

Mass Timber Shafts and Shaft-Wall Solutions for Mass Timber Buildings

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Photo: Alex Schreyer

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Questions related to specific materials, methods, and services will be addressed at the conclusion of this presentation.



Course Description

The rapid growth of mass timber construction in the U.S. has led to a variety of solutions for shaft wall framing. Mass timber buildings can have mass timber shaft walls, light-frame wood shaft walls, or shaft walls constructed of a different building material altogether. The prefabricated nature of mass timber shaft walls also makes them a good solution in light-frame projects. This presentation will cover material and detailing options for shaft walls within mass timber buildings as well as mass timber shaft walls in other building types. Specific emphasis will be on meeting structural demands and fire-resistance rating requirements for various construction types and building heights as demonstrated through case studies of real projects.

Learning Objectives

- 1. Review code provisions that define fire-resistance ratings for Types III, IV, and V construction.
- 2. Introduce shaft wall material and assembly options.
- **3**. Provide detailing options that establish fire-resistance continuity at framing intersections while meeting structural demands.
- 4. Discuss the benefits and challenges of mass timber shaft walls and their applicability to different building types.

Richard McLain, PE, SE Senior Technical Director – Tall Wood WoodWorks – Wood Products Council

Shaft Wall Resource

Code provisions, detailing options, project examples and more for lightframe wood and mass timber shaft walls

Free resource at woodworks.org

Shaft Wall Solutions for Light-Frame and Mass Timber Buildings

VOODWORKS

An overview of design considerations, detailing options and code requirements

It is fairly common for mid-rise wood buildings to include shaft walls made from other materials. However, wood shaft walls are a code-compliant option for both lightframe and mass timber projects—and they typically have the added benefits of lower cost and faster installation.

A shaft is defined in Section 202 of the 2018 International Building Code¹ (IBC) as "an enclosed space extending through one or more stories of a building, connecting vertical openings in successive floors, or floors and root." Therefore, shaft enclosure requirements apply to stairs, elevators, and mechanical-engineering-plumbing (MEP) chases in multi-story buildings. While these applications might be similar in their fire design requirements, they often have different construction constraints and scenarios where assemblies and detailing may also differ.

This paper provides an overview of design considerations, requirements, and options for light wood-frame and mass timber shaft walls under the 2018 and 2021 IBC, and considerations related to non-wood shaft walls in wood buildings.

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Fire Resistance

Fire Barrier Construction

Shaft enclosures are specifically addressed in IBC Section 713. However, because shaft enclosure walls need to be constructed as fire barriers per Section 713.2, many shaft wall requirements directly reference provisions of fire barriers found in Section 707.

Provisions addressing materials permitted in shaft wall construction are given in both the shaft enclosures section (713.3) and fire barriers section (707.2). These



Common Shaft Walls

Wood Studs



Concrete Walls



Shaftliner Panels Or CFS studs



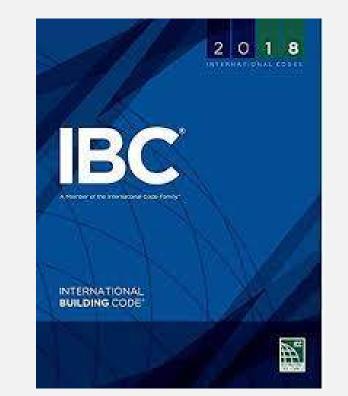


Shaft Walls

Shaft Walls Form Shaft Enclosures

"The purpose of shafts is to confine a fire to the floor of origin and to prevent the fire or the products of the fire (smoke, heat and hot gases) from spreading to other levels."

Source: IBC Commentary to Section 713.1



Types of Shaft Walls

Types of Shafts:

- » Elevator
- » Stair
- » Mechanical



- » Code requirements apply to any/all shaft enclosures.
- » Some points of shaft wall construction and detailing apply to all types of shafts.
- » Some are unique to each type of shaft.

Defining Shaft Wall Requirements

Code requirements for shaft enclosures contained in IBC Section 713:

SECTION 713 SHAFT ENCLOSURES

713.1 General. The provisions of this section shall apply to shafts required to protect openings and penetrations through floor/ceiling and roof/ceiling assemblies. *Interior exit stairways* and *ramps* shall be enclosed in accordance with Section 1023.

- » IBC 713.2: Shaft Walls shall be constructed as Fire Barriers
- » Many shaft wall provisions contained in *IBC Section 707: Fire Barriers*

Interior Fire-Rated Walls: Differences

Fire walls

- Building Separation
- Openings are protected and limited
- Continuous from foundation to/through roof and exterior wall to/through exterior wall
- Structural stability

Fire Barrier

- Shafts; Occupancy Separation
- Openings are protected and limited
- Continuous from floor to floor through concealed spaces at each level

Fire Partition

- Dwelling Unit Separation; Corridors
- Openings are protected
- May terminate at a fire rated floor/ceiling/roof assembly

Shaft Wall Hourly Rating

Section 713: Shaft Enclosures

713.4: Fire-Resistance Rating

- » **<u>2 hours</u>** when connecting 4 stories or more
- » **<u>1 hour</u>** when connecting less than 4 stories
- » Number of connected stories includes basement but not mezzanine
- » Fire rating of shaft walls shall not be less than floor assembly penetrated, but need not exceed 2 hours

Shaft Wall Materials

Type III Construction:

707.2 Materials.

Fire barriers shall be of <u>materials permitted</u> by the building type of construction.

- » Any material permitted by code for all interior elements
- » Fire-retardant treated wood for exterior walls

Type IV-HT Construction:

- » Heavy/mass timber members for interior elements
- > 1-hr min rating for all interior walls/part lons
- » Fire retardant treated wood or CLT for exterior walls

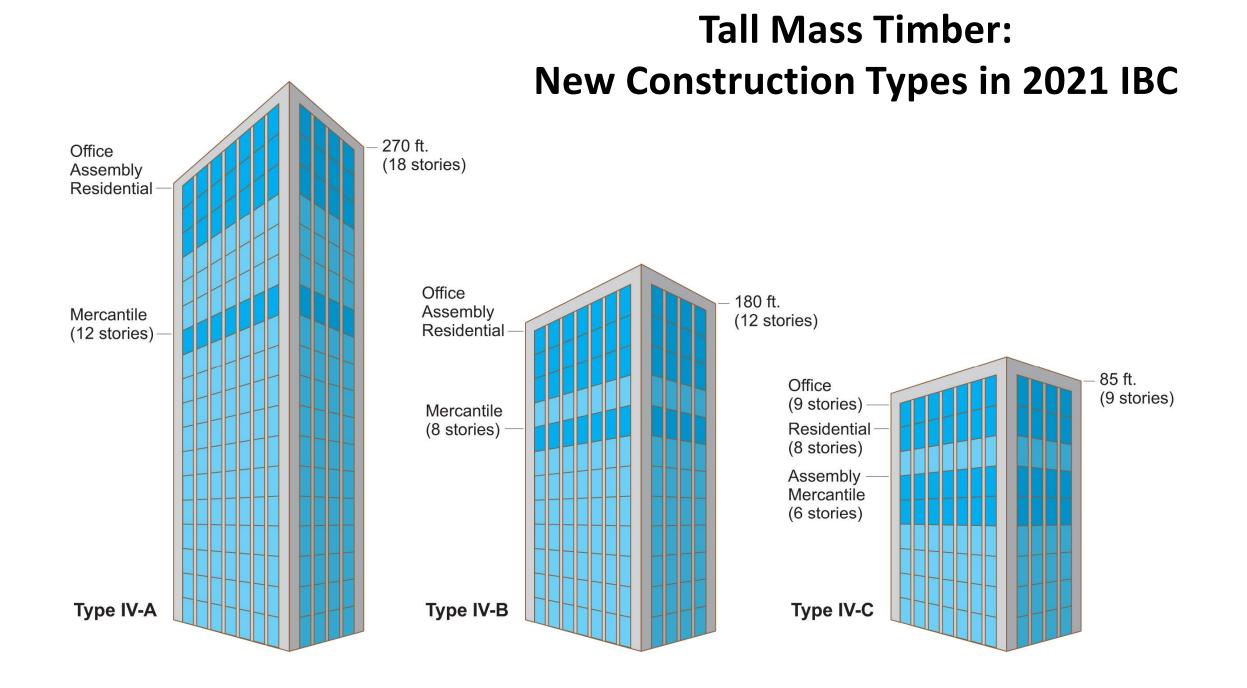
Type V Construction:

» Any material permitted by code for all interior and exte

Shaft Wall Materials

	Type III	Type IV-HT	Type V		
Interior Shaft Walls	Any code- permitted wood framing	Heavy timber or any code-permitted, 1-hr wood framing	Any code- permitted wood framing Any code- permitted wood framing		
Exterior Shaft Walls	Fire-retardant treated wood	Fire-retardant treated wood or CLT			

There is no restriction on the use of combustible materials (light-frame wood and mass timber) in shaft walls or fire barriers in mass timber buildings of Types III, IV-HT or V construction



NC = *non-combustible*

MT = mass timber

Shaft Enclosures in Tall Timber



Shaft material requirements and timber exposure limitations

IV-A	IV-B	IV-C
Up to 12 Stories or 180 ft: MT protected with 2 layers 5/8" type X gyp (if 2 HR req'd) or 3 layers 5/8" type X gyp (if 3 HR req'd) both sides	NC or MT protected with 2 layers 5/8" type X gyp (IBC 2021 602.4.2.6) both sides	NC or MT protected with 1 layer 5/8" type X gyp (IBC 602.4.3.6) both sides
Above 12 Stories or 180 ft: Noncombustible shafts (IBC 2021 602.4)		

Fire-resistance ratings req'd

2 HR (not less than FRR of floor assembly penetrated, IBC 713.4)

Shaft Enclosure Design in Tall Timber



The 2021 International Building Code (IBC) introduced three new construction types—Type IV-A, IV-B and IV-C— which allow tall mass timber buildings. For details on the new types and their requirements, see the WoodWorks paper, *Tall Wood Buildings in the 2021 IBC – Up to 18 Stories of Mass Timber.*¹ This paper builds on that document with an in-depth look at the requirements for shaft walls, including when and where wood can be used.

Shaft Enclosure Requirements in the 2021 IBC

A shaft is defined in Section 202 of the 2021 IBC as "an enclosed space extending through one or more stories of a building, connecting vertical openings in successive floors, or floors and roof." Therefore, shaft enclosure requirements apply to stairs, elevators, and mechanical/ electrical/plumbing (MEP) chases in multi-story buildings. While these applications may be similar in their fire design requirements, they tend to differ in terms of their assemblies, detailing, and construction constraints.

Shaft enclosures are specifically addressed in IBC Section 713. However, because shaft enclosure walls must be constructed as fire barriers per Section 713.2, many shaft wall requirements reference provisions for fire barriers found in Section 707.

Allowable Shaft Wall Materials

Provisions addressing materials permitted in shaft wall construction can be found in both the shaft enclosures section (713.3) and fire barriers section (707.2) of the code



Generate Architecture and Technologies + MIT – John Klein

A relatively new category of wood products, mass timber can encompass well known and widely used products such as glue-laminated timber (glulam) and pail-laminated

Continuity Provisions

Section 713: Shaft Enclosures

713.5 Continuity.

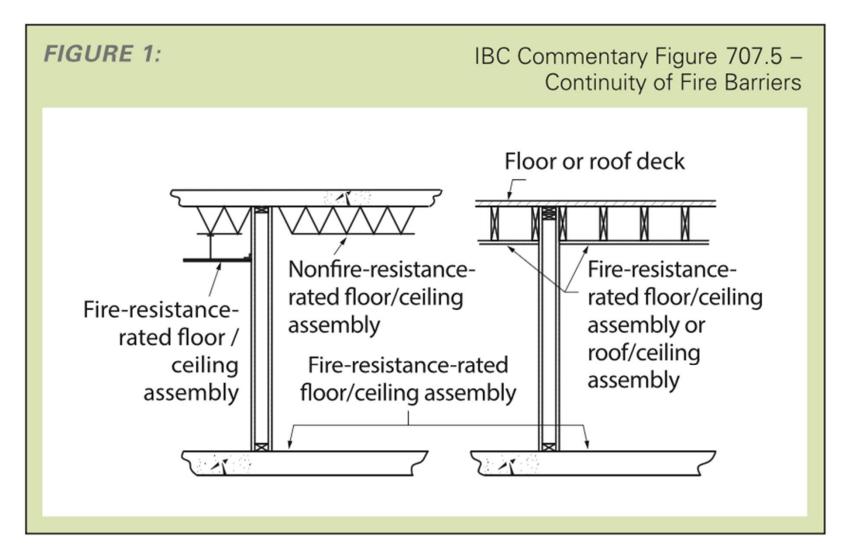
Shaft enclosures shall have continuity in accordance with 707.5 for fire barriers.

Section 707: Fire Barriers

707.5 Continuity.

Fire barriers <u>shall extend from the top of the foundation or floor/ceiling assembly below</u> <u>to the underside of the floor or roof sheathing, slab or deck above</u> and shall be securely attached thereto. Such fire barriers shall be <u>continuous though concealed space</u> such as the space above a suspended ceiling. Joints and voids at intersections shall comply with Sections 707.8 and 707.9.

Continuity Provisions



Continuity Provisions

What do these continuity provisions look like?

In mass timber construction, the mass timber floor panel is the "slab"

The mass timber floor panel (slab) does not interrupt the wall's continuity per the continuity definition of a fire barrier.

Platform mass timber floor to shaft wall permitted by code



Fire barriers, including shaft walls, must extend from top of slab to underside of slab (mass timber panel = slab). Slab does not obstruct continuity.

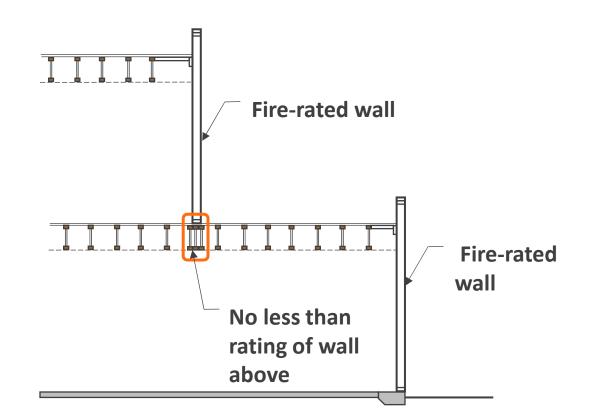
Supporting Construction Provisions

Section 707: Fire Barriers

707.5.1 Supporting Construction:

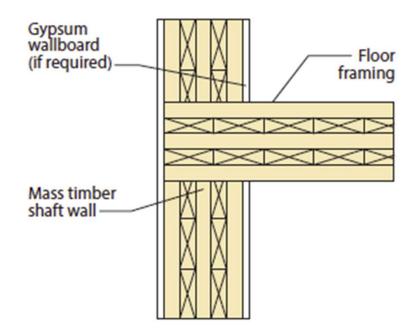
The supporting construction for a fire barrier shall be protected to afford the required fire-resistance rating of the fire barrier supported.

Ex., shaft walls that are not continuous to lowest level



Supporting Construction Provisions

Supporting construction differences (any?)





The intent of a fire barrier is to provide fire confinement. If a fire barrier wall is supported directly by a wall below, the intersecting floor panel should not be considered a supporting element.

Joints in Shaft Walls

Section 202: Definitions

Joint. The opening in or between adjacent assemblies that is created due to building tolerances, or is designed to allow independent movement of the building in any plane caused by thermal, seismic, wind or any other loading.

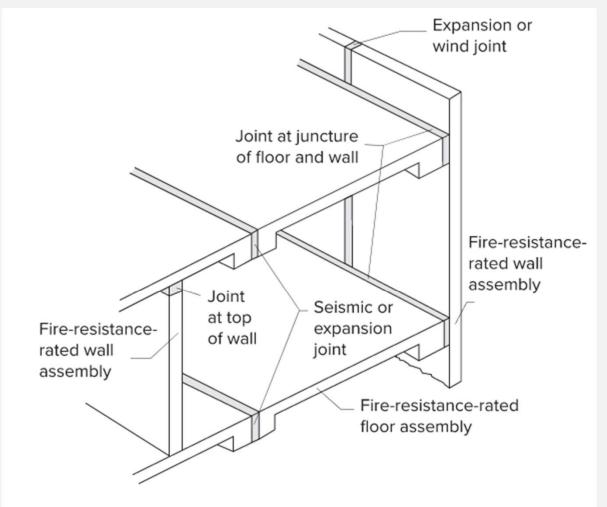


FIGURE 2: IBC Commentary Figure 715.1 – Examples of joint locations

Penetrations in Shaft Walls

Section 713: Shaft Enclosures

713.8 Penetrations.

Penetrations in shaft enclosure shall be protected in accordance with Section 714 as required for fire barriers. Structural elements such as beams or joists, where protected in accordance with Section 714 shall be permitted to penetrate a shaft enclosure.

Section 707: Fire Barriers

707.7 Penetrations.

Penetrations of fire barriers shall comply with Section 714.

Penetrations in Shaft Walls

Where are structural penetrations in shaft walls common?

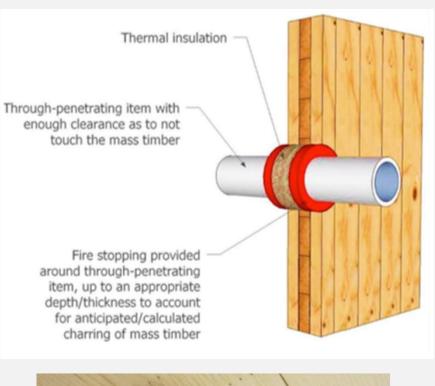
- » Mass Timber Beams to Shaft Wall Connection
- » Stair framing to Shaft Wall Connection



Penetrations in Shaft Walls

To some, a new way of thinking: Many are familiar with firestopping for MEP, but not structure, especially mass timber structure

- Some firestopping systems available as tested configurations for wood conditions
- Most manufacturers can provide engineering judgement details, certification statements for this condition

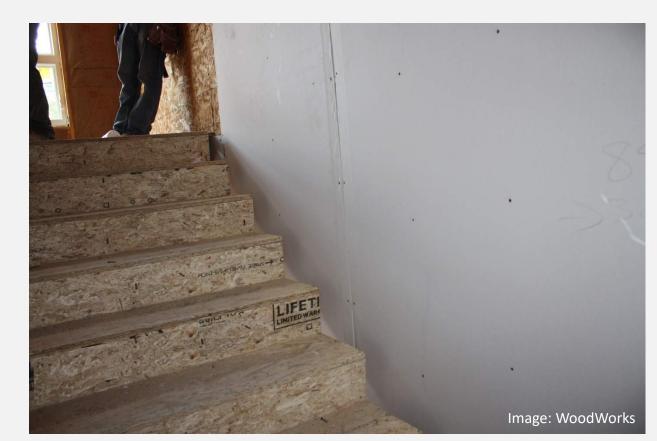




Shaft Wall Assemblies

Assembly selection considerations:

- » Fire resistance rating requirement (1-hr or 2-hr)
- » Size and height of shaft
- » Structural needs (gravity & lateral loads)
- » Acoustics
- » Space available for wall (allowed thickness)



Shaft Wall Assemblies

Options for Mass Timber Shaft Walls in Mass Timber Buildings

Exposed Mass Timber Shaft Walls



Mass Timber Shaft Walls w GWB

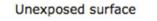


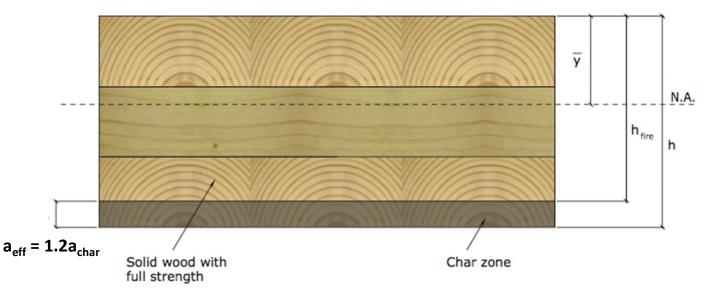
MT Fire Resistance Ratings (FRR)

How do you determine fire-resistance rating of exposed mass timber shaft walls?

- 1. Calculations in Accordance with IBC 722 -> NDS Chapter 16
- 2. Tests in Accordance with ASTM E119







Fire exposed surface

MT Fire Resistance Ratings (FRR)

Design Example: 2-hr CLT Wall

TECHNICAL REPORT NO. 10

Example 6: Exposed CLT Wall - Allowable Stress Design

Cross-laminated timber (CLT) wall with an unbraced height of L=120 inches and loaded in compression in the strong-axis direction. The design loads are w_{ivve} =14,000 plf and w_{dead} =6,150 plf including estimated self-weight of the CLT panel. Walls above are supported on a CLT floor slab and aligned with a CLT wall below. Sealing of wall joints with fire-rated caulk restricts hot gases from venting through half-lap joints at edges of CLT panel sections. Calculate the required section dimensions for a 2-hr structural fire resistance time when subjected to an ASTM E119 fire exposure.

Calculate column load:

P_{load} = P_{dead} + P_{snow} = 6,150 plf + 14,000 plf = 20,150 lb/foot of width.

From PRG 320, select a 7-ply CLT panel made from 1-3/8 in x 3-1/2 in. lumber boards (CLT thickness of 9-5/8 inches). For CLT grade E1, tabulated properties are:

Reference compression stress, $F_{c,0} = 1800$ psi	(PRG 320 Annex A, Table A1)
Reference bending moment, FbSeff,0 = 18,375 ft-lb/ft of width	(PRG 320 Annex A, Table A2)
Reference bending stiffness, Eleff,0 = 1,089x10 ⁶ lb-in ² /ft of width	(PRG 320 Annex A, Table A2)
Reference shear stiffness, GA _{eff,0} = 1.4x10 ⁶ lb/ft of width	(PRG 320 Annex A, Table A2)

Calculate the effective wall compression capacity:

A_{parallel} = bd of strong axis plies = 4(12)(1.375) = 66 in ² /ft of width	(NDS 10.3.1)
$P_c = F_{c,0}(A_{\text{parallel}}) = (1800)(66) = 118,800 \text{ lb/ft of width}$	(NDS 10.3.1)

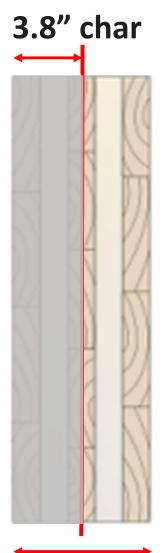
Calculate the apparent wall buckling capacity:

Using NDS Equation 10.4-1, the value for (EI)_{app} can be calculated. Since PRG-320 assumes that E/G = 16 for CLT, NDS Equation 10.4-1 can be rewritten as:

$$(EI)_{app} == \frac{EI_{eff}}{1 + \frac{K_s EI_{eff}}{GA_{eff}L^2}}$$

For pinned-pinned column buckling, Ks=11.8; therefore:

 $(EI)_{app} = \frac{1,089 \times 10^6}{1 + \frac{(11.8)(1,089 \times 10^6)}{(1.4 \times 10^6)(120)^2}} = 665 \times 10^6 \ lb/in^2/ft \ of \ width$





Credit: AWC TR10

MT Fire Resistance Ratings (FRR)

Inventory of Fire Tested MT Assemblies

Table 2: North American Fire Resistance Tests of Mass Timber Wall Assemblies



Mass Timber Panel	Manufacturer	CLT Grade or Timber Grade	Exposed Side Protection	Panel Connection	Unexposed Side Protection	Load Rating	Fire Resistance Achieved (Hours)	Actual Fire Endurance	Source	Testing Lab
3-ply (114mm 4.488 in)	Nordic	SPF 1650 Fb 1.5E MSR x SPF #3	2 layers 1/2" Type X gypsum	Half-Lap	None	Reduced 76% Axial Capacity	1.5	106 min	1 (Test 2)	NRC Fire Laboratory
3-ply (3.78" 99mm)	Structurlam	V2	1 layer 5/8" Type X gypsum	Half-Lap	None	Reduced 60% Max Design Load	1	76 min	8	Intertek December 2013
3-ply (3.78" 99mm)	Structurlam	V2	1 layer 5/8" Type X gypsum	Half-Lap	None	Unreduced 100% Max Design Load	1	66 min	9	Intertek November 2014
3-ply (105mm)	Nordic	E1	1 layer 5/8" Type C or Type X gypsum	Half-Lap	1 layer 5/8" Type C or Type X gypsum	Reduced, 30% Allowable Compression Parallel to Grain	1	Not Provided	10	UL (V320)
3-ply (105mm)	Nordic	E1	None	Half-Lap	None	Loaded, See Manufacturer	0.5	32 min	20	Intertek 5/17/2012
3-ply (78mm)	Nordic	E1	5/8" Type X gypsum over 2x3 SPF Studs @ 24" oc with 2 1/5" mineral wool between stude	Half-Lap	None	Loaded, See Manufacturer	1	83 min	22	Intertek 12/30/2011
5-ply (131 mm)	Nordic	E1	2 layers 5/8" Type C or Type X gypsum	Half-Lap	2 layers 5/8" Type C or Type X gypsum	Reduced, 30% Allowable Compression Parallel to Grain	2	Not Provided	10	UL (V320)
5-ply (175mm)	Nordic	E1	1 layer 5/8" Type C or Type X gypsum	Half-Lap	1 layer 5/8" Type C or Type X gypsum	Reduced, 30% Allowable Compression Parallel to Grain	2	Not Provided	10	UL (V320)
5-ply (175mm)	Nordic	E1	None	Half-Lap	None	Reduced, 30% Allowable Compression Parallel to Grain	1	Not Provided	10	UL (V320)
5-ply (175mm)	Nordic	E1	2 layers 5/8" Type X gypsum	Spline	2 layers 5/8" Type X gypsum	Loaded, See Manufacturer	3.5	219 min	5	NRC Fire Laboratory Nov 2014
5 ply (6 7/8")	Smartlam	SL-V4	None	Half-Lap	None	Loaded, See Manufacturer	2	120 min	11	Western Fire Center 5/25/2017
5-ply (175mm 6.875")	Nordic	SPF 1950 Fb MSR x SPF #3	None	Half-Lap	None	Reduced 37% Axial Capacity	1.5	113 min	1 (Test 4)	NRC Fire Laboratory
5 ply (105mm)	Structurlam	SPF #1/#2 x SPF #1/#2	None	Half-Lap	None	Reduced 25% Axial Capacity	<1	57 min	1 (Test 8)	NRC Fire Laboratory
5-ply (175mm 6.875")	DR Johnson	V1	None	Half-Lap	None	Loaded, See Manufacturer	2	120 min	13 (Test 1)	Western Fire Center 9/28/2016
5-ply (175mm 6.875")	SmartLam	SL-V4	None	Half-Lap	None	Loaded, See Manufacturer	1.5	101 min	13 (Test 2)	Western Fire Center 9/30/2016
5-ply (175mm 6.875")	Smartlam	V1	None	Half-Lap	None	Loaded, See Manufacturer	2	120 min	13 (Test 7)	Western Fire Center 1/26/2017
C 1			l .		1	1 2 1 1 1	1 1			MOOT C

Shaft Wall Assemblies

Options for Light-Frame Shaft Walls in Mass Timber Buildings

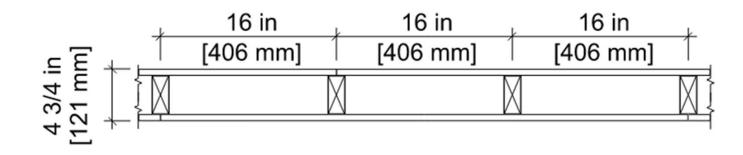


FIGURE 4: UL U305

1-Hour Single Wall

- UL U305
- GA WP 3510
- UL U311
- IBC 2012 Table 721.1(2), Item 14-1.3
- UL U332

1-Hour Double Wall

• UL U341

1-Hour Wall with Shaftliner

- UL V455
- UL V433

Shaft Wall Assemblies

Options for Light-Frame Shaft Walls in Mass Timber Buildings



UL U301

FIGURE 5: UL U334

6 1/2 in [165 mm] 16 in

[406 mm]

- UL U334
- IBC 2012 Table 721.1(2) Item Number 14-1.5
- IBC 2012 Table 721.1(2) Item Number 15-1.16

2-Hour Double Wall

- UL U342
- UL U370
- GA WP 3820

2-Hour Wall with Shaftliner

<u>16 in</u> [406 mm]

16 in

[406 mm]

• UL U336

- UL U373
- UL U375
- UL V455
- UL V433
- GA ASW 1000

Shaftliner Systems - Benefits & Limitations

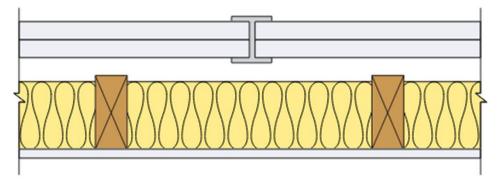
Benefits

Allows installation from one side only

 useful in small MEP shafts where
 finishing from inside isn't possible

Limitations

- » Some have height limitations, both per story and overall system
- » Not structural, requires back-up wood wall



59 STC Sound Transmission

Test Reference: RAL TL 10-290

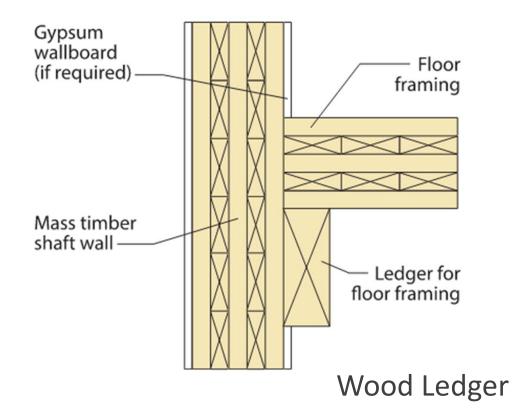
Two layers 1" (25.4 mm) shaftliner inserted in H-studs 24" (610 mm) o.c., min. 3/4" (19 mm) air spacing between liner panels and adjacent or wood metal framing

Sound tested with 2"x4" stud wall with 1/2" (12.7 mm) wallboard or interior panels and 3-1/2" (89 mm) fiberglass insulation in stud space

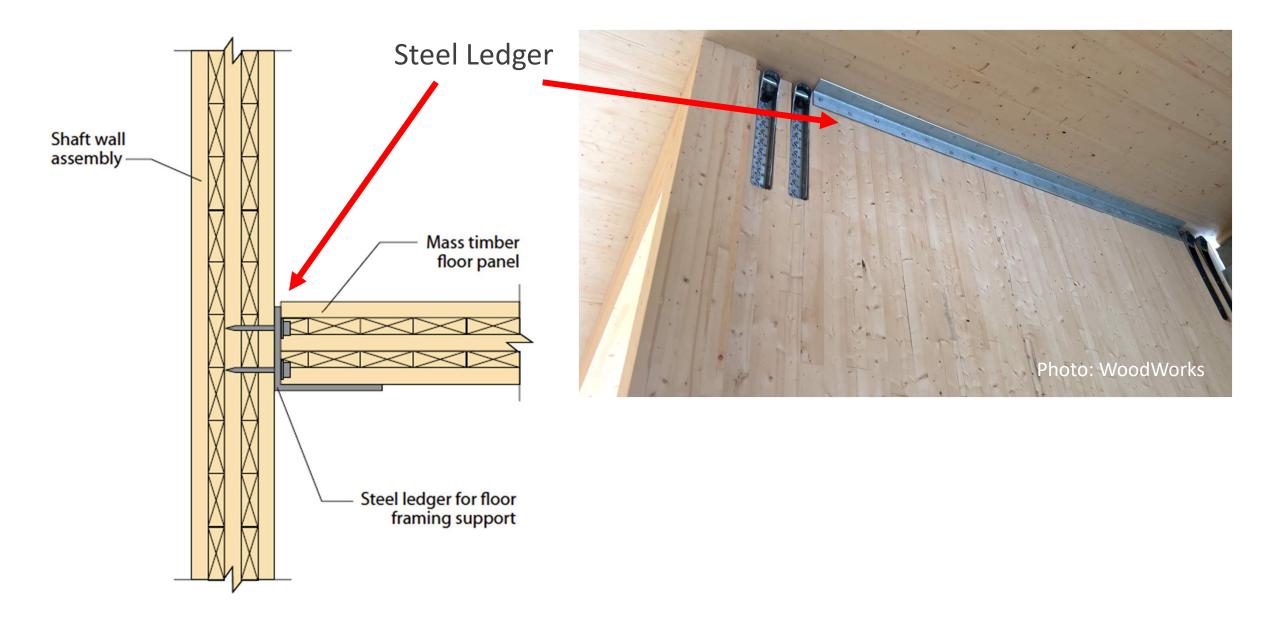
FIGURE 8: UL U373 Credit: Georgia Pacific

Floor to Shaft Wall Detailing

Construction erection and sequencing will inform efficient floor to wall intersection



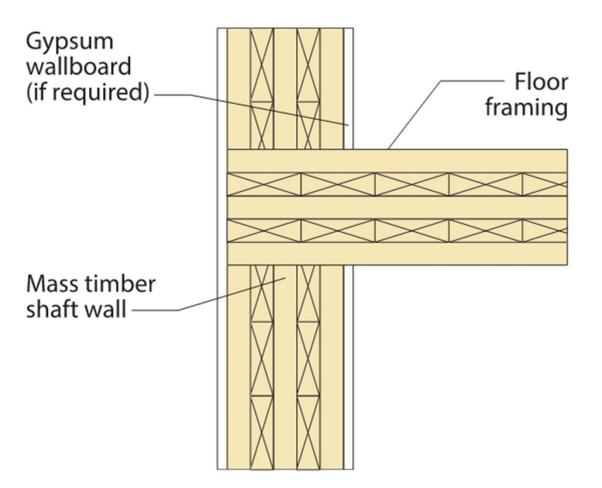


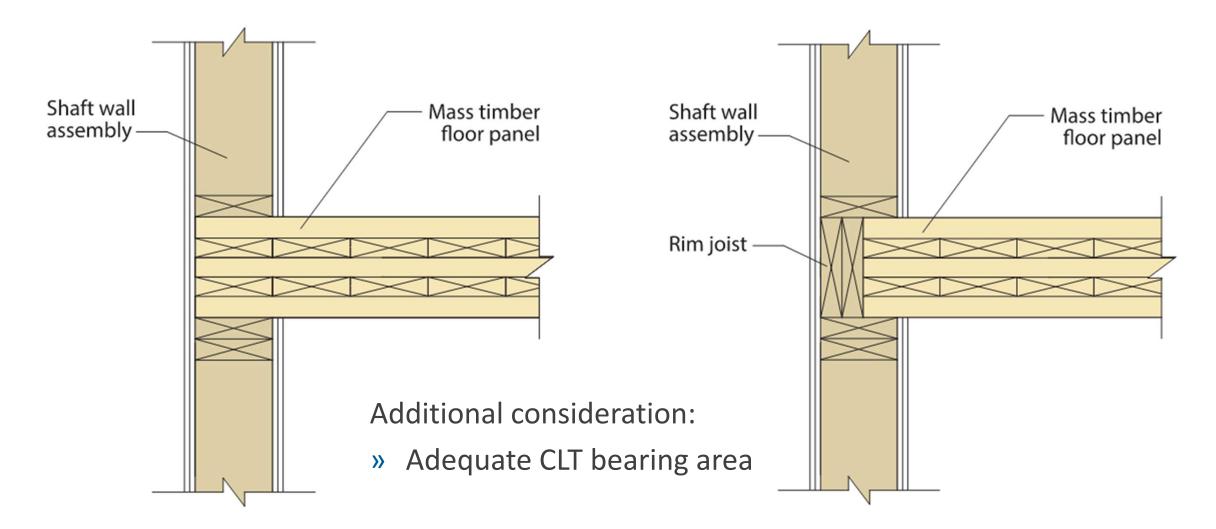


Recall fire barrier continuity definition:

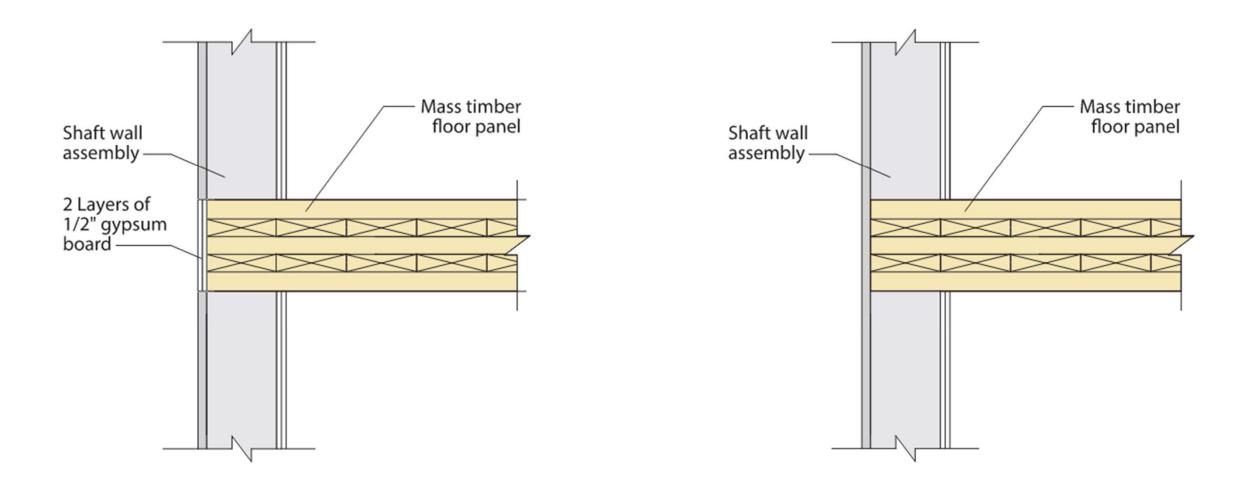
shall extend ... to the <u>underside of</u> the floor or roof sheathing, slab or <u>deck above</u> and shall be securely attached thereto

CLT is the "slab," and it is not disrupting the continuity of the shaft wall. **Platform Construction**

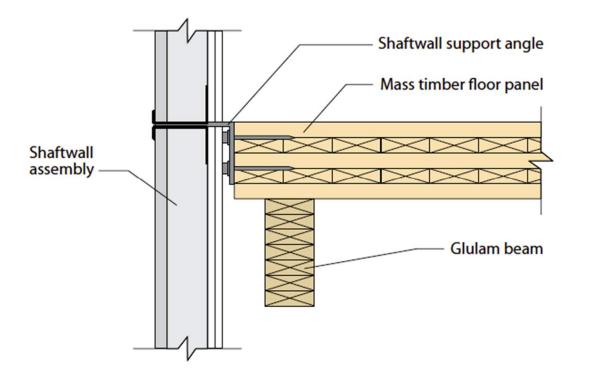


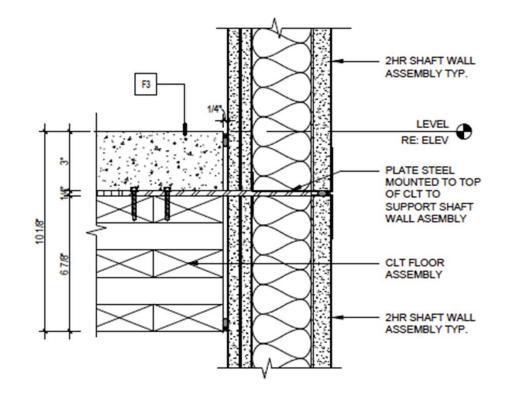


Shaft wall – Support Details



Shaftliner Systems – Support Details





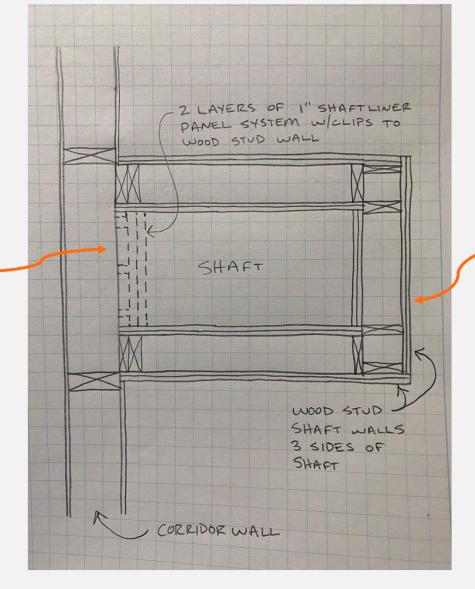
Shaftliner Systems – Configuration Options

Can also utilize wood framed shaft walls or mass timber shaft walls on 1-3 sides and CH studs with shaftliner on remaining side(s)



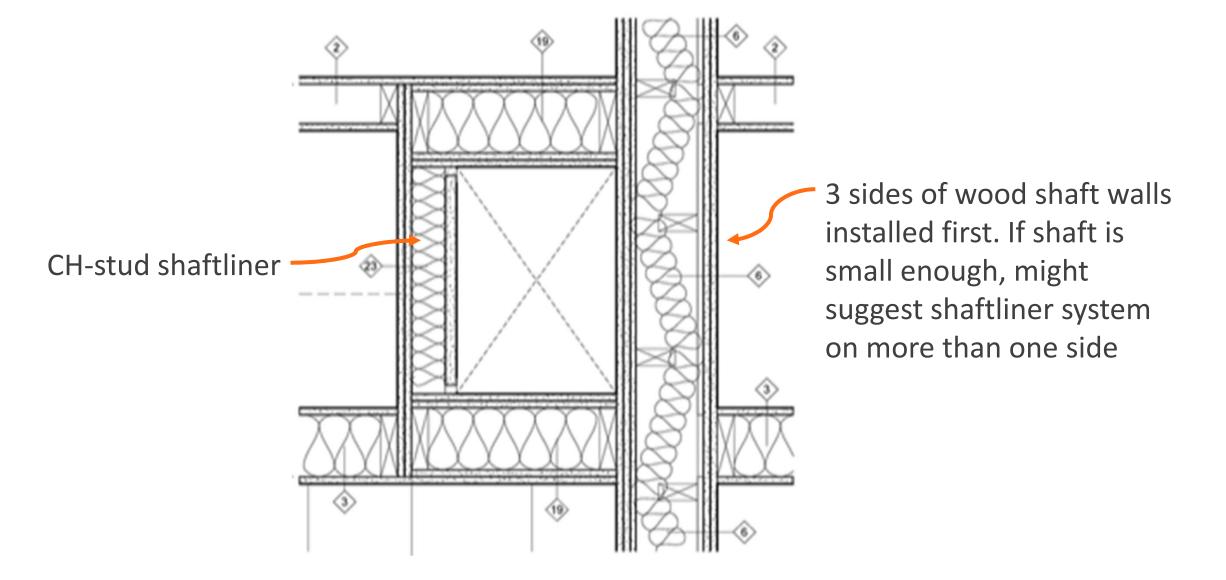
Shaftliner Systems – Configuration Options

H-stud shaftliner with wood stud wall backup



3 sides of wood shaft walls installed first. If shaft is small enough, might suggest shaftliner system on more than one side

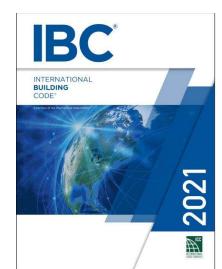
Shaftliner Systems – Configuration Options

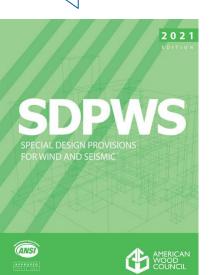


Considerations for Lateral Systems

Prescriptive Code Compliance:

- Concrete Shear Walls
- ✓ Steel Braced Frames
- ☑ Light Frame Wood Shear Walls (65 ft max)
- CLT Shear Walls (65 ft max) < 2021 SDPWS, ASCE 7-22
- CLT Rocking alls Currently in testing!





7-16 Minimum Design Loads and Associated Criteria for Buildings and Other Structures

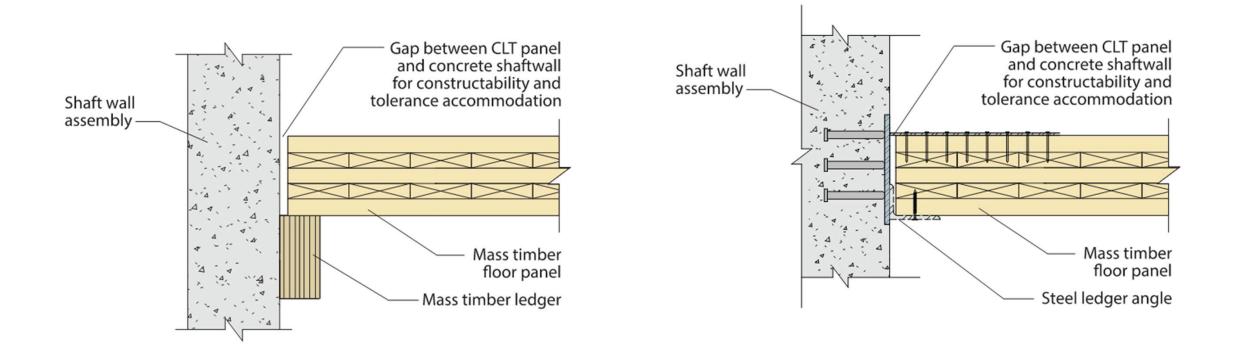






GU0 A prescriptive method for CLT rocking walls has been proposed (in case we get questioned on this) Guest User, 2023-05-04T16:26:34.220

Consider Differential Material Movements & Tolerances in Detailing



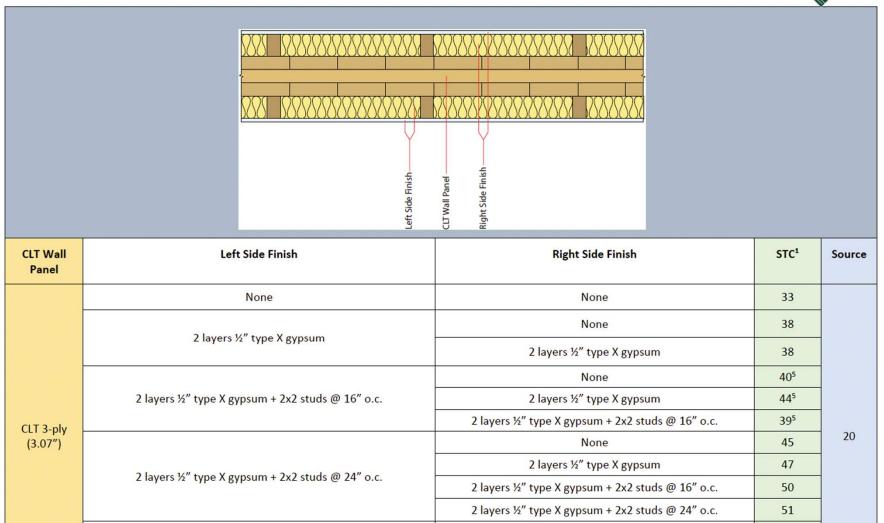
Mass Timber Shaft Wall Acoustics



Acoustics & Sound Control

Inventory of Tested Assemblies

Table 7: Single CLT Wall



Mass Timber Shafts and Shaft Wall Solutions for Mass Timber Buildings

Presented by: Matt Harwood (Holmes)

Content also provided by: Alyson Blair (Holmes) Chris Grosse (LEVER Architecture)



Disclaimer: This presentation was developed by a third party and is not funded by WoodWorks or the Softwood Lumber Board.

Project Information

- Confidential Office Project in the Pacific Northwest
- 6 Stories Mass Timber Type III-A over Type I-A basement structure
- Team
 - Architect: LEVER
 - Structural Engineer: Holmes
 - Fire & Life Safety Code Consulting: Holmes

- IBC Section 713: Shaft Enclosures
- Shafts are constructed as fire barriers

713.2 Construction. Shaft enclosures shall be constructed as *fire barriers* in accordance with Section 707 or horizontal assemblies in accordance with Section 711, or both.

- Shaft fire resistance rating (independent of construction type)
 - 1 hr: < 4 stories
 - 2 hrs: 4 stories or more

*not less than the floor it is penetrating

• 713.5 Shaft continuity requirements per fire barrier requirements

713.5 Continuity. Shaft enclosures shall be constructed as *fire barriers* in accordance with Section 707 or *horizontal assemblies* constructed in accordance with Section 711, or both, and shall have continuity in accordance with Section 707.5 for *fire barriers* or Section 711.2.2 for *horizontal assemblies*, as applicable.

• 707.5 Fire barrier continuity requirements

707.5 Continuity. *Fire barriers* shall extend from the top of the foundation or floor/ceiling assembly below to the underside of the floor or roof sheathing, slab or deck above and shall be securely attached thereto. Such *fire barriers* shall be continuous through concealed space, such as the space above a suspended ceiling. Joints and voids at intersections shall comply with Sections 707.8 and 707.9

• 707.5.1 Supporting Construction: required to be the same fire resistance rating of the fire barrier being supported

707.5.1 Supporting construction. The supporting construction for a *fire barrier* shall be protected to afford the required *fire-resistance rating* of the *fire barrier* supported. Hollow vertical spaces within a *fire barrier* shall be fireblocked in accordance with Section 718.2 at every floor level.

Floor & secondary members FRR requirements per construction type

	Type I		Type II		Type III			Type IV				Type V	
	A	В	А	В	А	В	Α	В	С	HT	А	В	
FRR	2	2	1	0	1	0	2	2	2	HT	1	0	

Shaft FRR (independent of construction type*)

	< 4 stories	4 or more stories
FRR	1 hr	2 hr

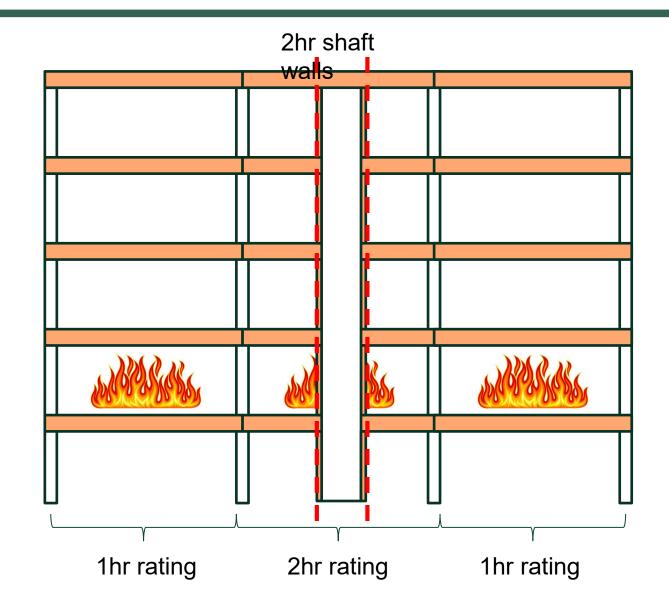
Floor & secondary members FRR requirements per construction type

	Type I		Type II		Тур	Type III		Type IV				Type V	
	A	В	А	В	А	В	Α	В	С	HT	A	В	
FRR	2	2	1	0	1	0	2	2	2	HT	1	0	

Shaft FRR (independent of construction type*)

	< 4 stories	4 or more stories
FRR	1 hr	2 hr

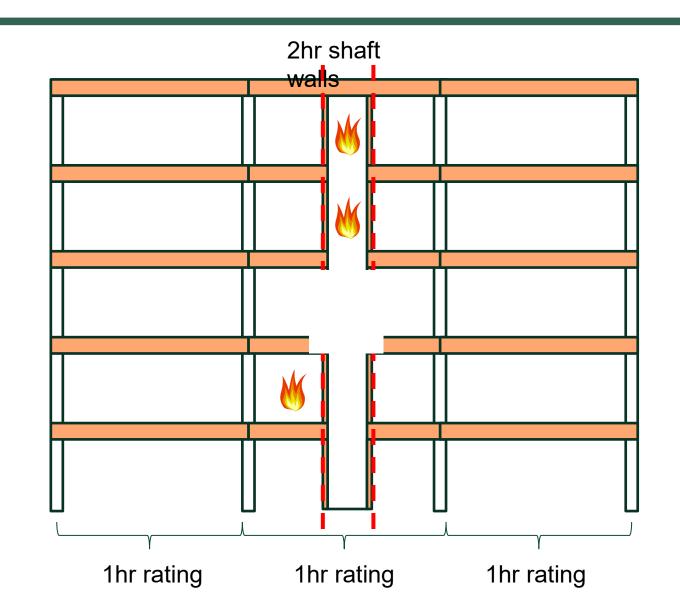
"Why have a shaft supported by a floor with a fire rating higher than the remainder of the building? Won't you just end up with your shaft still standing while the building collapses around it?"



"Why have a shaft supported by a floor with a fire rating higher than the remainder of the building? Won't you just end up with your shaft still standing while the building collapses around it?"

Answer:

- Required by IBC!
- Without this, potential for fire spread through 2hr shaft to occur at 1hr fire exposure



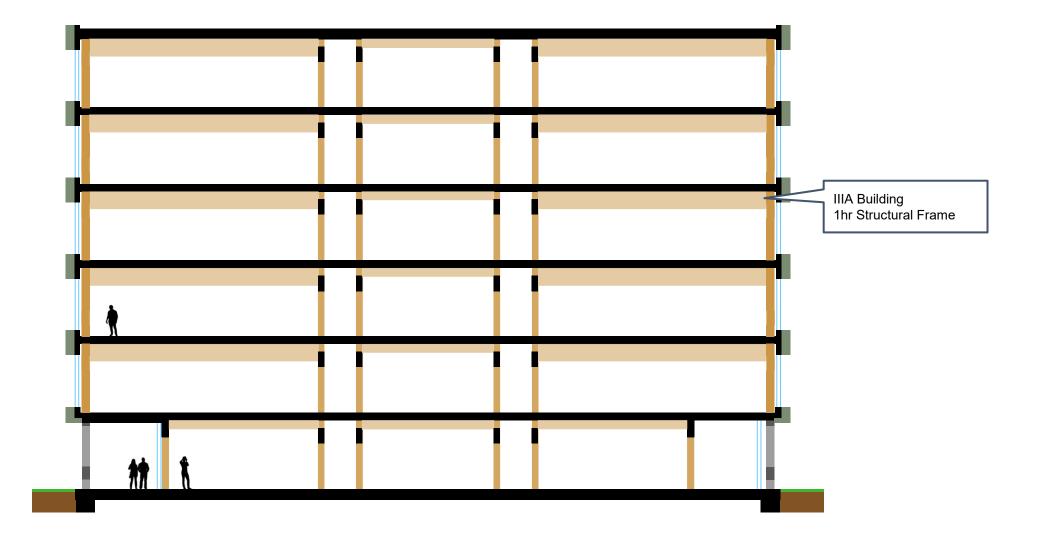
Building Overview

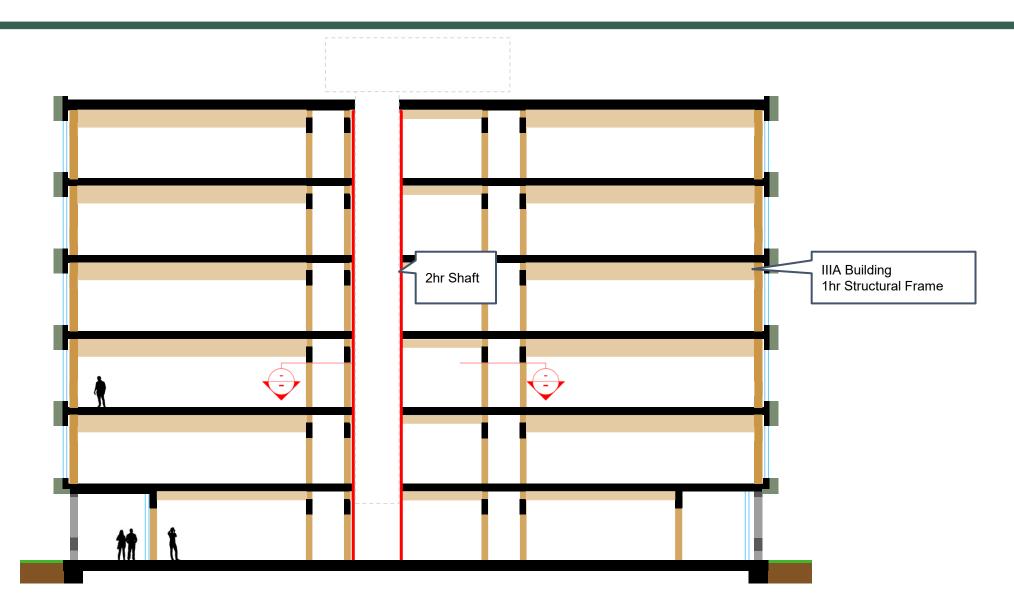
270'x140' 6 storey all mass timber superstructure

Structural System:

- Number of innovative structural solutions outside of design codes
- Long-span composite (CLT/GLT) floor cassettes (requires testing)
- Perimeter lateral & gravity structure (requires testing)
- Internal CLT shear walls with BRB hold-downs (relies on previous testing)

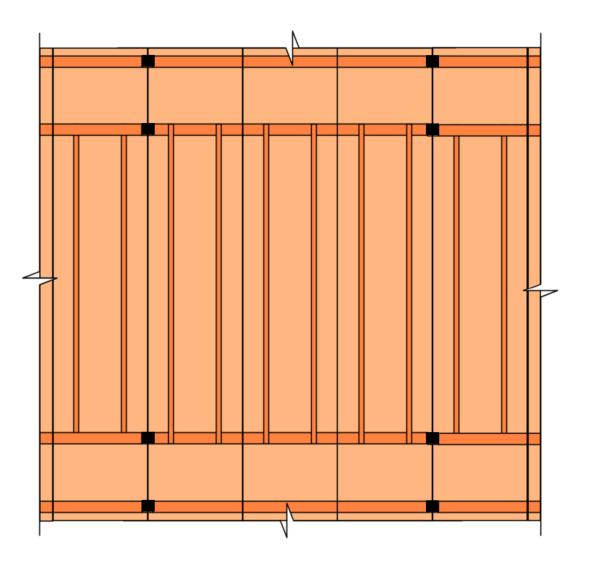






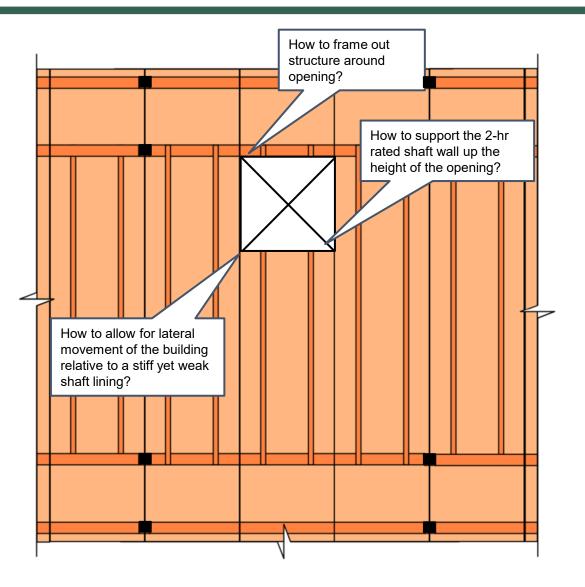
General Structural Gravity System:

- Columns
- Girders spanning east-west
- Mass Timber Double T Cassettes spanning north-south



Particular shaft difficulties

- This project uses an exterior distributed lateral system, therefore no internal shear cores to frame out shaft openings
- All shaft openings must accommodate lateral movement without attracting lateral load
- Intent is to avoid cracking of brittle materials during a serviceability level event
- Exposed cassette soffit makes framing out openings difficult

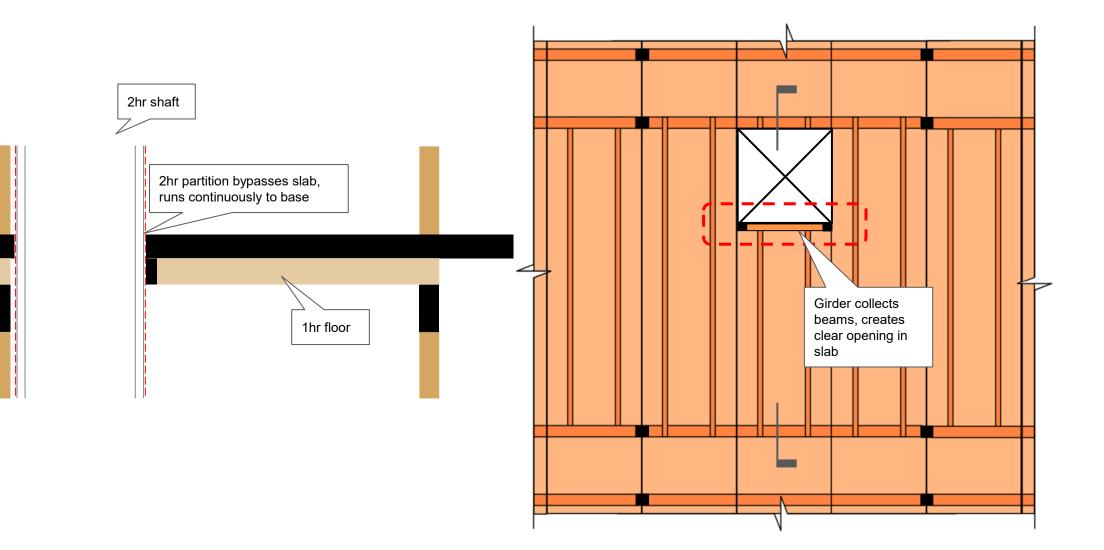


Number	Name		Shaft		Floor			Evampla	Reason to use	Reason to not use	Notes				
Number	Name	Rating	Vertical	Lateral	Rating	Vertical	Lateral	Example	Reason to use	Reason to not use	Notes				
1a	Typ Shear /Grav Core			Self		Shaft -	Tied into shaft	CMU core, platform framed wood construction	Typical construction., simple detailing	Core not strong enough to resist loads attracted. Could detail to yield but may crack at SLE. Poor seismic performance.					
1b	Isolated Shear /Grav Core						Isolated from shaft	CMU core	Stops core attracting seismic load	Tricky detailing to slip floor relative to shaft, negates benefit of CMU core, seismic gap may be large					
1c	Isolated Shear Core		Self	Self		1Hr	Framed around	Share	Shaftlined core w/ steel frame	Simpler detailing than 1b	Still requires seismic gap, shaft likely not strong enough				
1d	Tied-in Shaft	2Hr										shaft		Shaftlined core w/ steel frame	Doesn't require seismic gap, shaft strength not required
1e	Tied-in Grav Core			Floor		Shaft	Tied into shaft	Sim to platform framing but with 'flexible' walls	Typical construction., simple detailing	Vertical load down shaft needs to be resisted at transfer slab level. Also shaft can't sit on floor which is preferred detail	Option 2 - Second Design				
2a	Typ Partition Framing	Floor	Floor	Floor		Framed around		Typ light-framed partition on 1hr floor	Often used on projects, similar to typ partition framing	1hr structure supporting 2hr shaft doesn't comply with IBC					
2b	2hr Partition Framing		FIOOP		shaft 2Hr			Typ light-framed partition on 2hr floor	Per above but complies with IBC	Satisfies all requirements, however requires addn'l bays of 2hr structure in project	Option 3 - Final Design				

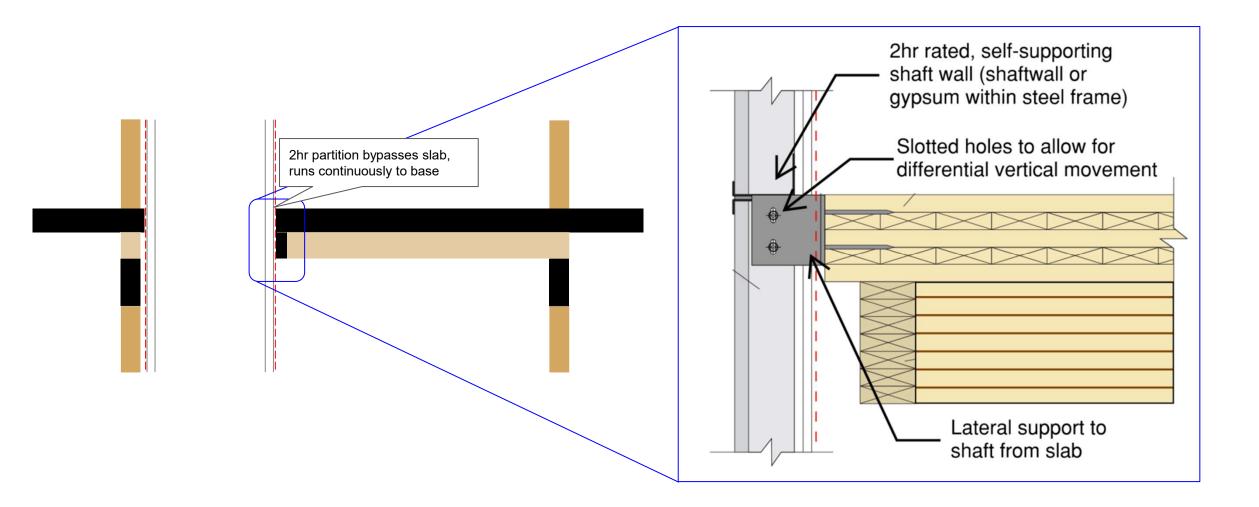


Number	Name		Shaft			Floor		Example	Dessen to use	Bassan to not use	Natas
Number	Name	Rating	Vertical	Lateral	Rating	Vertical	Lateral		Reason to use	Reason to not use	Notes
1a	Typ Shear /Grav Core					Shaft	Tied into shaft	CMU core, platform framed wood construction	Typical construction., simple detailing	Core not strong enough to resist loads attracted. Could detail to yield but may crack at SLE. Poor seismic performance.	
1b	Isolated Shear /Grav Core			Self		Shart	Isolated from shaft	CMU core	Stops core attracting seismic load	Tricky detailing to slip floor relative to shaft, negates benefit of CMU core, seismic gap may be large	
1c	Isolated Shear Core		Self		1Hr	Framed around	Shure	Shaftlined core w/ steel frame	Simpler detailing than 1b	Still requires seismic gap, shaft likely not strong enough	
1d	Tied-in Shaft	2Hr				shaft		Shaftlined core w/ steel frame	Doesn't require seismic gap, shaft strength not required	Detailing would be difficult. Shaft needs to be slipped in- plane to not attrack load & crack	Option 1 - Initial Design
1e	Tied-in Grav Core			Floor		Shaft	Tied into shaft	Sim to platform framing but with 'flexible' walls	Typical construction., simple detailing	Vertical load down shaft needs to be resisted at transfer slab level. Also shaft can't sit on floor which is preferred detail	Option 2 - Second Design
2a	Typ Partition Framing		Floor	Floor		Framed around		Typ light-framed partition on 1hr floor	Often used on projects, similar to typ partition framing	1hr structure supporting 2hr shaft doesn't comply with IBC	
2b	2hr Partition Framing		FIGOT	FIGOT	2Hr	shaft		Typ light-framed partition on 2hr floor	Per above but complies with IBC	Satisfies all requirements, however requires addn'l bays of 2hr structure in project	Option 3 - Final Design

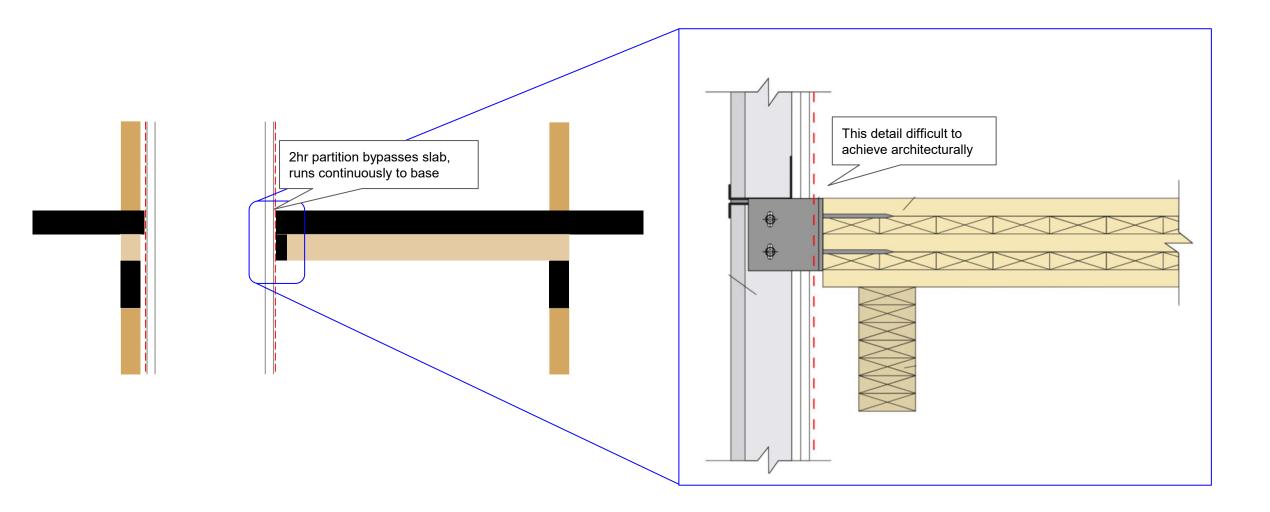
1: Floor Framed Around Shaft Opening



1: Floor Framed Around Shaft Opening



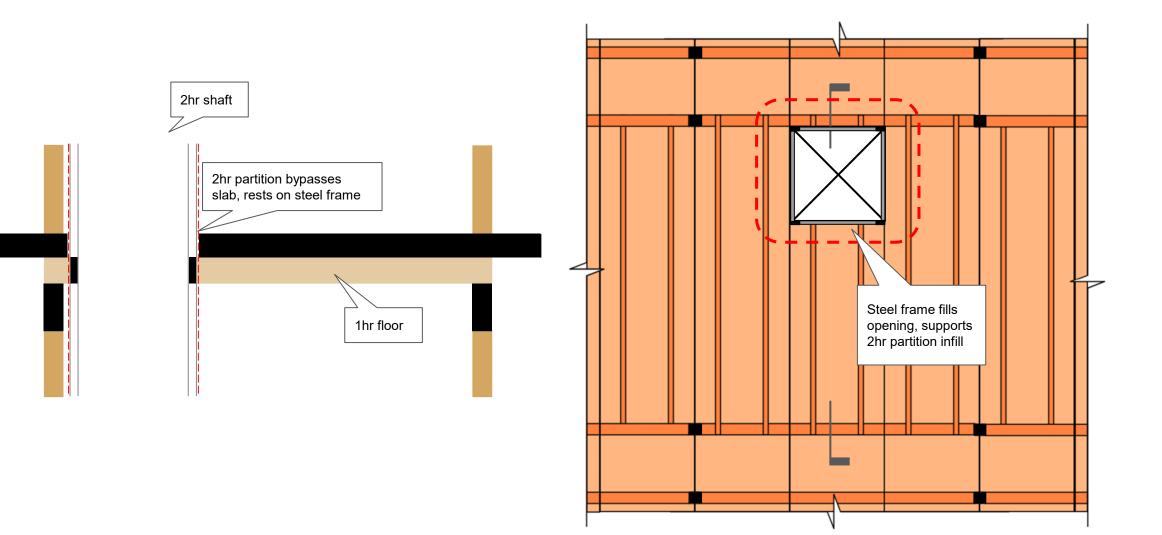
1: Floor Framed Around Shaft Opening



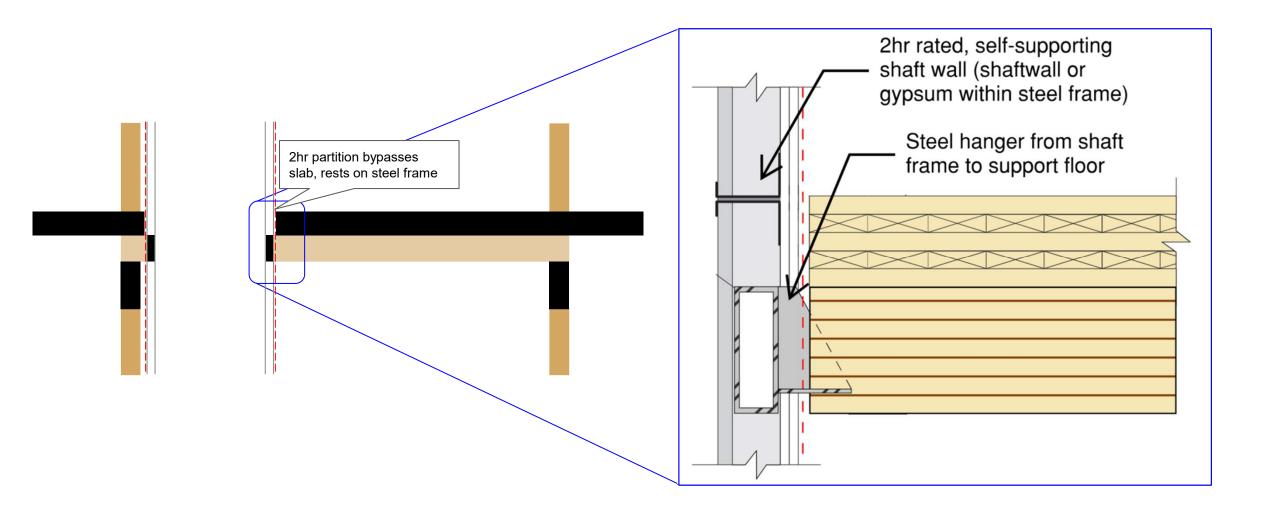


Number	Name		Shaft			Floor		Example	Passan ta una	Bassan to not use	Notes					
Number	Name	Rating	Vertical	Lateral	Rating	Vertical	Lateral		Reason to use	Reason to not use						
1a	Typ Shear /Grav Core					Shaft	Tied into shaft	CMU core, platform framed wood construction	Typical construction., simple detailing	Core not strong enough to resist loads attracted. Could detail to yield but may crack at SLE. Poor seismic performance.						
1b	Isolated Shear /Grav Core	Self 2Hr		Self		Shart	Isolated from shaft	CMU core	Stops core attracting seismic load	Tricky detailing to slip floor relative to shaft, negates benefit of CMU core, seismic gap may be large						
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1e	Tied-in Grav Core			Floor		Shaft	Tied into shaft	Sim to platform framing but with 'flexible' walls	Typical construction., simple detailing	Vertical load down shaft needs to be resisted at transfer slab level. Also shaft can't sit on floor which is preferred detail	Option 2 - Second Design					
2a	Typ Partition Framing		Floor	Floor		Framed around		Typ light-framed partition on 1hr floor	Often used on projects, similar to typ partition framing	1hr structure supporting 2hr shaft doesn't comply with IBC						
2b	2hr Partition Framing	Floor	Floor		FIOOT		shaft 2Hr			Typ light-framed partition on 2hr floor	Per above but complies with IBC	Satisfies all requirements, however requires addn'l bays of 2hr structure in project	Option 3 - Final Design			

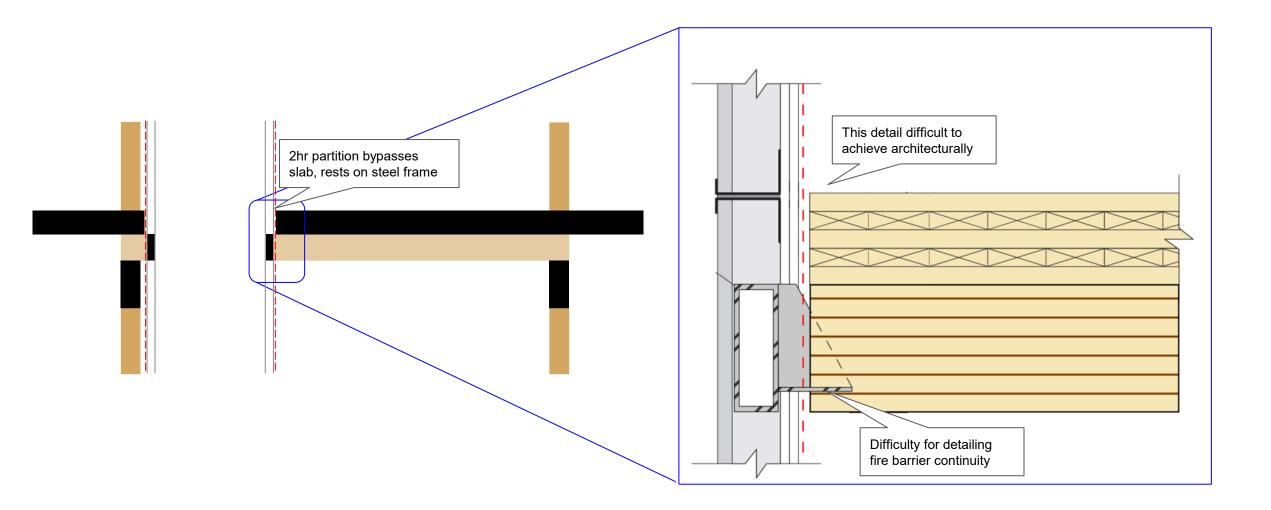
2: Floor Supported by Shaft



2: Floor Supported by Shaft



2: Floor Supported by Shaft



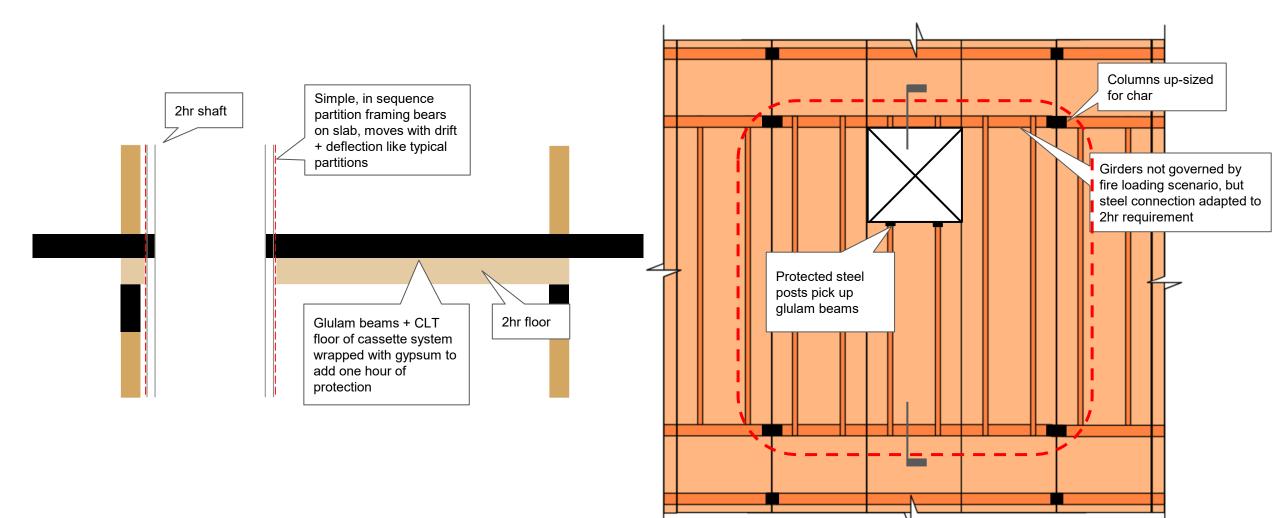
Shaft Schematic Approaches



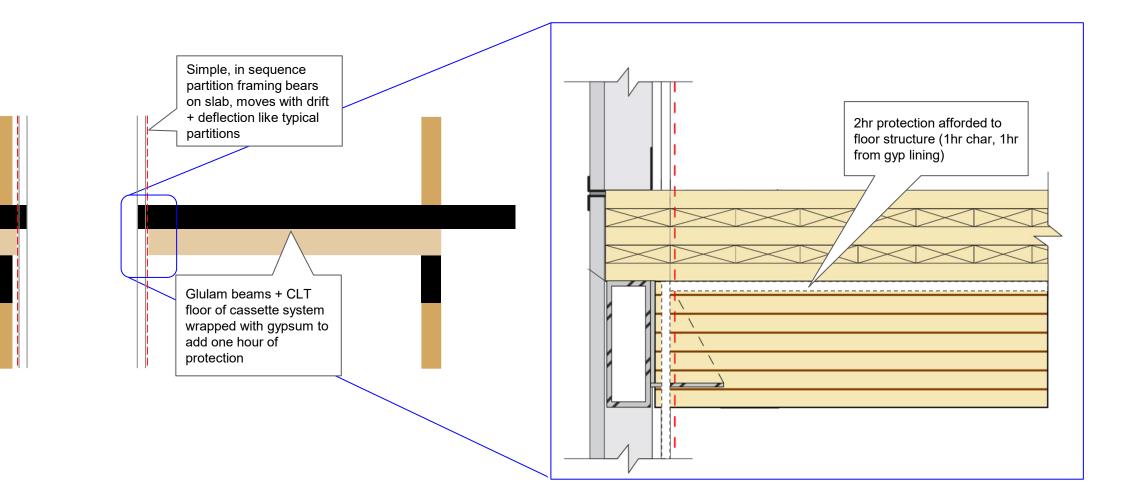
Shaft Schematic Approaches

Number	Name	Shaft			Floor		Fyampla	Desses to use	Process to motives	Notes	
		Rating	Vertical	Lateral	Rating	Vertical	Lateral	Example	Reason to use	Reason to not use	Notes
1a	Typ Shear /Grav Core		Self	Self	1Hr	Shaft	Tied into shaft	CMU core, platform framed wood construction	Typical construction., simple detailing	Core not strong enough to resist loads attracted. Could detail to yield but may crack at SLE. Poor seismic performance.	
1b	Isolated Shear /Grav Core						Isolated from shaft	CMU core	Stops core attracting seismic load	Tricky detailing to slip floor relative to shaft, negates benefit of CMU core, seismic gap may be large	
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1e	Tied-in Grav Core					Shaft		Sim to platform framing but with 'flexible' walls	Typical construction., simple detailing	Vertical load down shaft needs to be resisted at transfer slab level. Also shaft can't sit on floor which is preferred detail	Option 2 - Second Design
2a	Typ Partition Framing			E1.		Framed around		Typ light-framed partition on 1hr floor	Often used on projects, similar to typ partition framing	1hr structure supporting 2hr shaft doesn't comply with IBC	
2b	2hr Partition Framing			11001	2Hr	shaft		Typ light-framed partition on 2hr floor	Per above but complies with IBC	Satisfies all requirements, however requires addn'l bays of 2hr structure in project	Option 3 - Final Design

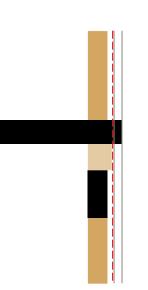
3: Shaft Supported by Floor



3: Shaft Supported by Floor



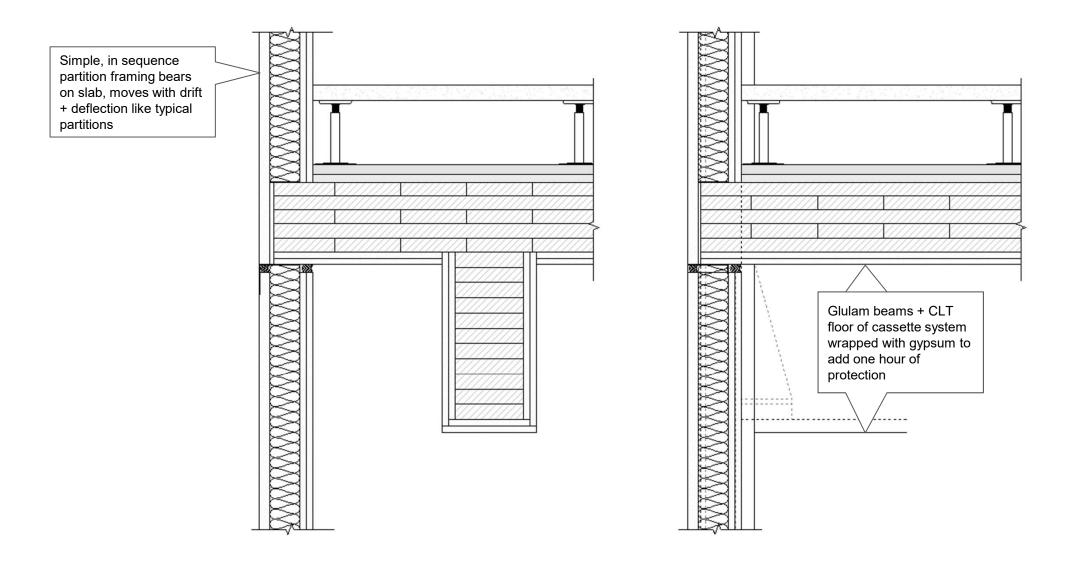
3: Shaft Supported by Floor



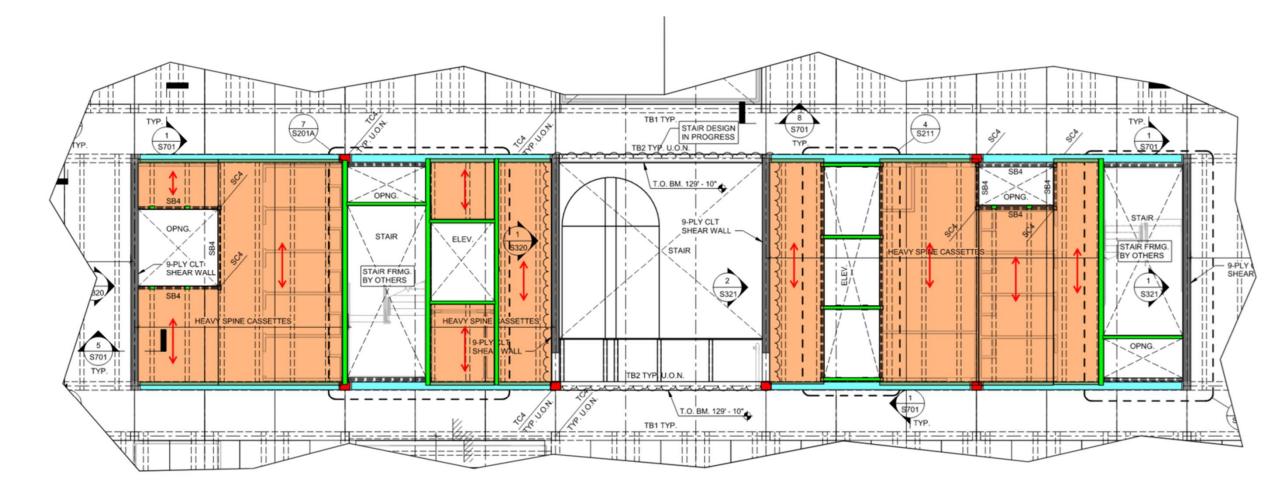
Simple, in sequence partition framing bears on slab, moves with drift + deflection like typical partitions	
\wedge	
Glulam beams + CLT floor of cassette system wrapped with gypsum to add one hour of protection	

2hr Structure Impacts							
Element	1hr	2hr					
CLT Deck & GLT Ribs	Unwrapped	(2) layers ^₅ %" gyp added					
GLT Girders	Unwrapped	Unwrapped					
GLT Girder - Column Connection	Embedded bearing pl w/ 1.8" blocking for char protection	Embedded bearing pl w/ 1.8" gyp for insulation protection					
GLT Column Size (at base)	14.25x18"	14.25x25.5"					

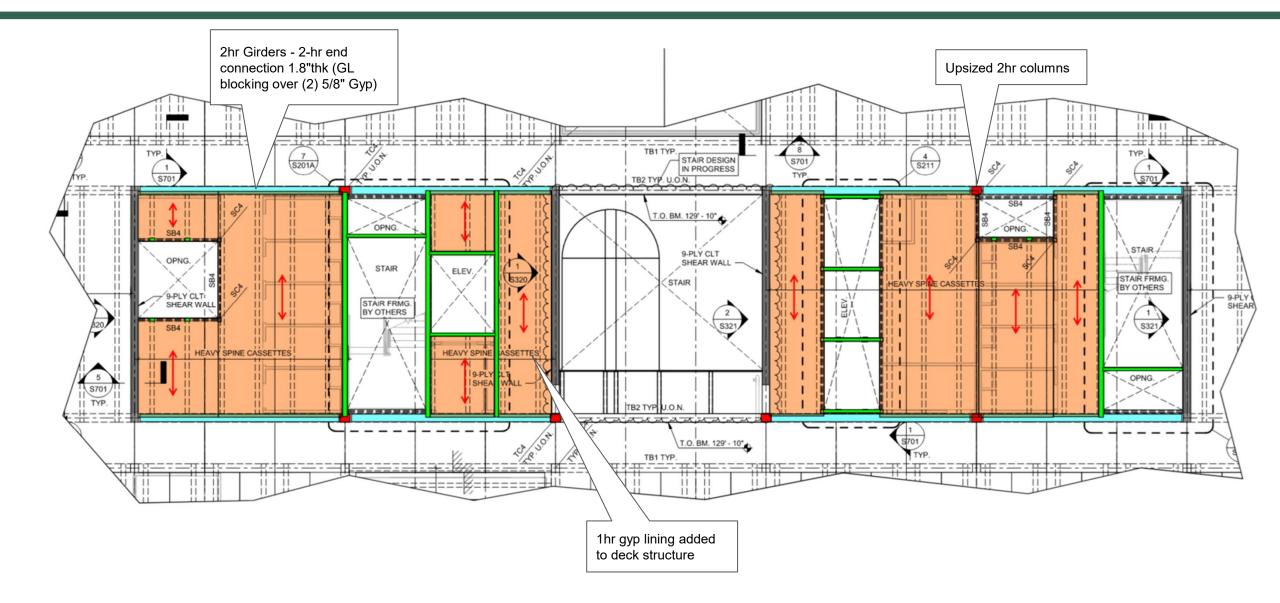
Detailing



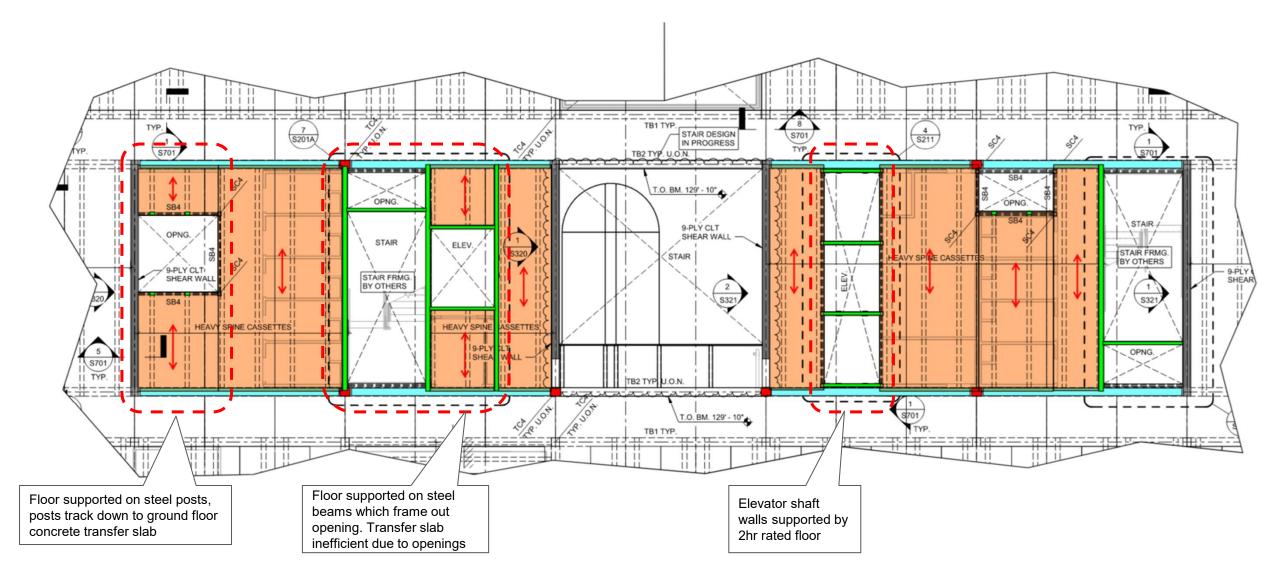
Framing Variations



Framing Variations



Framing Variations



Questions?



Chelsea Drenick, SE Regional Director | CA-North, NV, UT WoodWorks (303) 588-1300 chelsea.drenick@woodworks.org



Matt Harwood Senior Structural Engineer Holmes US (971) 337-8114 matt.harwood@holmes.us This concludes The American Institute of Architects Continuing Education Systems Course

Photo: Quality Contractors