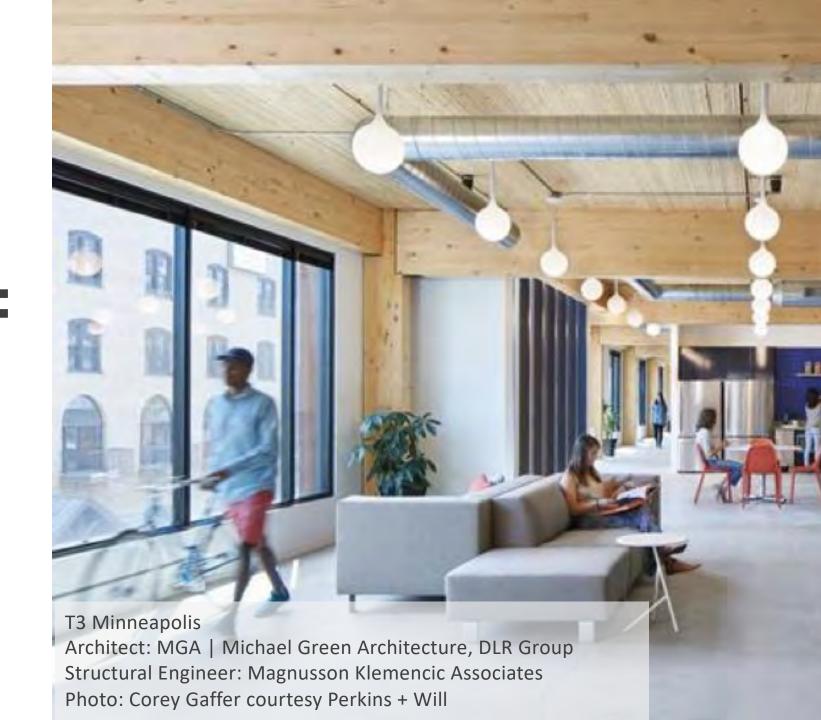




Mass Timber Construction: Products, Performance and Design

Archie Landreman, CSI WoodWorks





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This course is registered with AIA CES for continuing professional education. As such, it does not include content that may be deemed or construed to be an approval or endorsement by the AIA of any material of construction or any method or manner of handling, using, distributing, or dealing in any material or product.

Questions related to specific materials, methods, and services will be addressed at the conclusion of this presentation.

> Course Description

Due to their high strength, dimensional stability and positive environmental performance, mass timber building products are quickly becoming materials of choice for sustainably-minded designers. This presentation will provide a detailed look at the variety of mass timber products available, including glue-laminated timber (glulam), cross laminated timber (CLT), nail laminated timber (NLT), heavy timber decking, and other engineered and composite systems. Applications for the use of these products under modern building codes will be discussed, and examples of their use in U.S. projects reviewed. Mass timber's ability to act as both structure and exposed finish will also be highlighted, as will its performance as part of an assembly, considering design objectives related to structural performance, fire resistance, acoustics, and energy efficiency. Other topics will include detailing and construction best practices, lessons learned from completed projects and trends for the increased use of mass timber products in the future.



> Learning Objectives

- 1. Identify mass timber products available in North America and consider how they can be used under current building codes and standards.
- 2. Review completed mass timber projects that demonstrate a range of applications and system configurations.
- 3. Discuss benefits of using mass timber products, including structural versatility, prefabrication, lighter carbon footprint, and reduced labor costs.
- 4. Highlight possibilities for the expanded use and application of mass timber in larger and taller buildings.

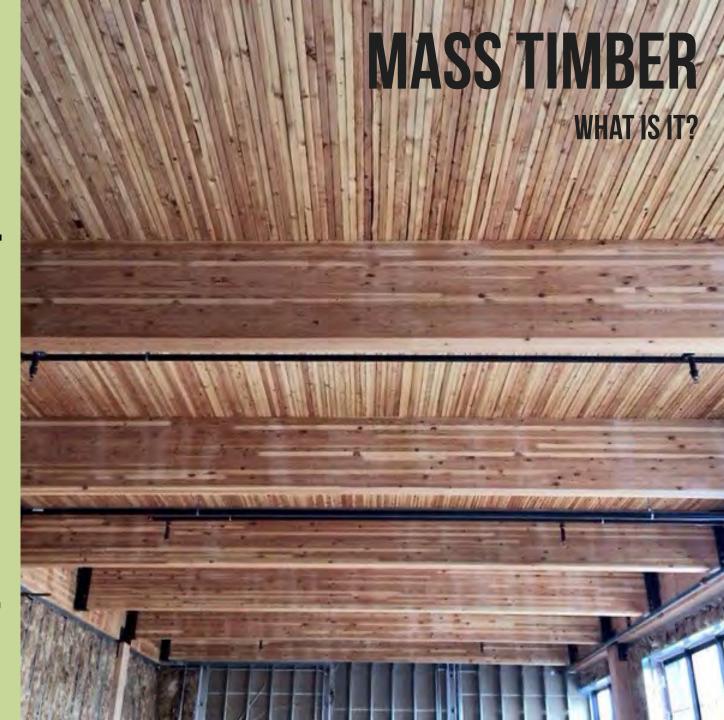








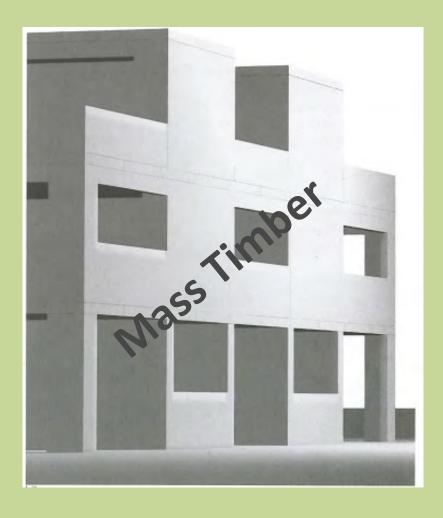
MASS TIMBER IS A CATEGORY OF FRAMING STYLES OFTEN USING SMALL **WOOD MEMBERS FORMED INTO LARGE PANELIZED SOLID WOOD CONSTRUCTION INCLUDING CLT, NLT OR** GLULAM PANELS FOR FLOOR, **ROOF AND WALL FRAMING**



BUILDING FRAME SYSTEMS

















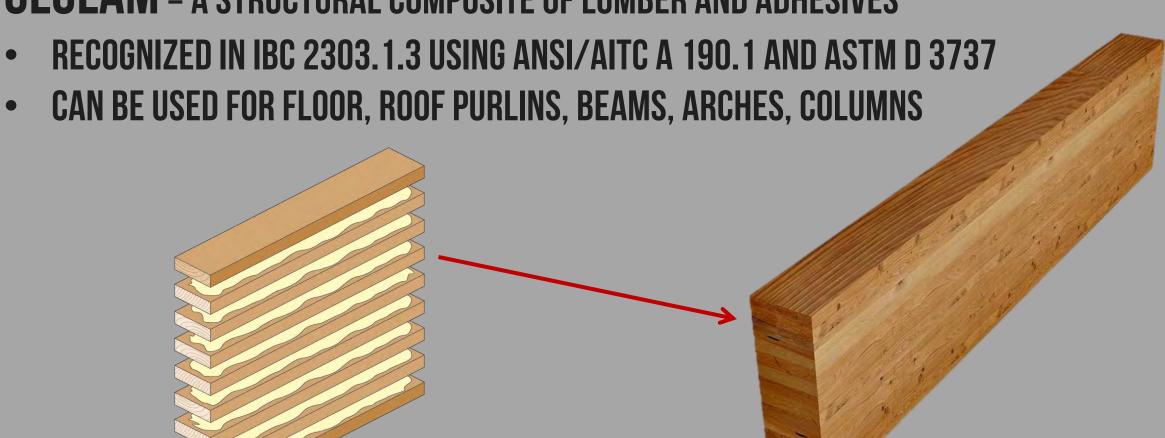
GLULAM





GLULAM

GLULAM = A STRUCTURAL COMPOSITE OF LUMBER AND ADHESIVES



GLULAM

GLULAM LAYUP:

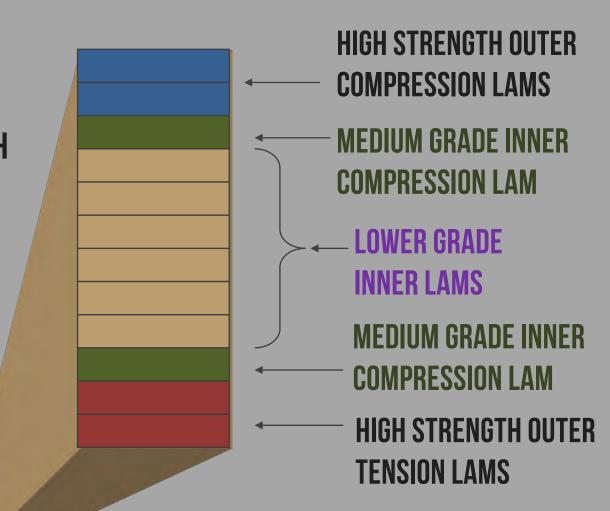
VARY STRENGTH OF LAMINATIONS

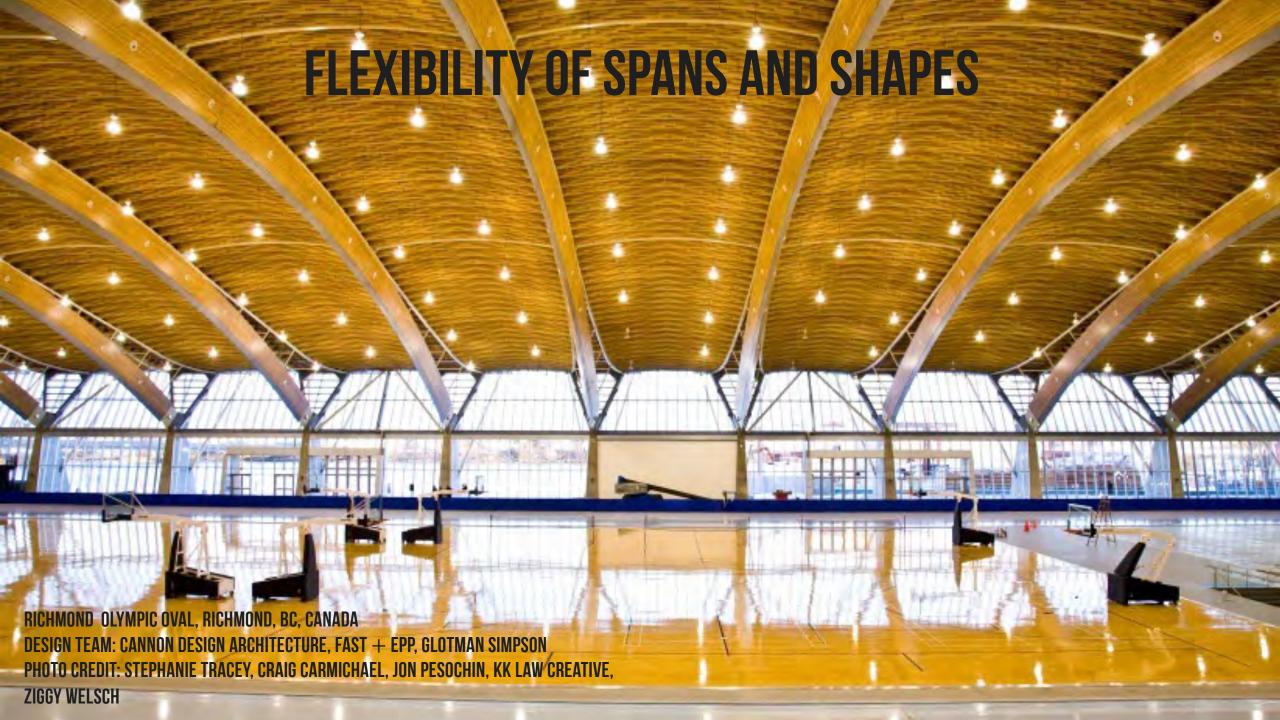
 HIGHER STRENGTH LAMS AT TOP AND BOTTOM -TENSION AND COMPRESSION STRESSES ARE HIGH

LOWER STRENGTH LAMS IN CENTER PLIES

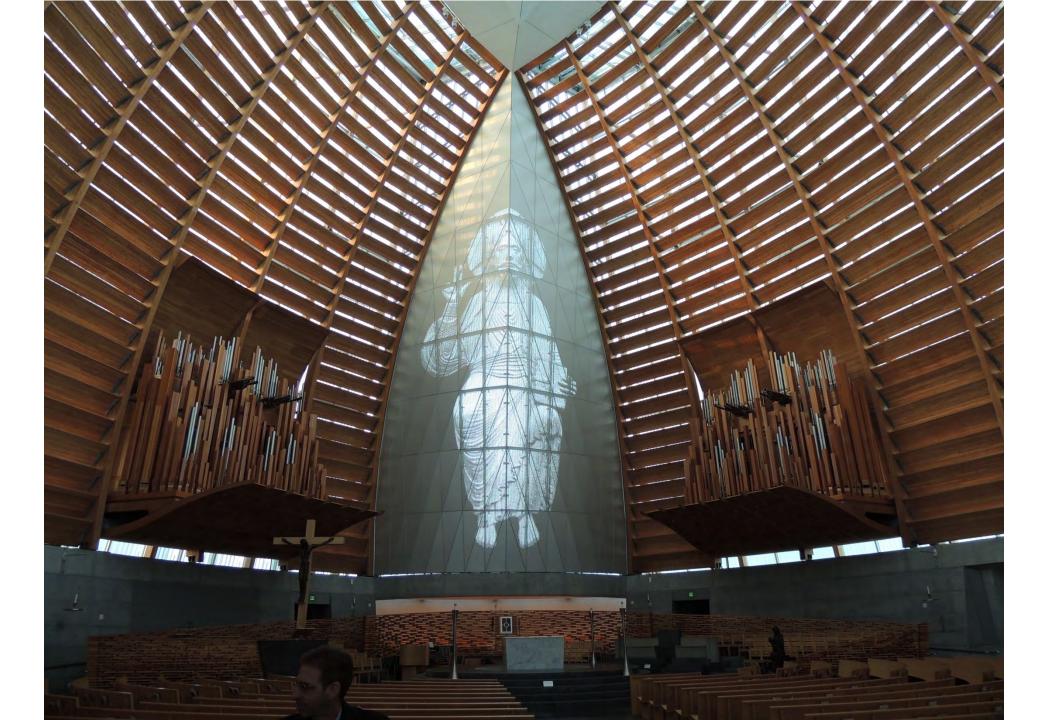


IMAGE: APA

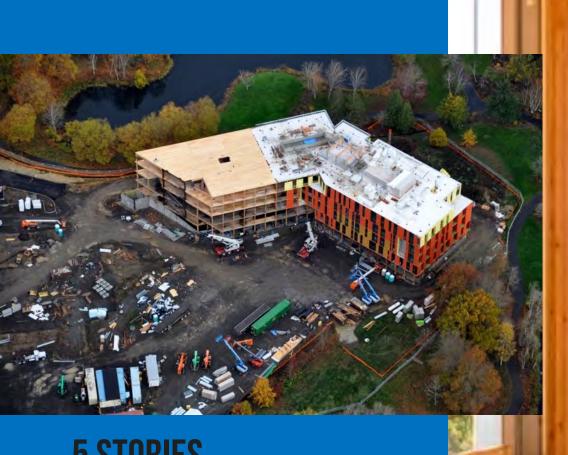








FIRST TECH CREDIT UNION



5 STORIES 156,000 SF







Photo credit: StructureCraft Builders/Freres Lumber



What is it?

Nail-laminated timber (NLT) is mechanically laminated to create a solid timber panel. NLT is created by placing dimension lumber (nominal 2x, 3x, or 4x thickness and 4 in. to 12 in. width) on edge and fastening the individual laminations together with nails.

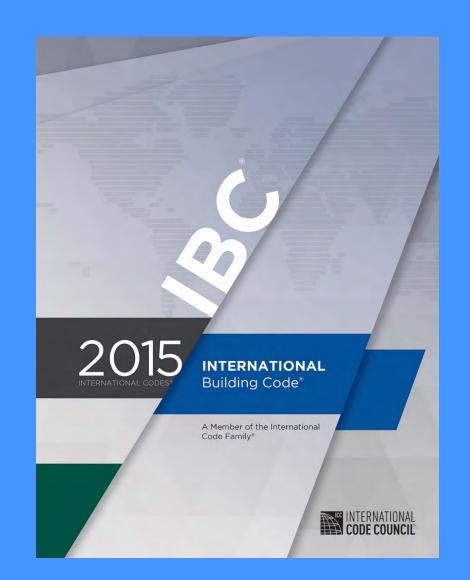
Image: Think Wood

NAIL-LAMINATED TIMBER (NLT) PANELS

When does the code allow it to be used?

IBC defines NLT as mechanically laminated decking per IBC 2304.9.3

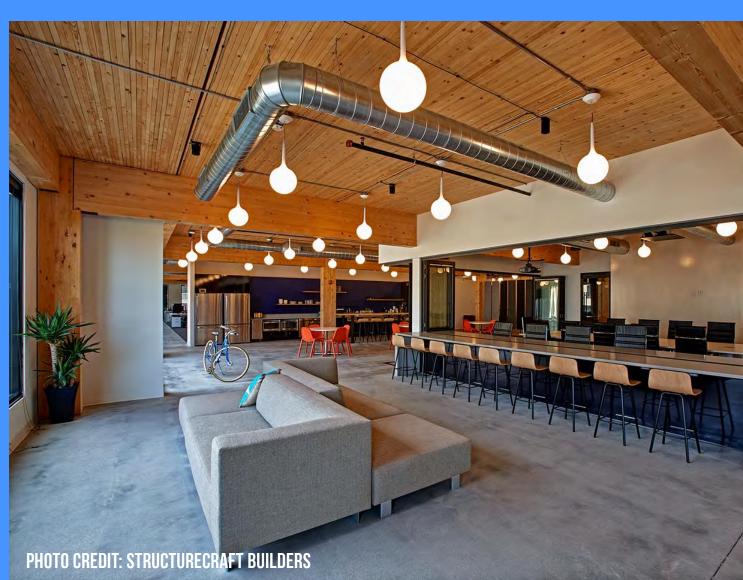
Permitted anywhere that combustible materials and heavy timber are allowed, plus more

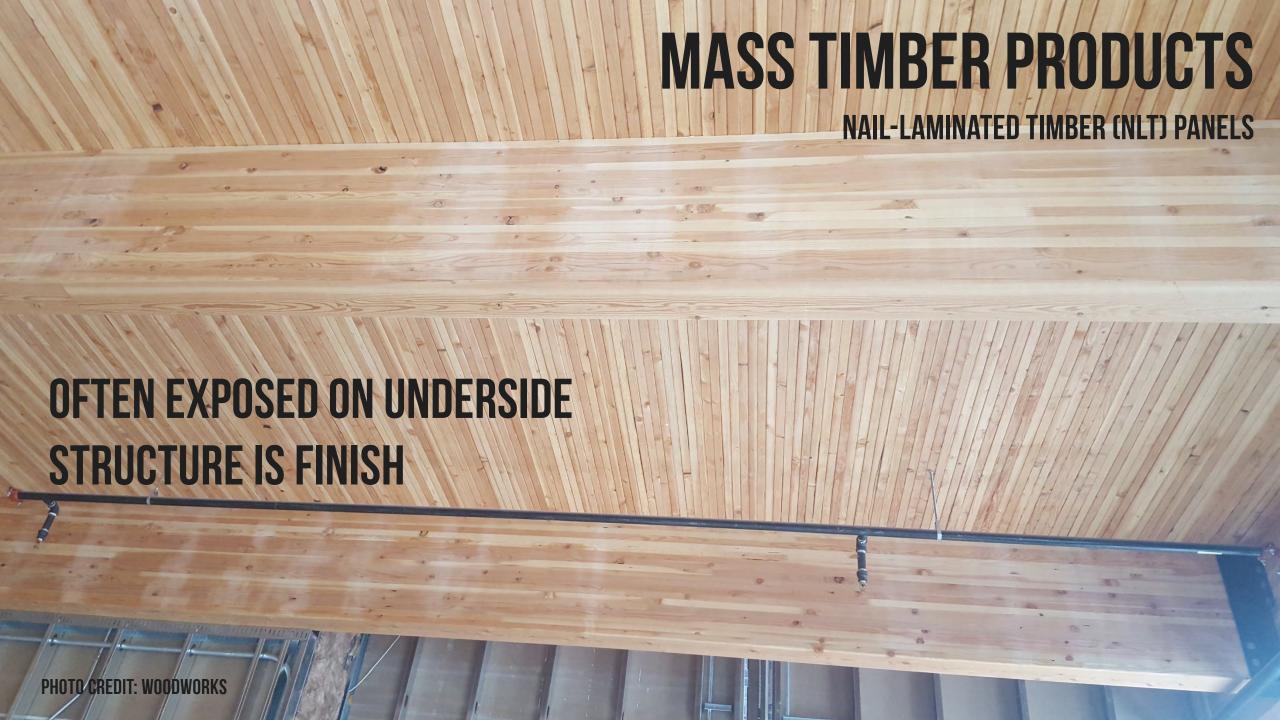


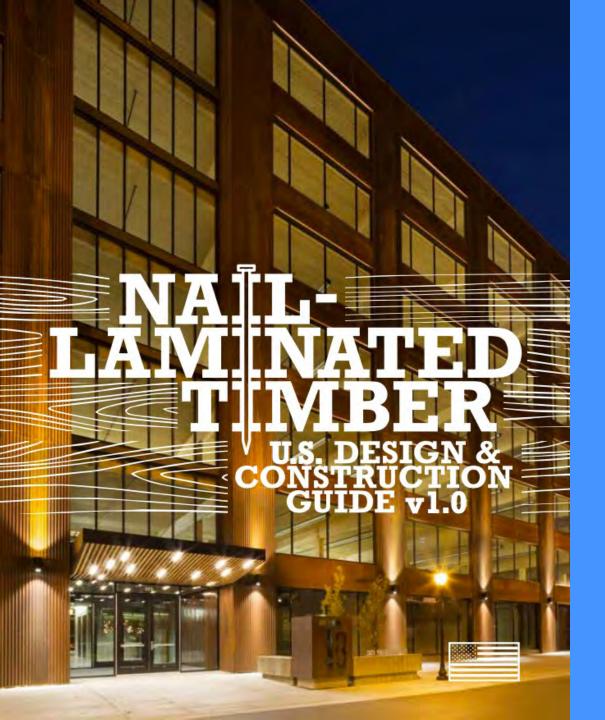
NAIL-LAMINATED TIMBER (NLT) PANELS

When is it used?

NLT is typically used for floor and roof panels. Plywood/OSB added to one face can provide in-plane shear capacity, allowing the product to be used as a diaphragm. Can also be used for walls, shafts.







CONTENT INCLUDES:

- ARCHITECTURE
- FIRE
- STRUCTURE
- ENCLOSURE
- SUPPLY AND FABRICATION
- CONSTRUCTION AND INSTALLATION
- ERECTION ENGINEERING

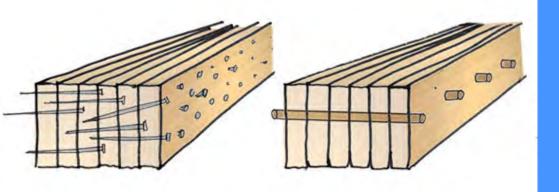
FREE DOWNLOAD AT WWW.THINKWOOD.COM/NLTGUIDE











DOWEL-LAMINATED TIMBER (DLT) PANELS

DLT:

- SIMILAR TO NLT NAILS CONNECTING LAMS REPLACED WITH HARDWOOD DOWELS
- COMMON IN EUROPE OFTEN REFERRED TO AS BRETTSTAPEL
- NOT CURRENTLY RECOGNIZED AS PRESCRIPTIVELY PERMITTED MATERIAL IN IBC
- TIMBER FRAMERS GUILD RESOURCES ON DOWEL DESIGN



Dowel Laminated Timber

The All Wood Panel

Mass Timber Design Guide

DLT: SIMILAR TO NLT — BUT LAMS ARE USUALLY FINGER JOINTED IN DLT SO JOINT LAYUPS NOT A CONCERN

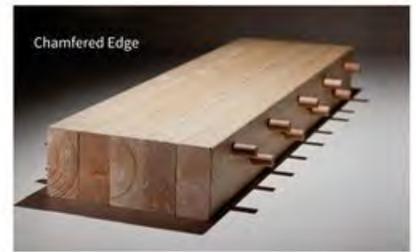
CREDIT: STRUCTURECRAFT BUILDERS

VARIOUS PROFILE OPTIONS

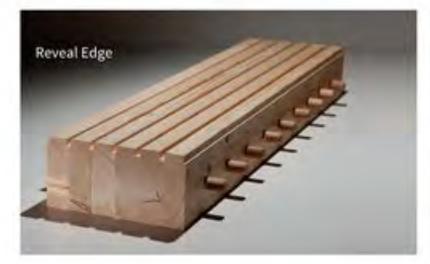
MASS TIMBER PRODUCTS

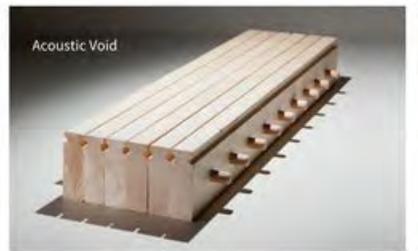
DOWEL-LAMINATED TIMBER (DLT) PANELS











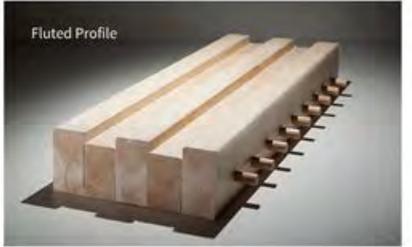


PHOTO CREDIT: STRUCTURECRAFT BUILDERS









GLUE-LAMINATED TIMBER (GLT) PANELS

GLULAM DECKING:

- SIMILAR TO DEEP GLULAM BEAMS LAID ON THEIR SIDE
- SAME CODE REFERENCES AND MANUFACTURING STANDARDS AS GLULAM BEAMS AND COLUMNS
- BE CAREFUL OF DESIGN STRESSES AND LAYUPS USED SPEC UNIFORM LAYUP (ALL LAMS SAME SPECIES & GRADE)



IMAGE SOURCE: STRUCTURECRAFT BUILDERS

GLUE-LAMINATED TIMBER (GLT) PANELS





IMAGE SOURCE: STRUCTURECRAFT BUILDERS

					Use with	Table	5A A	djustn	nent Fac	tors					
	Bending About X-X Axis (Loaded Perpendicular to Wide Faces of Laminations)								Bending About Y-Y Axi (Loaded Parallel to Wide Face of Laminations)						
	Bending		Compression Perpendicular to Grain		Shear Parallel to Grain	Modulus of Elasticity		Bending	Compression Perpendicular to Grain	Shear Parallel to Grain		fulus of sticity			
	Bottom of Bea Stressed in Tension (Positive Bendin	n Top of Beam Stressed in Terreion 19) (Negative Bending)	Tension Face	Compression Face		For Deflection Calculations	For Stability Calculations				For Deflection Calculations	Far Statility Calculations			
ecies	F _{bx} ⁺	F _{bx}		F _{c⊥x}	F _{vx} (2)	Ex	E _{x min}	F _{by}	F _{c_ly}	F _{vy} (2)(3)	E _v	E _{y min}			
er/ Core	(psi)	(psi)	(psi)		(psi)	(10 ⁶ psi)	(10 ⁶ psi)	(psi)	(psi)	(psi)	(10 ⁶ psi)	(10 ⁶ psi)			
	2400	1450		650	265	1.8	0.95	1450	560	230	1.6	0.85			
F/DF F/DF F/DF F/DF	2400 2400 2400 2400 2400	1850 2400 1450 2400 2400	650 650 650 650 650	650 650 650 650 650	265 265 265 265 265 265	1.8 1.8 1.8 1.8 1.8	0.95 0.95 0.95 0.95 0.95	1450 1550 1400 1750 1550	560 560 560 560 560	230 230 230 230 230 230	1.6 1.6 1.7 1.7	0.85 0.85 0.90 0.90 0.90			
P/SP	2400	2000	740	740	300	1.8	0.95	1700	650	260	1.6	0.85			

NDS SUPPLEMENT LISTS DIFFERENT
DESIGN VALUES FOR BENDING.
LAYUP COMBINATIONS TYPICALLY
OPTIMIZED FOR BEAM APPLICATIONS.
LAYUP COMBINATIONS AREN'T EFFECTIVE
IN GLT DECKING APPLICATIONS



THICKNESSES IN 1" INCREMENTS STRUCTURAL PROPERTIES IN APA PRODUCT REPORT PR-L325

MASS TIMBER PRODUCTS

MASS PLYWOOD PANELS (MPP)



Table 1	ASD Reference	Design	Values (a,b,c) for	r Freres M	IPP (For	Use in th	eUS)

				Major Strengt	h Direction		Minor Strength Direction				
MPP Layup	Layup ID	Thickness, t _p (in.)	(F _b S) _{eff,f,0} (Ibf-ft/ft)	(EI) _{eff,f,0} (10 ⁶ lbf-in. ² /ft)	(GA) _{eff,f,0} (10 ⁶ lbf/ft)	V _{s,0} (lbf/ft)	(F _b S) _{eff,f,90} (lbf-ft/ft)	(EI) _{eff,f,90} (10 ⁶ lbf-in. ² /ft)	(GA) _{eff,f,90} (10 ⁶ lbf/ft)	V _{s,90} (lbf/ft)	
	F16-2	2	1,110	16	0.82	2,190	210	2.8	0.17	695	
	F16-3	3	1,870	51	1.23	2,190	355	9.0	0.26	695	
	F16-4	4	3,325	122	1.64	2,925	630	21	0.34	930	
	F16-5	5	5,200	238	2.05	3,650	985	42	0.43	1,160	
	F16-6	6	7,500	410	2.46	4,375	1,420	72	0.69	1,390	
F16	F16-7	7	10,200	652	2.66	5,100	1,930	114	0.81	1,630	
	F16-8	8	13,325	973	3.04	5,825	2,525	170	0.91	1,860	
	F16-9	9	16,850	1,385	3.42	6,575	3,200	242	1.04	2,090	
	F16-10	10	20,825	1,900	3.80	7,300	3,950	333	1.15	2,320	
	F16-11	11	25,175	2,529	4.18	8,025	4,775	443	1.27	2,550	
	F16-12	12	29,975	3,283	4.56	8,750	5,675	575	1.38	2,775	

For SI: 1 in. = 25.4 mm; 1 ft = 304.8 mm; 1 lbf = 4.448N

$$\delta = \frac{22.5wL^4}{(EI)_{eff}} + \frac{3wL^2}{2(GA)_{eff}}$$

where: δ = Estimated deflection, inches; w = uniform load, plf;

L = span, feet; (EI)_{eff} = tabulated effective bending stiffness, 10⁶

(GA)_{eff} = tabulated effective in-plane (planar) shear rigidity, 10⁶ lbf/ft



⁽a) Tabulated values are allowable design values.

⁽b) Tabulated values are limited to MPP manufactured with 1-inch-thick Freres 1.6E Douglas-fir LVL.

Deflection under a specified uniformly distributed load, w, acting perpendicular to the face of a single deflections due to moment and shear effects using the effective bending stiffness, (EI)_{eff}, and the effectives:

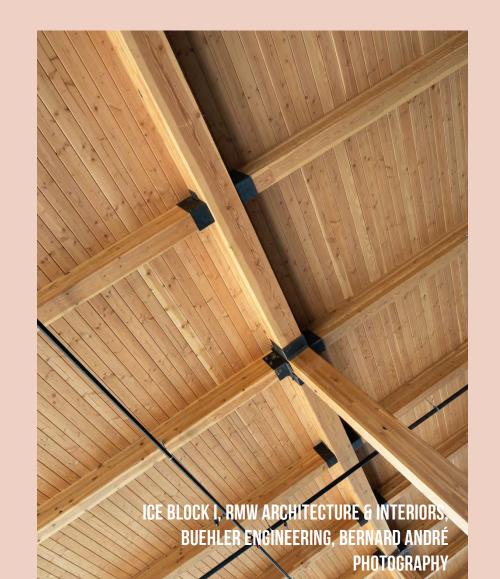


TONGUE AND GROOVE DECKING

TONGUE AND GROOVE DECKING:

2X, 3X OR 4X SOLID OR LAMINATED WOOD DECKING LAID FLAT WITH INTERLOCKING TONGUE AND GROOVE ON NARROW (SIDE) FACE

- RECOGNIZED IN IBC 2304.8 (LUMBER DECKING)
- 2X USUALLY HAS A SINGLE T&G; 3X AND 4X USUALLY HAVE A DOUBLE T&G
- 6" AND 8" ARE COMMON WIDTHS
- CAN BE USED FOR FLOOR, ROOF DECKING





CAN BE USED BY ITSELF AS A
DIAPHRAGM: SDPWS TABLE 4.2D
OR ADD LAYER OF WSP ON TOP, TREAT
AS BLOCKED DIAPHRAGM

MASS TIMBER PRODUCTS

TONGUE AND GROOVE DECKING



TAG DECKING DESIGN

TONGUE AND GROOVE DECKING

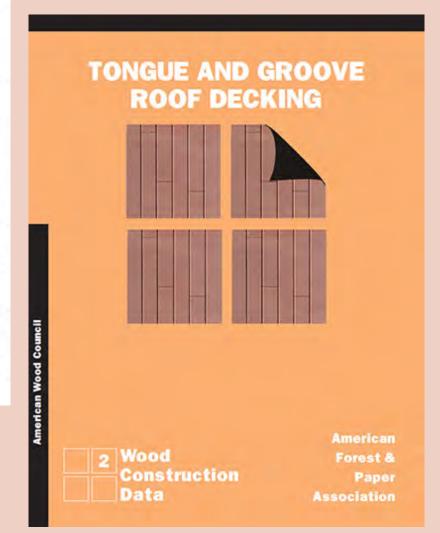
Table 4E Reference Design Values for Visually Graded Decking^{1,2}

(Tabulated design values are for normal load duration and dry service conditions, unless specified otherwise. See NDS 4.3 for a comprehensive description of design value adjustment factors.)

USE WITH TABLE 4E ADJUSTMENT FACTORS

			Design valu	es in pounds per					
		Bending					1 1		
Species and commercial grade	Size classification	Single Member	Repetitive Member (F _b)(C _r)	Compression perpendicular to grain F _{cL}	Modulus of Elasticity		Specific Gravity ³	Grading Rules Agency	
		Fb			E	Emin	G		
BALSAM FIR									
Select	2*-4* thick	-	1,650	-	1,500,000	550,000	0.36	NELMA	
Commercial	4'-12'wide	-	1,400		1,300,000	470,000	0.30		
COAST SITKA SPRUCE)							
Select	2"-4" thick	1,250	1,450	455	1,700,000	620,000	0.43	NLGA	
Commercial	4*& wider	1,050	1,200	455	1,500,000	550,000	0.43		
COAST SPECIES		1 1							
Select	2*-4* thick	1,250	1,450	370	1,500,000	550,000	0.43	NICA	
Commercial	4*& wider	1,050	1,200	370	1,400,000	510,000	0.43	NLGA	
DOUGLAS FIR-LARCH									
Select Dex	2"-4" thick	1,750	2,000	625	1,800,000	660,000	0.50	WCLIB	
Commorpial Day	et etuate	1.450	4 850	626	1 700 000	820.000	0.50	WOLIB	

NDS SUPPLEMENT CONTAINS ALLOWABLE DESIGN VALUES AWC'S WCD-2 CONTAINS SPAN TABLES

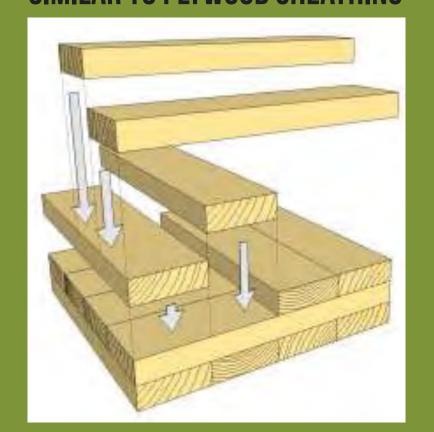








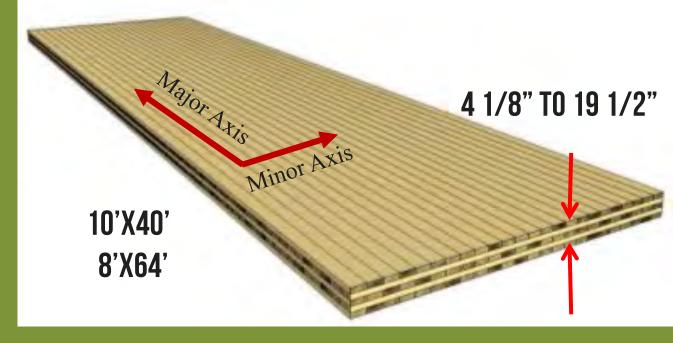
WHAT IS CLT? SOLID WOOD PANEL 3 LAYERS MIN. OF SOLID SAWN LAMS 90 DEG. CROSS-LAMS SIMILAR TO PLYWOOD SHEATHING





MASS TIMBER PRODUCTS

CROSS-LAMINATED TIMBER (CLT)



COMMON CLT LAYUPS

3-PLY 3-LAYER



5-PLY 5-LAYER



7-PLY 7-LAYER



9-PLY 9-LAYER

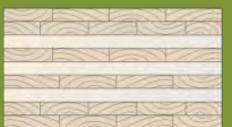


MASS TIMBER PRODUCTS

CROSS-LAMINATED TIMBER (CLT)



7-PLY 5-LAYER



9-PLY 7-LAYER





CANDLEWOOD SUITES

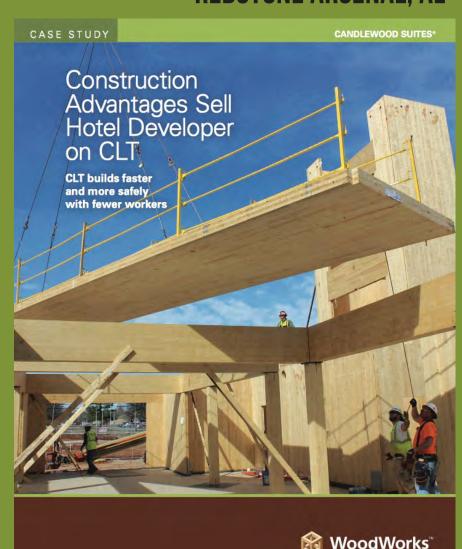




- 62,600 SF, 4 STORY HOTEL, 92 PRIVATE ROOMS
- CLT UTILIZED FOR WALLS, ROOF PANELS, AND FLOOR PANELS
- 1,557 CLT PANELS; TYPICAL FLOOR PANEL IS 8'X50' & WEIGHS 8,000 LBS
- COMPLETED LATE 2015

CANDLEWOOD SUITES

REDSTONE ARSENAL, AL



PAL Portfolio	Typical New PAL Hotel (Actual*)	Redstone Arsenal (Actual)	Difference
Gross square feet (sf)	54,891	62,688	+14%
Average # of employees	18 (peak 26)	10 (peak 11)	-43%
Structural duration (days)	123	78	-37%
Structural person hours	14,735	8,203	-44%
Structural production rate/day	460 sf	803 sf	+75%
Overall schedule	15 months	12 months	-20%

^{*} PAL New Build Hotel Historical Average Source: Lendlease



Savings on this CLT project compared to typical light gauge steel construction

Candlewood Suites at Redstone Arsenal, AL 4 Stories, 62k SF

43%

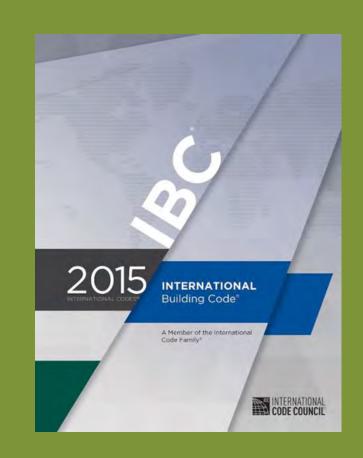
CROSS-LAMINATED TIMBER (CLT)

IN 2015 IBC, CLT IS NOW DEFINED IN CHAPTER 2 DEFINITIONS:

[BS] CROSS-LAMINATED TIMBER. A prefabricated engineered wood product consisting of not less than three layers of solid-sawn lumber or *structural composite lumber* where the adjacent layers are cross oriented and bonded with structural adhesive to form a solid wood element.

AND IS REFERENCED IN CHAPTER 23:

2303.1.4 Structural glued cross-laminated timber. Cross-laminated timbers shall be manufactured and identified in accordance with ANSI/APA PRG 320.



ANSI/APA PRG 320-2018

AMERICAN NATIONAL STANDAR

Standard for Performance-Rated Cross-Laminated Timber

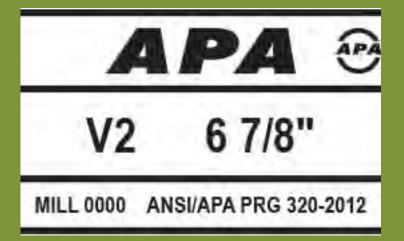




MASS TIMBER PRODUCTS

CROSS-LAMINATED TIMBER (CLT)

CLT PRODUCT STANDARD



ANSI / APA PRG 320 STANDARD FOR PERFORMANCE RATED CROSS-LAMINATED TIMBER

Timber Concrete Composite



Timber Concrete Composites

Design Considerations:

- Unique composite connection options
- Not standardized in code

 requires unique analysis

 and project approval









CLAY CREATIVE

PORTLAND, OR

- ~8' FINISHED FLOOR TO BOTTOM OF BEAM
- 25'X30' AT PERIMETER
- 30'X30' BAYS AT CENTER
- 2X6 NLT SPANS 15'
- EXTERIOR STEEL MOMENT FRAME KEEPS CORE AREA MORE VERSATILE



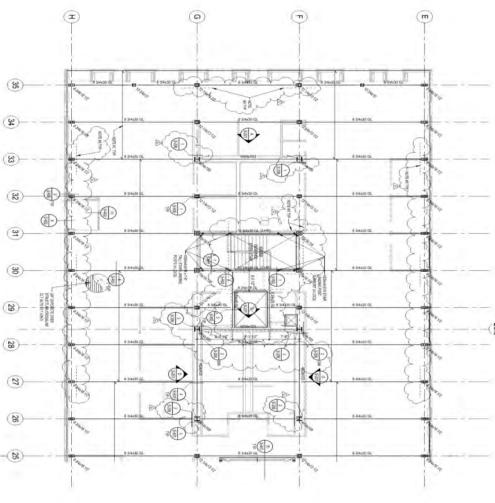


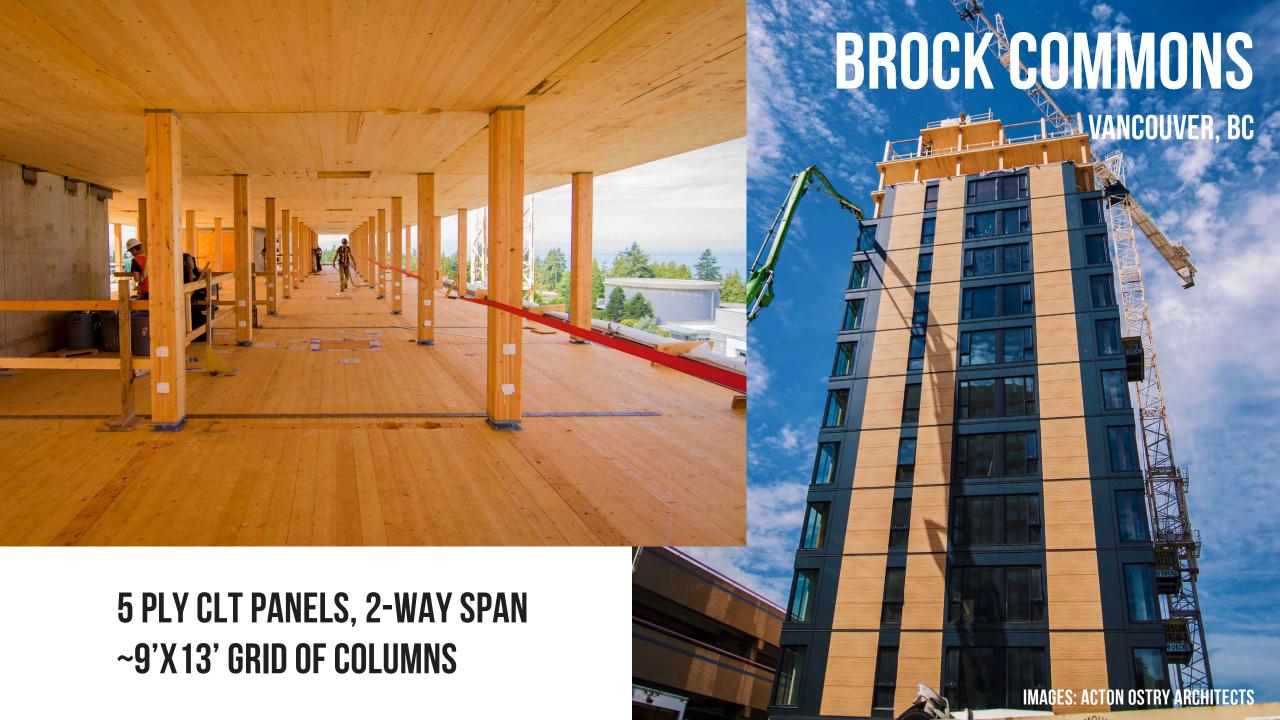
IMAGE CREDIT: SWINERTON BUILDERS

5-1/2", 5-PLY CLT SPANS 12'











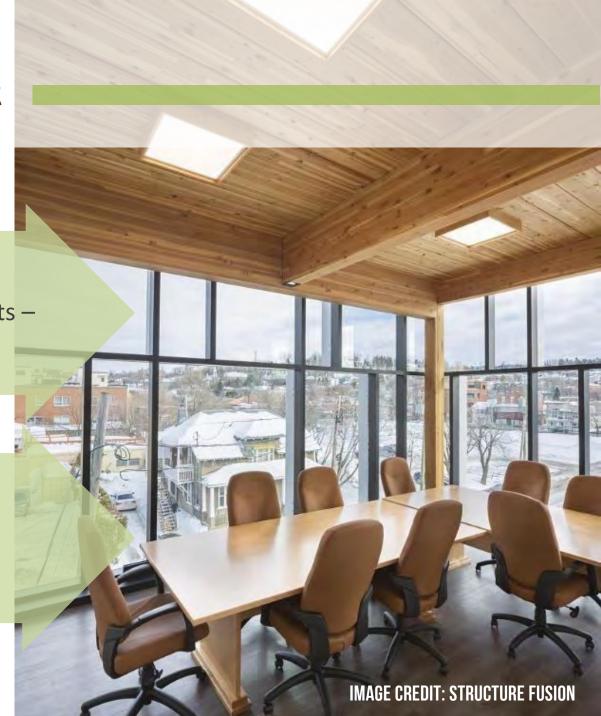
MARKET DRIVERS FOR MASS TIMBER

PRIMARY DRIVERS

- » Construction Efficiency & Speed
- » Construction site constraints –Urban Infill
- » Innovation/Aesthetic

SECONDARY DRIVERS

- » Carbon Reductions
- » Structural Performance lightweight



WHAT IS THE ULTIMATE APPEAL AND DRIVER FOR ANY BUILDING AND MATERIAL TYPE?



MURRAY GROVE,
LONDON UK
8 STORIES OF CLT OVER 1
STORY CONCRETE PODIUM

8 STORIES BUILT IN 27
DAYS (~1/2 THE TIME OF PRECAST CONCRETE)

SOURCE: ARCHITECT MAGAZINE³

MASS TIMBER APPEAL

REDUCED CONSTRUCTION TIME



LESS TIME ON SITE = LESS \$\$



FRANKLIN ELEMENTARY SCHOOL, FRANKLIN, WV

45,200 FT2 2 STORY ELEMENTARY SCHOOL

2.5 MONTHS TO ERECT CLT

SOURCE: CITY CONSTRUCTION⁴

REDUCED CONSTRUCTION TIME

1 Floor = 3 Days

17 Floors Erected in 9.5 Weeks

Brock Commons, Vancouver, BC Source: naturally:wood⁵



ALTERNATE TO CONCRETE & MASONRY









MATERIAL MASS



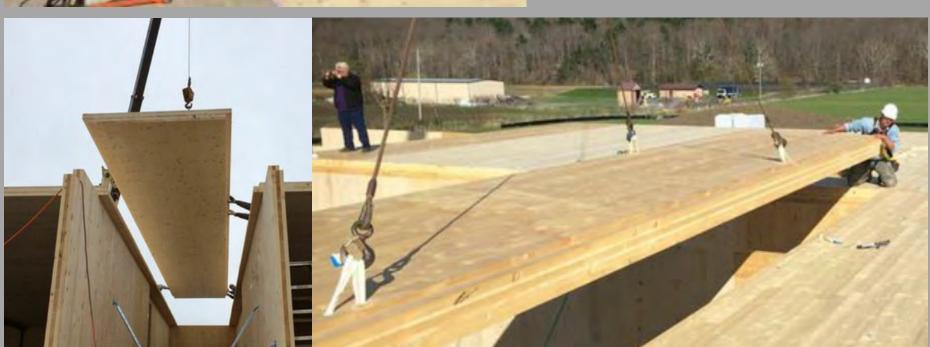


MASS TIMBER APPEAL

MATERIAL MASS

75% LIGHTER WEIGHT THAN CONCRETE

SOURCE: STRUCTURLAM⁷







FORTE', VICTORIA HARBOR, MELBOURNE, AUSTRALIA ARCHITECT: LENDLEASE | SOURCE: LENDLEASE⁸

MASS TIMBER APPEAL

MATERIAL MASS

COMPLETED IN 2012
10 STORIES
~ 105 FT. TALL, > 18.6 K SQFT.
3 YEAR INVESTMENT IN R&D
POOR SOILS REQUIRED A MUCH LIGHTER
BUILDING



ESTIMATED ENVIRONMENTAL IMPACT OF WOOD USE



Volume of wood products used: 2,233 cubic meters of CLT and Glulam



U.S. and Canadian forests grow this much wood in: 6 minutes



Carbon stored in the wood: 1,753 metric tons of CO,



Avoided greenhouse gas emissions: 679 metric tons of CO₂



Total potential carbon benefit: 2,432 metric tons of CO₂

THE ABOVE GHG EMISSIONS ARE EQUIVALENT



511 cars off the road for a year



Energy to operate a home for 222 years

and J. O'Connor, 2010, A Synthesis of Research on Wood Products and Greenhouse Gas Impacts, FPInnovations (this relates to carbon stored and avoided GHG).

*CO2 in this case study refers to CO2 equivalent

SOURCE: NATURALLY:WOOD9

MASS TIMBER APPEAL

REDUCED EMBODIED CARBON

BROCK COMMONS, VANCOUVER, BC



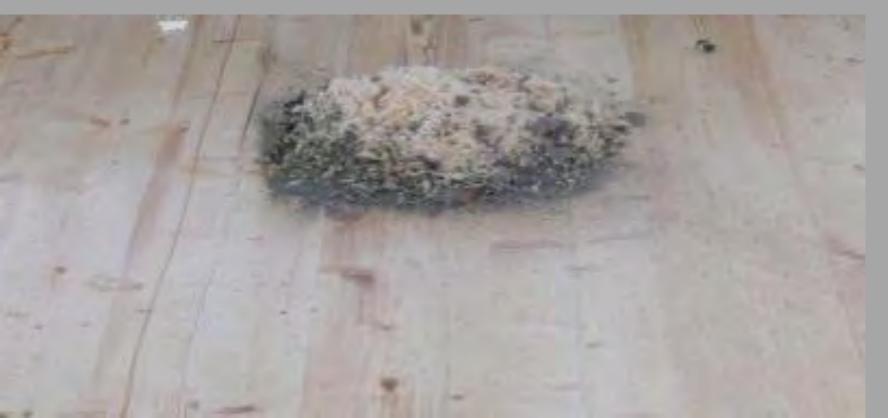
PHOTO CREDIT: ACTON OSTRY ARCHITECTS

^{*}Estimated by the Wood Carbon Calculator for Buildings, based on research by Sathre, R.



MASS TIMBER APPEAL

MINIMAL WASTE





MASS TIMBER ELEMENTS FABRICATED TO TIGHT TOLERANCES

MASS TIMBER APPEAL

PREFABRICATED AND PRECISE



COMPUTER NUMERICALLY CONTROLLED (CNC)
CONNECTIONS

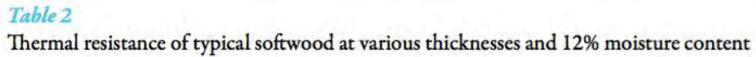




MASS TIMBER APPEAL

ENERGY EFFICIENT





Thickness	1 in. (25 mm)	4 in. (100 mm)	6 in. (150 mm)	8 in. (200 mm)	
R-value (h·ft.2.0F·Btu-1)	1.25	5.00	7.50	10.00	
RSI (m ² ·K·W ⁻¹)	0.22	0.88	1.30	1.80	

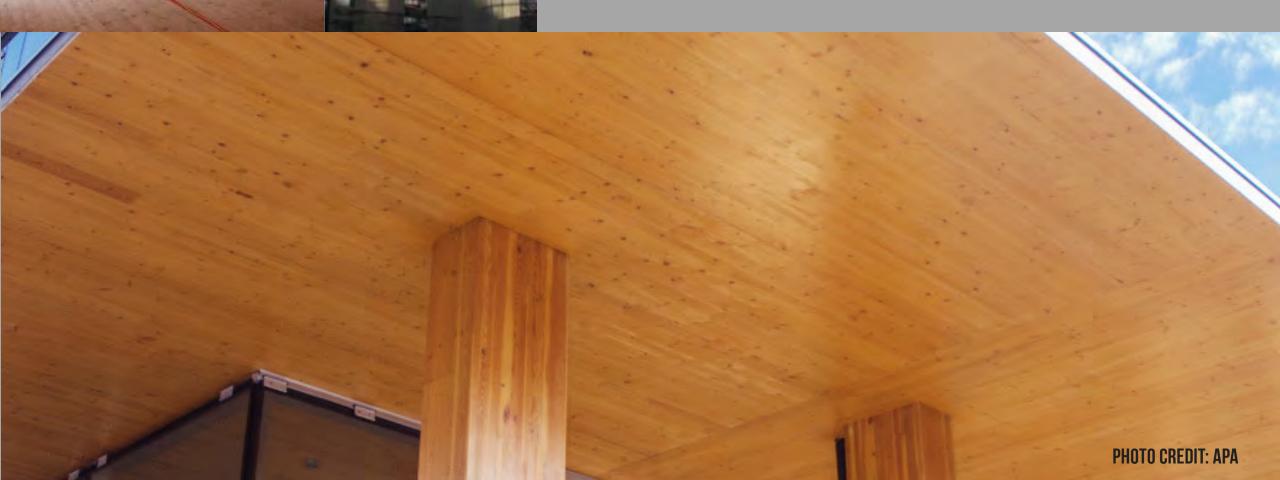
CLT HAS AN R-VALUE OF APPROXIMATELY 1.25 PER INCH OF THICKNESS.

SOURCE: US CLT HANDBOOK 10



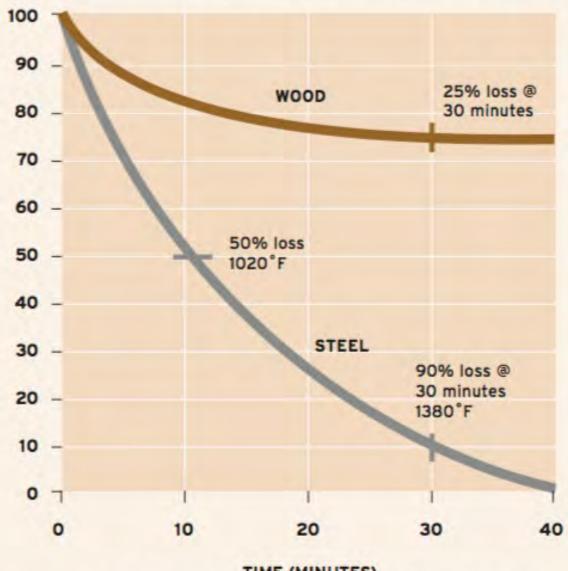
MASS TIMBER APPEAL

STRUCTURAL FLEXIBILITY





COMPARATIVE STRENGTH LOSS OF WOOD VERSUS STEEL



TIME (MINUTES)

Results from test sponsored by National Forest Products
Association at the Southwest Research Institute

SOURCE: AITC

MASS TIMBER DESIGN

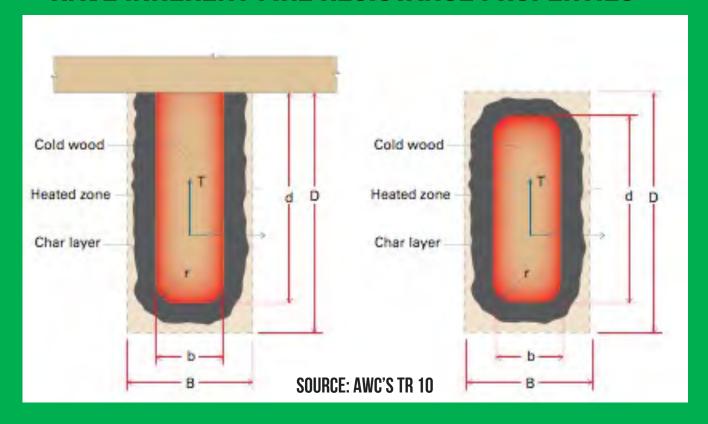
FIRE RESISTANCE

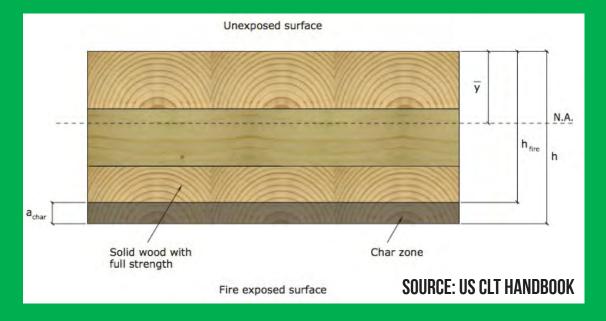


FIRE RESISTANCE

At time At time t = 0Eccentric= f(t) N.A. N.A.

SIMILAR TO HEAVY TIMBER, MASS TIMBER PRODUCTS HAVE INHERENT FIRE RESISTANCE PROPERTIES





FIRE RESISTANCE

Construction type selection dictates prescriptive fire resistance requirements:

- Type IV Construction (minimum sizes)
- Other than type IV: Demonstrated fire resistance:
 - IBC 703.3 allows several options, including:
 - ASTM E119 assembly test
 - Calculations per IBC 722 NDS Chapter 16

FIRE RESISTANCE

Mass timber in other than Type IV Construction:

- IBC 703.3 allows several options, including:
 - ASTM E119 assembly test (none currently exist for NLT)
 - Calculations per IBC 722 NDS Chapter 16

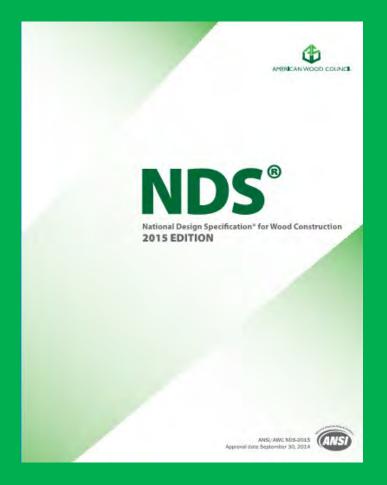
703.3 Methods for determining fire resistance. The application of any of the methods listed in this section shall be based on the fire exposure and acceptance criteria specified in ASTM E119 or UL 263. The required *fire resistance* of a building element, component or assembly shall be permitted to be established by any of the following methods or procedures:

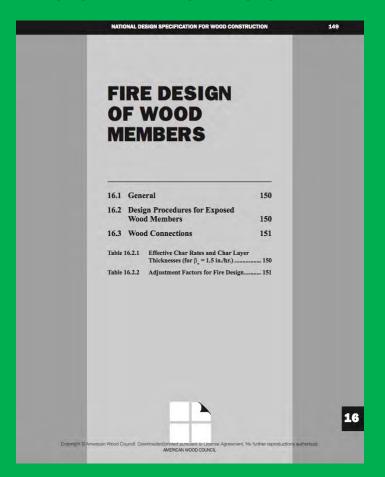
3. Calculations in accordance with Section 722.

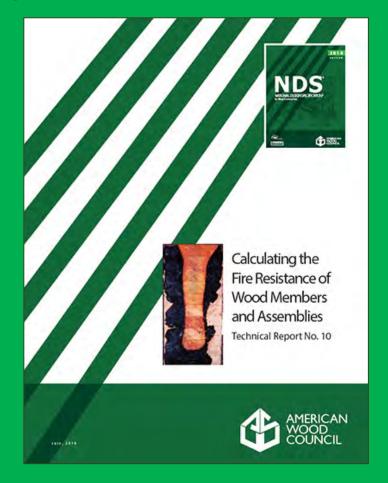
722.1 General. The provisions of this section contain procedures by which the *fire resistance* of specific materials or combinations of materials is established by calculations. These procedures apply only to the information contained in this section and shall not be otherwise used. The calculated *fire resistance* of concrete, concrete masonry and clay masonry assemblies shall be permitted in accordance with ACI 216.1/TMS 0216. The calculated *fire resistance* of steel assemblies shall be permitted in accordance with Chapter 5 of ASCE 29. The calculated *fire resistance* of exposed wood members and wood decking shall be permitted in accordance with Chapter 16 of ANSI/AF&PA *National Design Specification for Wood Construction (NDS)*.

FIRE RESISTANCE

FOR EXPOSED WOOD MEMBERS: IBC 722.1 REFERENCES AWC'S NDS CHAPTER 16 (AWC'S TR 10 IS A DESIGN AID TO NDS CHAPTER 16)









2015 NDS CHAPTER 16 INCLUDES CALCULATION OF FIRE RESISTANCE OF NLT, CLT, GLULAM, SOLID SAWN AND SCL WOOD PRODUCTS

MASS TIMBER DESIGN

FIRE RESISTANCE

Table 16.2.1B Effective Char Depths (for CLT with β_n =1.5in./hr.)

Required Fire Endurance				900	(in.	Depths) esses, h			
(hr.)	5/8	3/4	7/8	1	1-1/4	1-3/8	1-1/2	1-3/4	2
1-Hour	2.2	2.2	2.1	2.0	2.0	1.9	1.8	1.8	1.8
1½-Hour	3.4	3.2	3.1	3.0	2.9	2.8	2.8	2.8	2.6
2-Hour	4.4	4.3	4.1	4.0	3.9	3.8	3.6	3.6	3.6

SOURCE: AWC'S NDS



EDUCATION *

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WHY WOOD?

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How do you demonstrate the fire resistance of exposed wood members using char methods in lieu of gypsum?

Per IBC Sections 703.2 and 703.3, there are multiple ways to demonstrate fire-resistance ratings of structural members and assemblies. One method noted in Section 703.3 is to calculate the rating in accordance with IBC Section 722.

For calculated fire resistance of exposed wood members and decking, IBC Section 722.1 references Chapter 16 of the American Wood Council's National Design Specification® (NDS®) for Wood Construction. This chapter gives a nominal char rate of 1.5 inches of wood thickness per hour of fire resistance. Using the calculations and information contained in NDS Chapter 16, a fire rating of up to 2 hours can be calculated.

The 2015 NDS allows calculated fire resistance of a number of wood

products, including solid sawn lumber (this includes nail laminated timber), glue-laminated timber (glulam), laminated veneer lumber (LVL), parallel strand lumber (PSL), laminated strand lumber (LSL), and cross laminated timber (CLT). For structural members using this method of exposed fire-resistance calculations, the effective char rate, which is slightly higher than the nominal char rate and includes a heat-affected zone, is used. For non-

Additional design considerations for certain structural in NDS Chapter 16. Adjustment factors applied to the allowable design properties of exposed structural members rated for fire resistance are also presented in this chapter.



View All Expert Tips

Project Assistance

Our technical experts offer free project support from design through construction, on issues ranging from allowable heights and areas to structural design, lateral systems and fire- or acoustical-rated assemblies.

Get Assistance >

Ask an Expert

Q: Can exposed wood framing be used in buildings such as aquatic centers and pools? Are there durability concerns?

A: The main durability consideration associated with the use of wood in environments such as pools and aquatic centers is high moisture content in the wood. Buildings such as this may have high relative humidity and, as such, it is important to understand how relative humidity of a condition space affects exposed wood within that space

- Learn More

Have a question? Email Us >

Feature Project

PROJECT: Biomass Heating Plant, Hotchkiss School

FIRE RESISTANCE

AWC'S TECHNICAL REPORT 10 INCLUDES DISCUSSION OF FIRE TESTS AND DESIGN EXAMPLES

4.5 Exposed CLT Floor Example (Allowable Stress Design)

Simply-supported cross-laminated timber (CLT) floor spanning L=18 ft in the strong-axis direction. The design loads are $q_{live} = 80$ psf and $q_{dead} = 30$ psf including estimated self-weight of the CLT panel. Floor decking, nailed to the unexposed face of CLT panel, is spaced to restrict hot gases from venting through half-lap joints at edges of CLT panel sections. Calculate the required section dimensions for a one-hour fire resistance time.

For the structural design of the CLT panel, calculate the maximum induced moment. Calculate panel load (per foot of width):

$$W_{load} = (q_{dead} + q_{live}) = (30 \text{ psf} + 80 \text{ psf})(1 \text{ft width}) = 110 \text{ plf/ft of width}$$

Calculate maximum induced moment (per foot of width):

$$M_{\text{max}} = w_{\text{load}} L^2 / 8 = (110)(18^2)/8 = 4,455 \text{ ft-lb/ft of width}$$

From PRG 320, select a 5-ply CLT floor panel made from 13/8 in x 31/2 inch lumber boards (CLT thickness of 67/8 inches). For CLT grade V2, tabulated properties are:

Bending moment, $F_bS_{eff,0} = 4,675$ ft-lb/ft of width (PRG 320 Annex A, Table A2)

Calculating the Fire Resistance of Wood Members and Assemblies Technical Report No. 10

FREE DOWNLOAD AT AWC.ORG

SOURCE: AWC'S TR 10

FIRE RESISTANCE

MANY SUCCESSFUL CLT FIRE TESTS HAVE BEEN CONDUCTED, BOTH WITH AND WITHOUT GYPSUM **BOARD PROTECTION**



Fire Testing Laboratory



Page 1 of 53

0

TEST REPORT

American Wood Council

222 Catoctin Circle SE. Suite 201 Leesburg, VA 20175

Standard Methods of Fire Tests of Building Construction and Materials

ASTM E 119 - 11a

Test Report No: WP-1950

Assignment No: K-1089

Subject Material: Cross-Laminated Timber and Gypsum Board Wall Assembly (Load-Bearing)

October 4, 2012 Report Date: October 15, 2012

Michael J. Rizzo



REPORT NUMBER: 102891256SAT-001 ORIGINAL ISSUE DATE: February 27, 2017 REVISED DATE: N/A

EVALUATION CENTER

16015 Shady Falls Road Elmendorf, TX 78112 Phone: (210) 635-8100 Fax: (210) 635-8101 www.intertek.com

RENDERED TO

Structuriam Products LP 2176 Government Street Penticton, BC V2A 8B5 Canada

PRODUCT EVALUATED: CrossLam® CLT Un-restrained Load-Bearing Floor/Ceiling Assembly **EVALUATION PROPERTY: Fire Resistance**



Project No. 301006155 Final Report 2012/13

Preliminary CLT Fire Resistance Testing Report

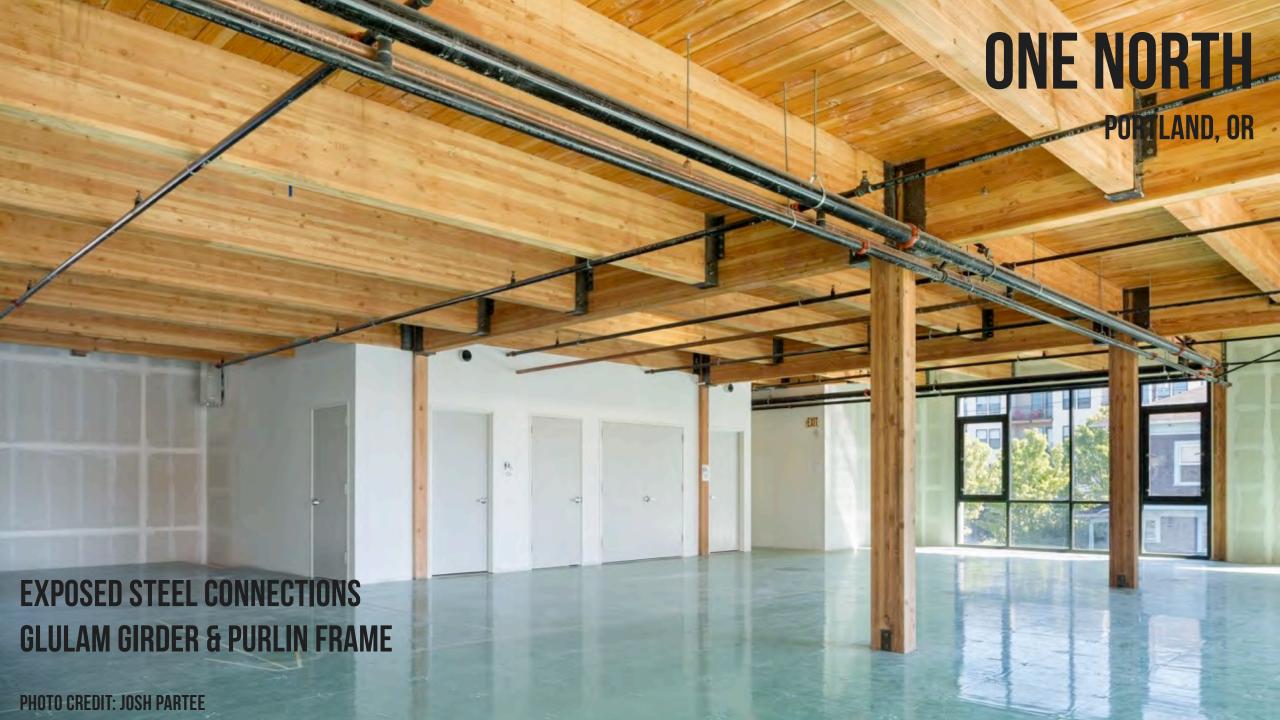
Lindsay Osborne, M.A.Sc. Christian Dagenais, Eng., M.Sc. Scientists Advanced Building Systems - Serviceability and Fire Group

Noureddine Bénichou, Ph.D. Senior Research Officer National Research Council of Canada - Fire Research Resource Centre

July 2012

CONTACT WOODWORKS FOR INFORMATION Leader

Test Methods for Fire Tests of Building Construction and Materials, and CAN/ULC S101, Standard Methods of Fire







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FAY











NET ZERO

LIVING BUILDING CHALLENGE CERTIFIED

TYPE IV CONSTRUCTION

250 YR DESIGN LIFE

COMPLETED 2013

BULLITT CENTER

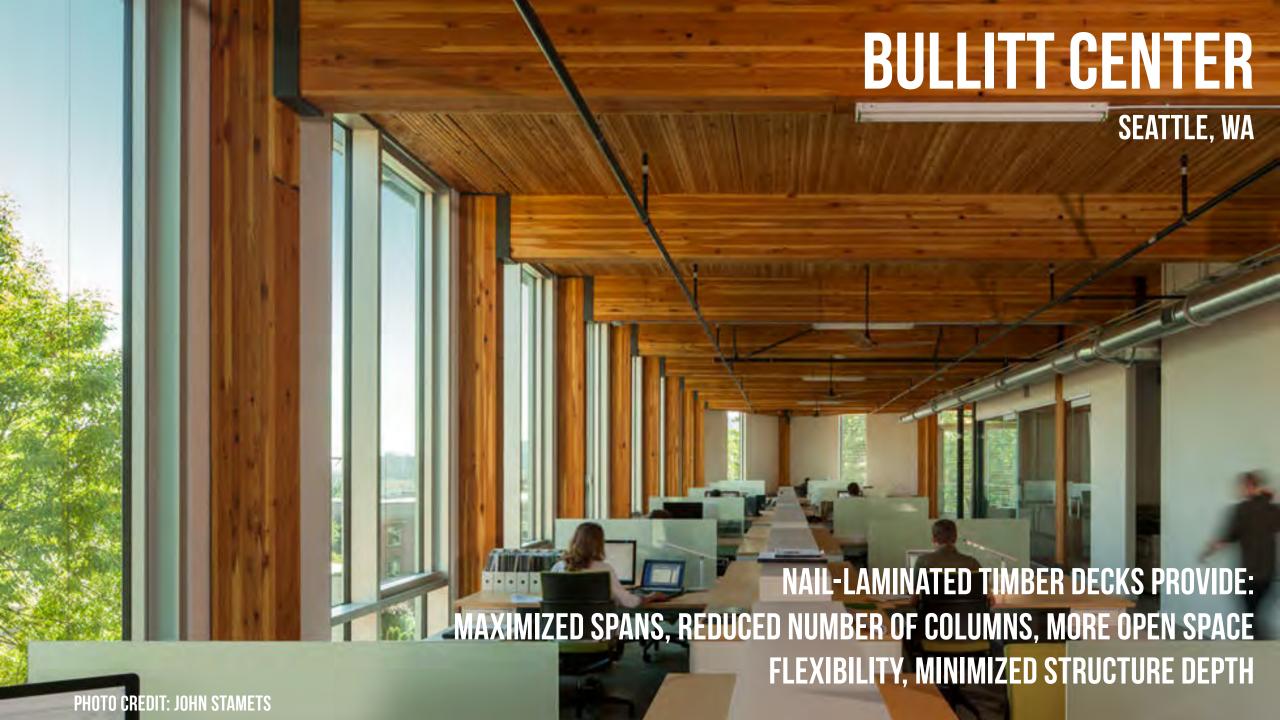
SEATTLE, WA



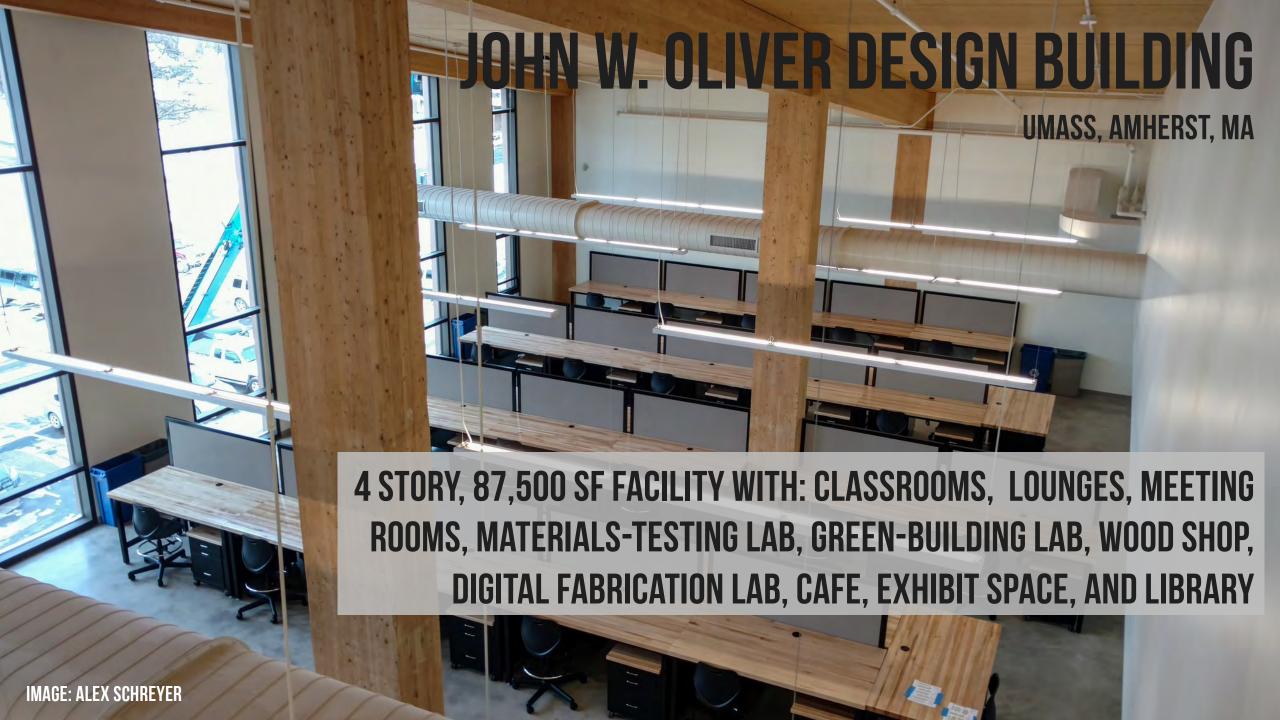
250 YEAR STRUCTURE

HEAVY TIMBER, CONCRETE & STEEL PHOTO CREDIT: MILLER HULL PARTNERSHIP















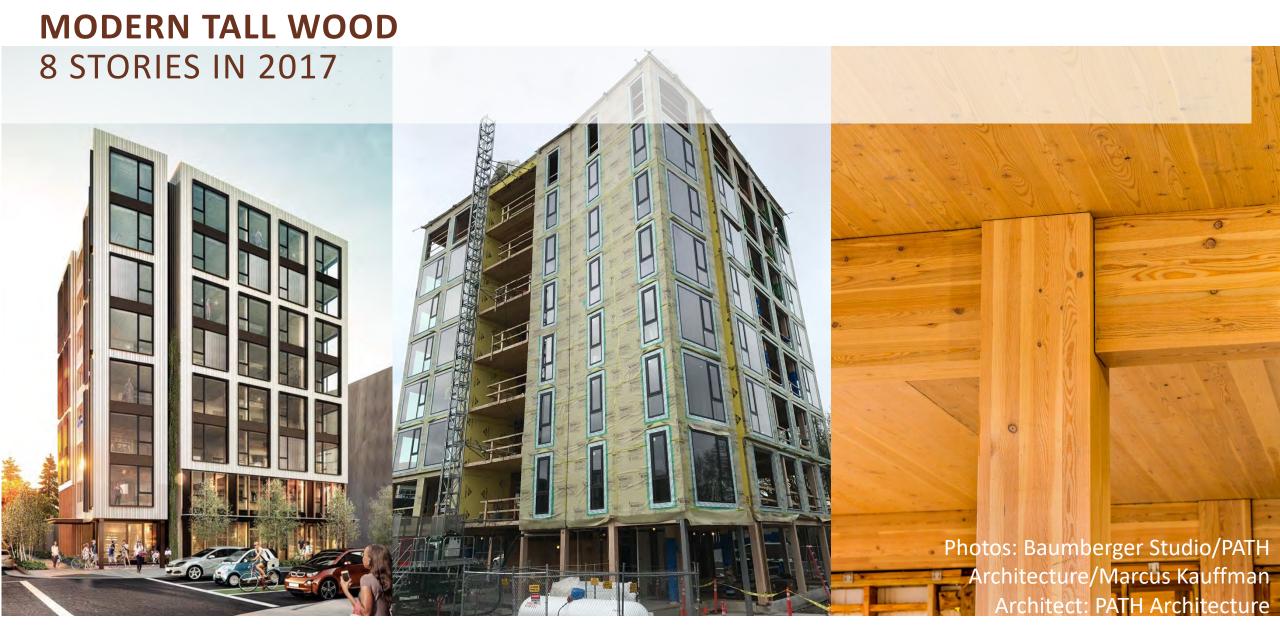


- » Current Prescriptive Code Limit 6 stories or 85 feet
- » Over 6 Stories Alternate Means and Methods Request (AMMR) through performance based design
- » Based on the 1910 Heights and Areas Act

MODERN TALL WOOD 7 STORIES IN 2016



Architect: DLR Group



Carbon12 – 32,000 sf – Portland, OR

New Building Types



18 STORIES
BUILDING HEIGHT 270
ALLOWABLE BUILDING AREA 972,000 SF
AVERAGE AREA PER STORY 54,000SF

TYPE IV-A

128

12 STORIES
BUILDING HEIGHT 180 FT
ALLOWABLE BUILDING AREA 648,000 SF
AVERAGE AREA PER STORY 54,000SF

54,000SF

AVERAGE AREA P

TYPE IV-C

IBC 2021

TYPE IV-B



9 STORIES
BUILDING HEIGHT 85'
ALLOWABLE BUILDING AREA 405,000 SF
AVERAGE AREA PER STORY 45,000 SF

324,000 SF ALLOWABLE BUILDING AREA 54,000 SF AVERAGE AREA PER STORY 6 STORIES MAXIMUM 85' -0" MAXIMUM BUILDING HEIGHT 324.00 SF MAXIMUM AREA TYPE IV- HT **IBC 2015**

BUSINESS OCCUPANCY [GROUP B]

*BUILDING FLOOR-TO-FLOOR HEIGHTS ARE SHOWN AT 12'-0" FOR ALL EXAMPLES FOR CLARITY IN COMPARISON BETWEEN 2015 TO 2021 IBC CODES.

New Building Types

Tall Mass Timber Type of Construction Comparison

Feature	Type IVA	Type IVB	Type IVC				
Description of new Type IV Types	100 % Noncombustible (NC) protection on all surfaces of Mass Timber (MT).	NC protection on all surfaces of mass timber (MT) except for limited exposed areas.	Exposed mass timber (MT), except shafts, concealed spaces, and outside of exterior walls.				
Permitted Materials							
Structural Building Elements	MT or NC	MT or NC	MT or NC				
Nonloadbearing Ext Walls	MT or NC	MT or NC	MT or NC				
Nonloadbearing Int Walls	MT or NC	MT or NC	MT or NC				
Exit and Hoistway Enclosures							
Highrise* to 12 stories or 180 feet:	NC or MT protected with 2 (or 3 when 3 hr FRR) layers of 5/8" Type X.	NC or MT protected with 2 layers of 5/8" Type X gypsum or equiv each side of enclosure.	NC or MT protected with one layer of 5/8" Type X gypsum each side of shaft or enclosure.				
Above 12 stories or 180 ft: *see IBC definition of highrise	NC	Not Permitted	Not Permitted				

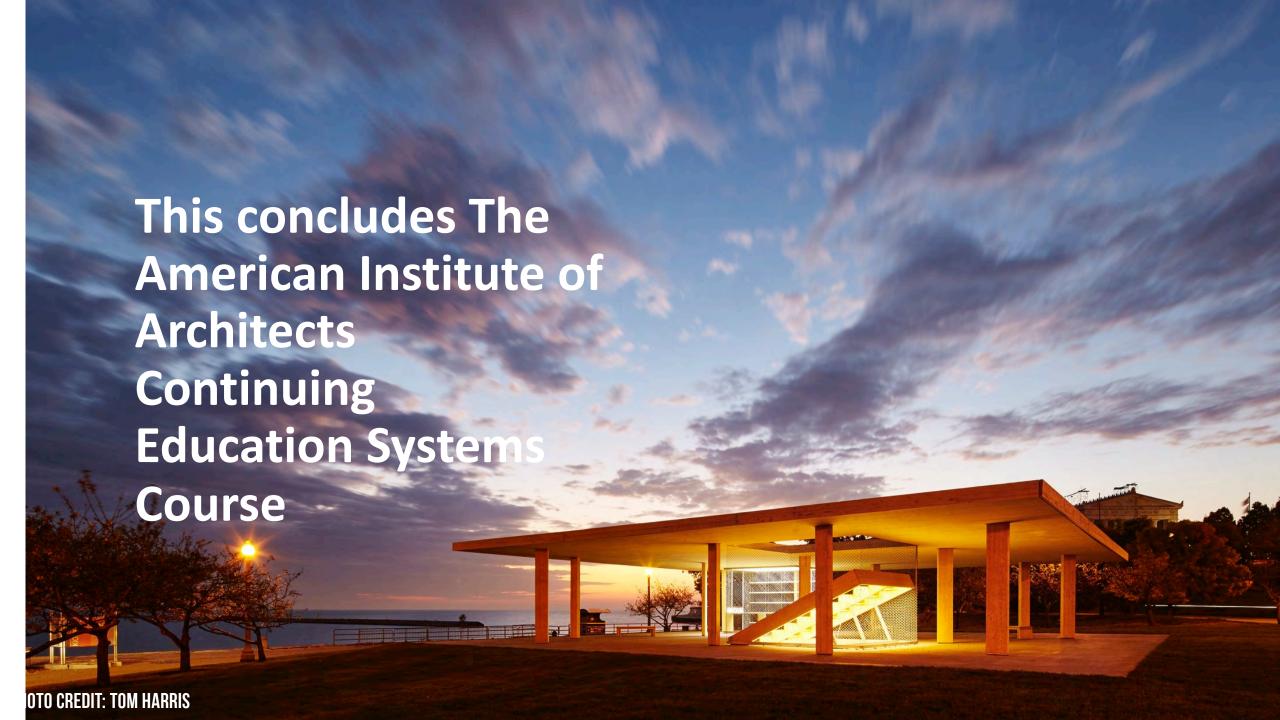
Source: AWC https://www.awc.org/tallmasstimber

Tall Mass Timber Type of Construction Comparison

Feature	Type IVA	Type IVB	Type IVC			
Noncombustible Protection						
Interior Protection						
Req'd 3 hr FRR	3 layers of 5/8" Type X gypsum	Same as Type IV-A for protected MT. Limited exposed MT elements must	All MT is allowed to be exposed except for 1 layer of 5/8" Type X on outside			
Req'd for 2 hr or less FRR	2 layers of 5/8" Type X gypsum	have same FRR, but may be calculated.	surfaces of exterior walls, inside and outside of shafts and exit enclosures, and in concealed spaces.			
Exterior Protection	Minimum of 1 layer of 5/8" Type X gypsum	Minimum of 1 layer of 5/8" Type X gypsum	Minimum of 1 layer of 5/8" Type X gypsum			
Floor Surface	1 inch of NC protection	1 inch of NC protection	No protection required			
Roof	No NC protection on exterior roof surface, 2 layers of 5/8" Type X gypsum on inside of roof deck.	No NC protection on exterior roof surface, 2 layers of 5/8" Type X gypsum on inside of roof deck.	No protection on roof surface or inside of roof deck is required (unless concealed space).			
Concealed Spaces	No exposed MT in concealed spaces. NC protection in concealed spaces.	No exposed MT in concealed spaces. NC protection in concealed spaces.	No exposed MT in concealed spaces. One layer of 5/8" Type X gypsum NC protection in concealed spaces.			
Table 601 Fire Resistance Rating (FRR)						
Primary Frame or						
Bearing Wall:	3 hr FRR (2 hr at roof)	2 hr FRR (1 hr at roof)	2 hr FRR (1 hr at roof)			
Floors:	2 hr FRR	2 hr FRR	2 hr FRR			
Roof:	1.5 hr FRR	1 hr FRR	1 hr FRR			
Fire Resistance Rating Trade-off	No FRR reduction for sprinkler in 403.2.1	No FRR reduction for sprinkler in 403.2.1	No FRR reduction for sprinkler in 403.2.1			

Source: AWC https://www.awc.org/tallmasstimber







ANY QUESTIONS?



Keep your **regional staff member** in mind for questions and support:

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Project Support Field Division, East
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