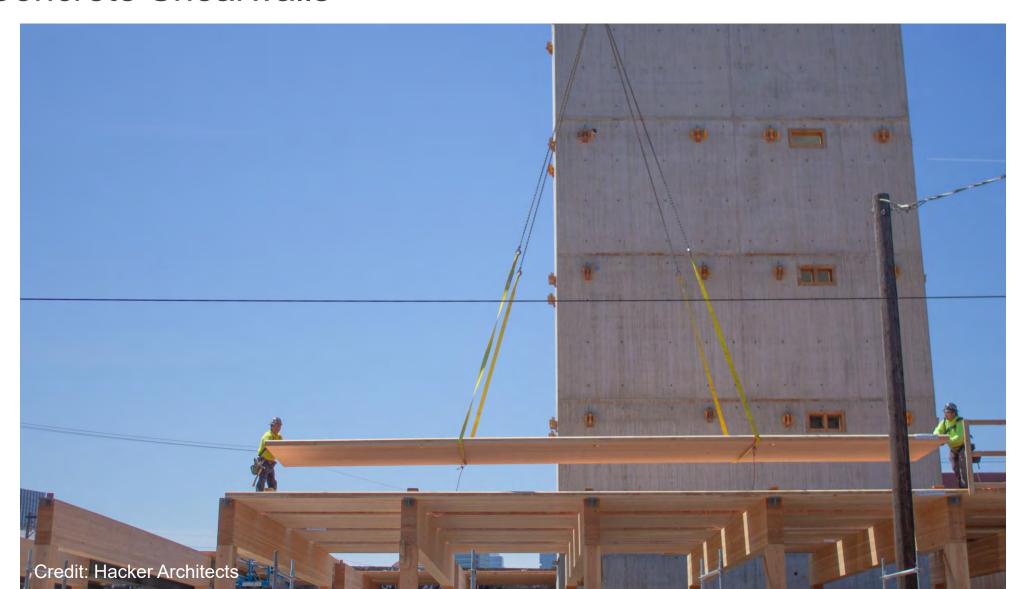
### In topping slab above MT

- Greater need for coordination prior to slab pour
- Limitations on what can be placed (thickness of topping slab)
- No opportunity for renovations later

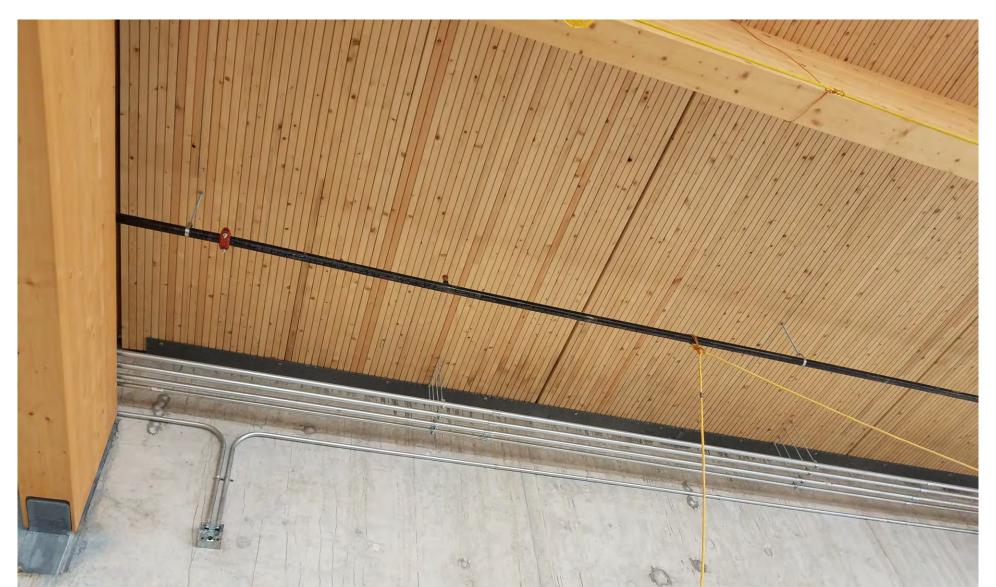




### **Concrete Shearwalls**



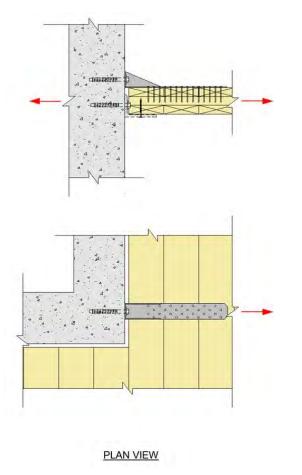
Connection to concrete core

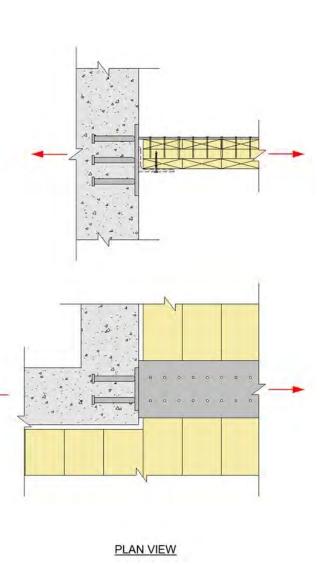


Connections to concrete core

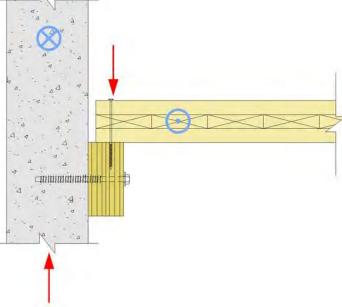
Tolerances & adjustability

Drag/collector forces

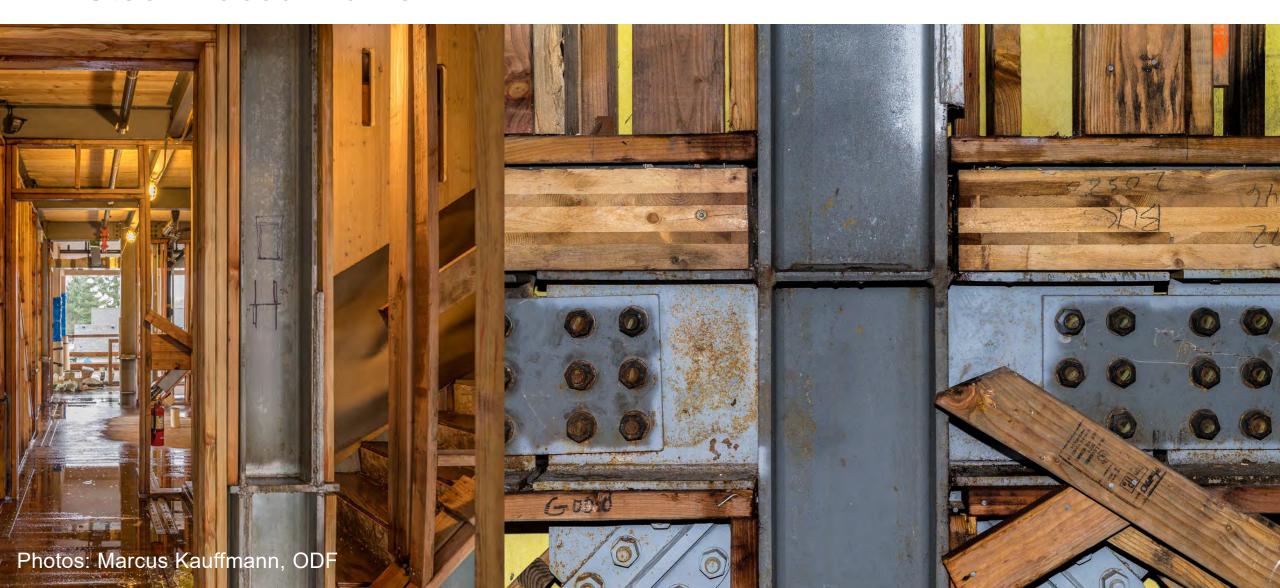






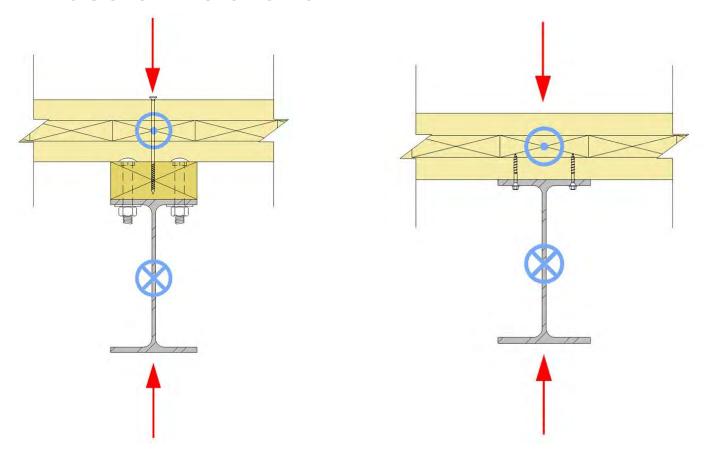


Steel Braced Frame



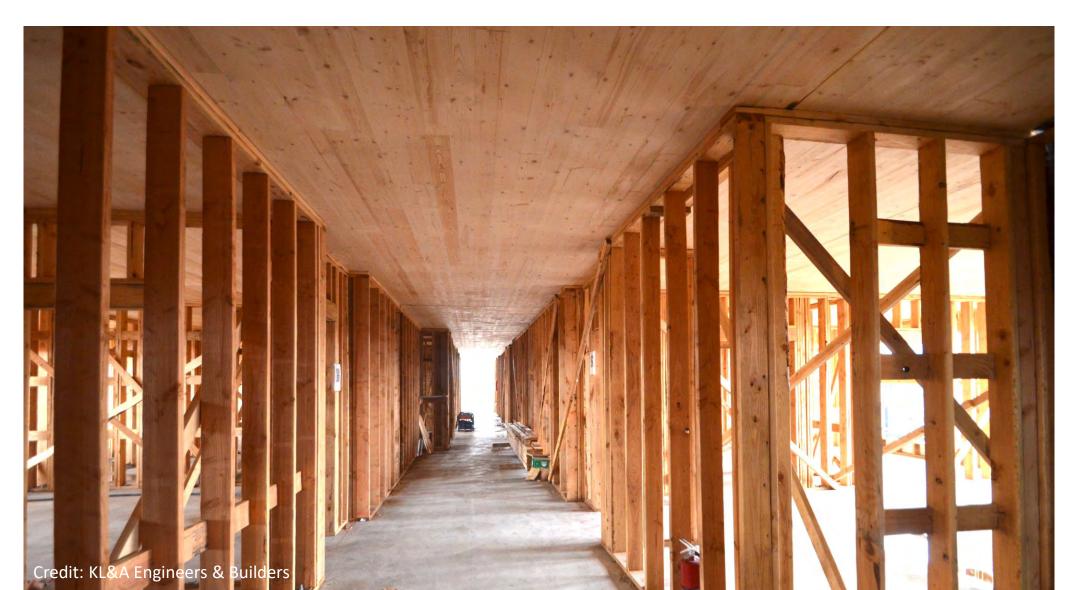
#### Connections to steel frame

- Tolerances & adjustability
- Consider temperature fluctuations
- Ease of installation





**Wood-Frame Shearwalls** 



#### Wood-frame Shearwalls:

- Code compliance
- Standard of construction practice well known
- Limited to 65 ft shearwall height, 85 ft overall building height (Type IIIA construction)

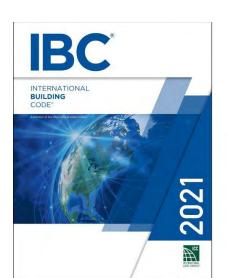


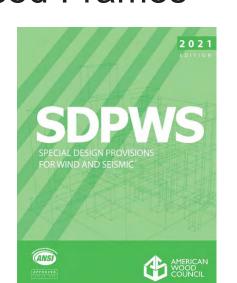




### **Prescriptive Code Compliance**

Concrete Shearwalls
Steel Braced Frames
Light Wood-Frame Shearwalls
CLT Shearwalls
CLT Rocking Walls
Timber Braced Frames















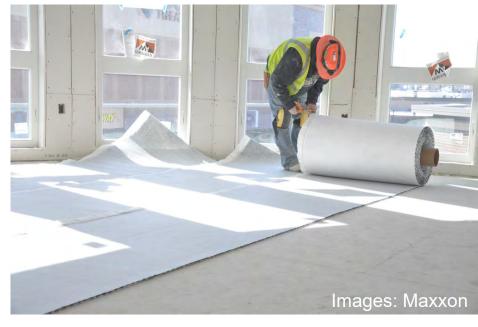
#### Consider Impacts of:

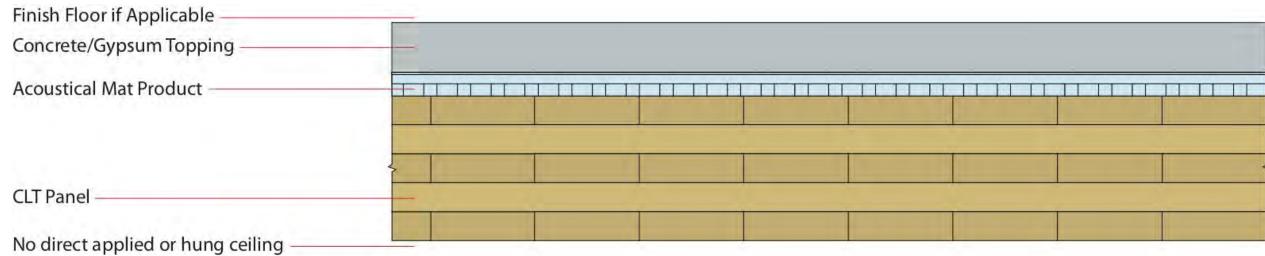
- Timber & Topping Thickness
- Panel Layout
- Gapped Panels
- Connections & Penetrations
- MEP Layout & Type



Credit: Rothoblaas



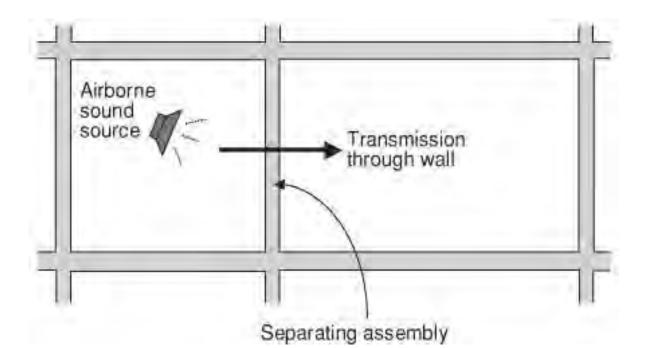




#### **Air-Borne Sound:**

### **Sound Transmission Class (STC)**

- Measures how effectively an assembly isolates air-borne sound and reduces the level that passes from one side to the other
- Applies to walls and floor/ceiling assemblies

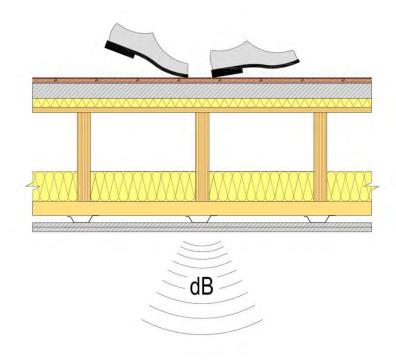




#### **Structure-borne sound:**

#### Impact Insulation Class (IIC)

- Evaluates how effectively an assembly blocks impact sound from passing through it
- Only applies to floor/ceiling assemblies





Code requirements only address residential occupancies:

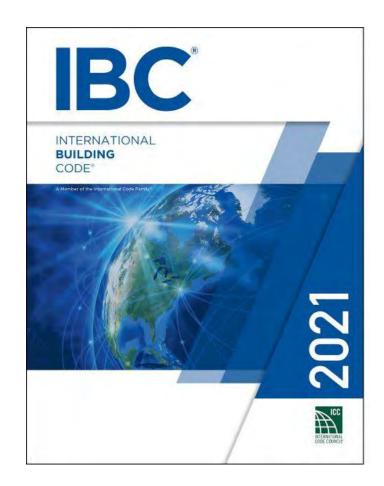
For unit to unit or unit to public or service areas:

#### Min. STC of 50 (45 if field tested):

Walls, Partitions, and Floor/Ceiling Assemblies

#### Min. IIC of 50 (45 if field tested) for:

Floor/Ceiling Assemblies

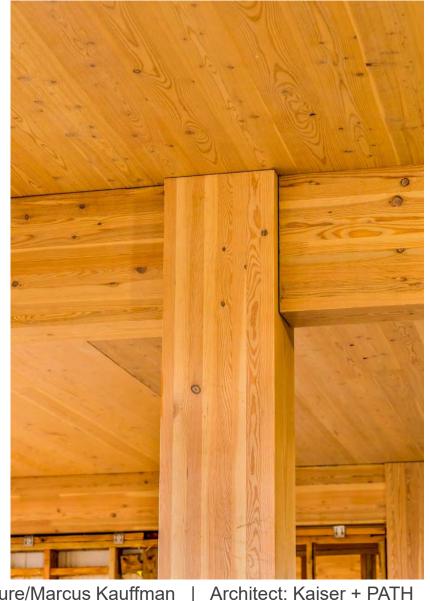


STC	What can be heard
25	Normal speech can be understood quite easily and distinctly through wall
30	Loud speech can be understood fairly well, normal speech heard but not understood
35	Loud speech audible but not intelligible
40	Onset of "privacy"
42	Loud speech audible as a murmur
45	Loud speech not audible; 90% of statistical population not annoyed
50	Very loud sounds such as musical instruments or a stereo can be faintly heard; 99% of population not annoyed.
60+	Superior soundproofing; most sounds inaudible

#### MT: Structure Often is Finish







Photos: Baumberger Studio/PATH Architecture/Marcus Kauffman

But by Itself, Not Adequate for Acoustics



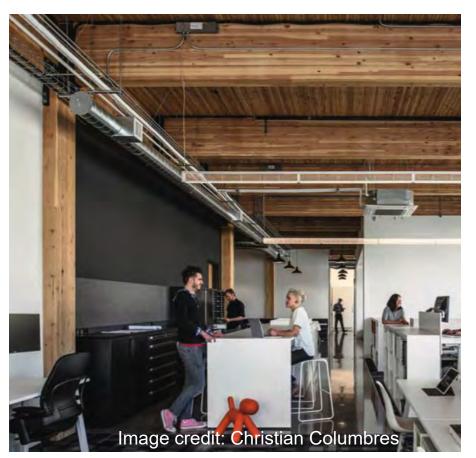


TABLE 1: Examples of Acoustically-Tested Mass Timber Panels

Mass Timber Panel	Thickness	STC Rating	IIC Rating
3-ply CLT wall⁴	3.07"	33	N/A
5-ply CLT wall⁴	6.875"	38	N/A
5-ply CLT floor <sup>5</sup>	5.1875"	39	22
5-ply CLT floor <sup>4</sup>	6.875"	41	25
7-ply CLT floor⁴	9.65"	44	30
2x4 NLT wall <sup>6</sup>	3-1/2" bare NLT 4-1/4" with 3/4" plywood	24 bare NLT 29 with 3/4" plywood	N/A
2x6 NLT wall <sup>6</sup>	5-1/2" bare NLT 6-1/4" with 3/4" plywood	22 bare NLT 31 with 3/4" plywood	N/A
x6 NLT floor + 1/2" plywood <sup>2</sup>	6" with 1/2" plywood	34	33

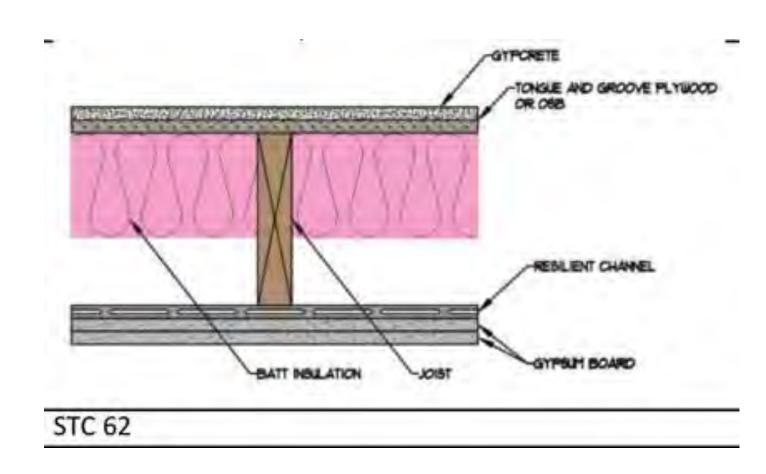
Regardless of the structural materials used in a wall or floor ceiling assembly, there are 3 effective methods of improving acoustical performance:

- 1. Add mass
- 2. Add noise barriers
- 3. Add decouplers

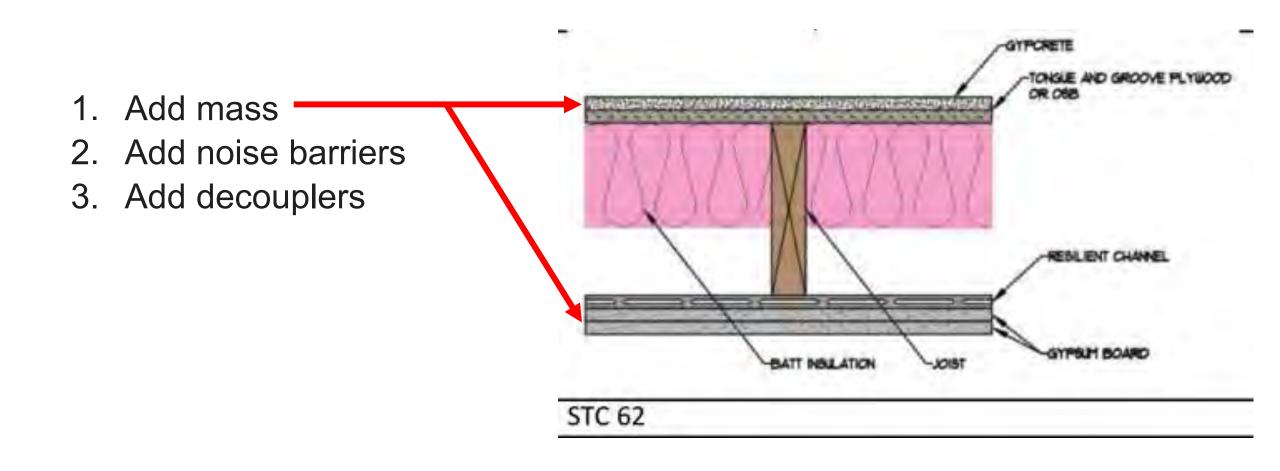


#### What does this look like in typical wood-frame construction:

- 1. Add mass
- 2. Add noise barriers
- 3. Add decouplers

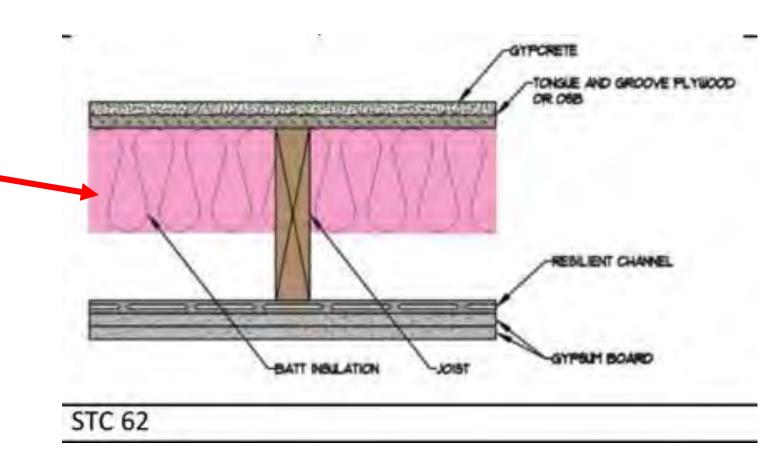


What does this look like in typical wood-frame construction:

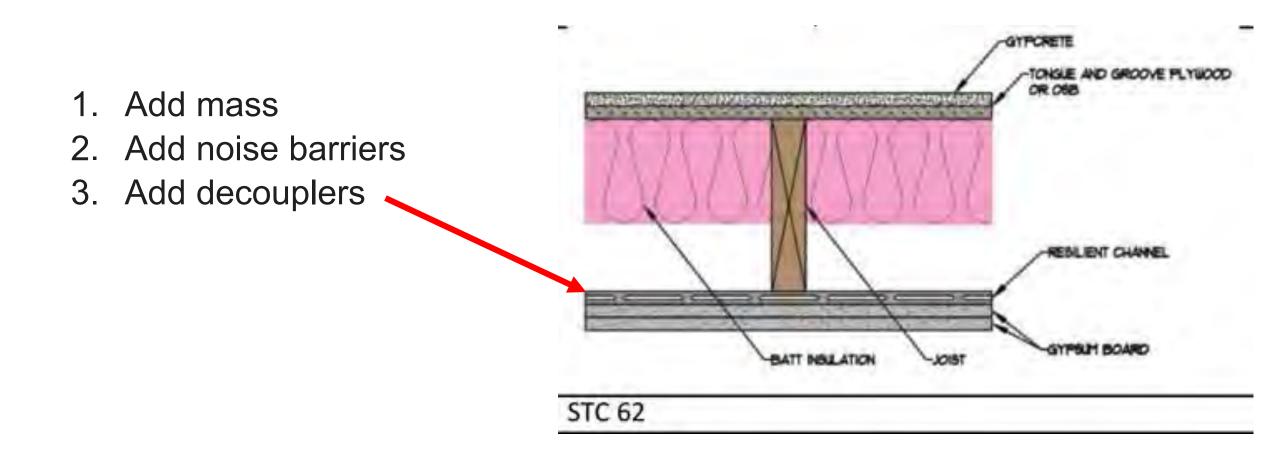


## What does this look like in typical wood-frame construction:

- 1. Add mass
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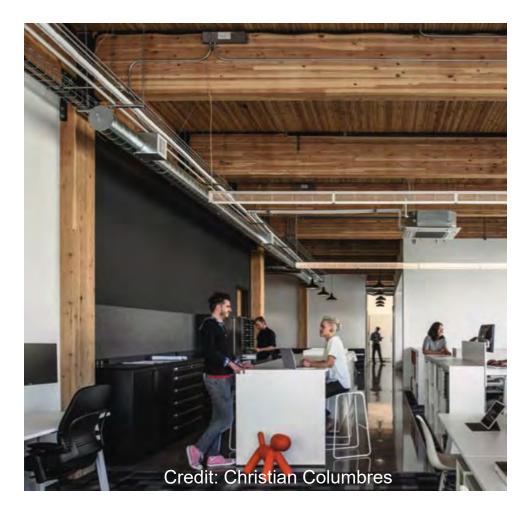


What does this look like in typical wood-frame construction:



# Mass timber has relatively low "mass" Recall the three ways to increase acoustical performance:

- 1. Add mass
- 2. Add noise barriers
- 3. Add decouplers



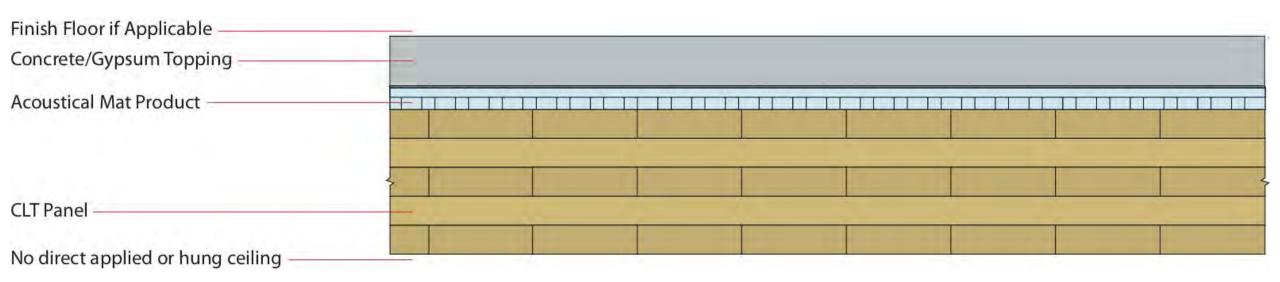






There are three main ways to improve an assembly's acoustical performance:

- 1. Add mass
  - 2. Add noise barriers
- Add decouplers



There are three main ways to improve an assembly's acoustical performance:

- 1. Add mass
- 2. Add noise barriers
- 3. Add decouplers

## **Acoustical Mat:**

- Typically roll out or board products
- Thicknesses vary: Usually ¼" to 1"+







Photo: Maxxon Corporation



Photo: Kinetics Noise Control, Inc.,11



# Common mass timber floor assembly:

- Finish floor (if applicable)
- Underlayment (if finish floor)
- 1.5" to 4" thick concrete/gypcrete topping
- Acoustical mat
- WSP (if applicable)
- Mass timber floor panels



## **Solutions Paper**



## Acoustics and Mass Timber: Room-to-Room Noise Control

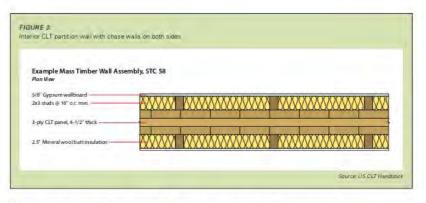
Richard McLain, PE, SE . Senior Technical Director . WoodWorks



The growing availability and code acceptance of mass timber—i.e., large solid wood panel products such as cross-laminated timber (CLT) and nail-laminated timber (NLT)— for floor, wall and roof construction has given designers a low-carbon alternative to steel, concrete, and masonry for many applications. However, the use of mass timber in multi-family and commercial buildings presents unique acoustic challenges.

While laboratory measurements of the impact and airborne sound isolation of traditional building assemblies such as light wood-frame, steel and concrete are widely available, fewer resources exist that quantify the acoustic performance of mass timber assemblies. Additionally, one of the most desired aspects of mass timber construction is the ability to leave a building's structure exposed as finish, which creates the need for asymmetric assemblies. With careful design and detailing, mass timber buildings can meet the acoustic performance expectations of most building types.

http://www.woodworks.org/wp-content/uploads/wood\_solution\_paper-MASS-TIMBER-ACOUSTICS.pdf



### Mass Timber Assembly Options: Walls

Mass timber panels can also be used for interior and extenor walls-both bearing and non-bearing. For interior walls, the need to conceal services such as electrical and plumbing is an added consideration. Common approaches include building a chase wall in front of the mass timber wall or installing gypsum wallboard on resilient channels that are attached to the mass timber wall. As with bare mass timber floor panels, bare mass timber walls don't typically provide adequate noise control, and chase walls also function as acoustical improvements. For example, a 3-ply CLT wall panel with a thickness of 3.07" has an STC rating of 33.1 in contrast, Figure 3 shows an interior CLT partition wall with chase walls on both sides. This assembly achieves an STC rating of 58, exceeding the IBC's acoustical requirements for multi-family construction. Other examples are included in the inventory of tested assemblies noted above.

#### Acoustical Differences between Mass Timber Panel Options

The majority of acoustically-tested mass timber assemblies include CLT. However, tests have also been done on other mass timber panel options such as NLT and dowel-laminated timber (DLT), as well as traditional heavy timber options such as tongue and groove decking. Most tests have concluded that CLT acoustical performance is slightly better than that of other mass timber options, largely because the cross-orientation of laminations in a CLT panel limits sound flanking.

For those interested in comparing similar assemblies and mass timber panel types and thicknesses, the inventory noted above contains tested assemblies using CLT, NLT, glued-laminated timber panels (GLT), and tongue and groove decking.

#### Improving Performance by Minimizing Flanking

Even when the assemblies in a building are carefully designed and installed for high acoustical performance, consideration of flanking paths—in areas such as assembly intersections, beam-to-column/wall connections, and MEP penetrations—is necessary for a building to meet overall acoustical performance objectives.

One way to minimize flanking paths at these connections and interfaces is to use resilient connection isolation and sealant strips. These products are capable of resisting structural loads in compression between structural members and connections while providing isolation and breaking hard, direct connections between members. In the context of

the three methods for improving acoustical performance noted above, these strips act as decouplers. With artight connections, interfaces and penetrations, there is a much greater chance that the acoustic performance of a mass timber building will meet expectations.



Accustical isolation strips

Printer Romanias



## **Inventory of Tested Assemblies**



### **Acoustically-Tested Mass Timber Assemblies**

Following is a list of mass timber assemblies that have been acoustically tested as of January 23, 2019. Sources are noted at the end of this document. For free technical assistance on any questions related to the acoustical design of mass timber assemblies, or free technical assistance related to any aspect of the design, engineering or construction of a commercial or multi-family wood building in the U.S., email help@woodworks.org or contact the WoodWorks Regional Director nearest you: http://www.woodworks.org/project-assistance

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# Questions? Ask us anything.

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