



Introduction to Wood: Design Best Practices

Anthony Harvey, PE
Regional Director





Jeff Stefani, PE
Jeff.Stefani@canfor.com

Glue-Lam Erectors, Inc.

Kyle Heminger
gluelamerectors@earthlink.net

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- Structural detailing of wood-frame and hybrid material systems
- Fire resistance and acoustical-rated assemblies
- Efficient and code-compliant lateral system design
- Alternate means of code compliance
- Energy-efficient detailing
- Application of advanced building systems and technologies



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ANY QUESTIONS?

Anthony Harvey, PE

Regional Director

(513) 222-3038

anthony.harvey@woodworks.org



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Questions related to specific materials, methods, and services will be addressed at the conclusion of this presentation.



Course Description

This presentation will provide an introductory review of allowable uses and best design practices associated with wood-frame construction. Design and detailing best practices will be explained to highlight factors that play important roles in the construction process and ultimate building performance. Discussion will also include optimal framing layouts, spacings and spans, truss design considerations, connection detailing and load path continuity.



Learning Objectives

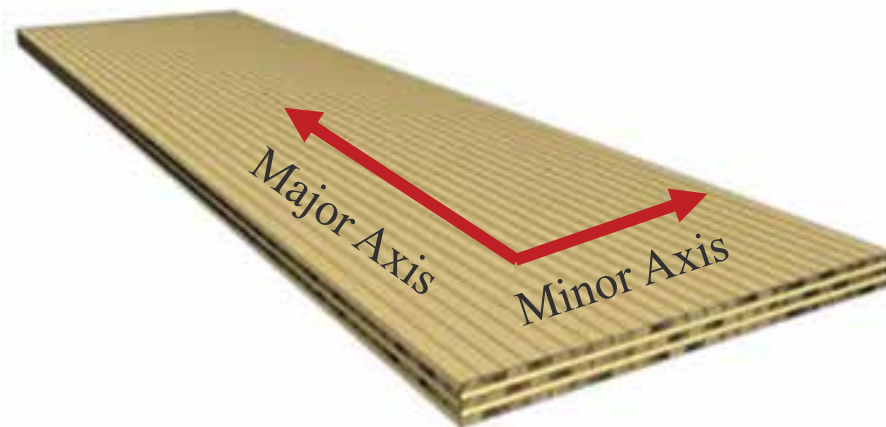
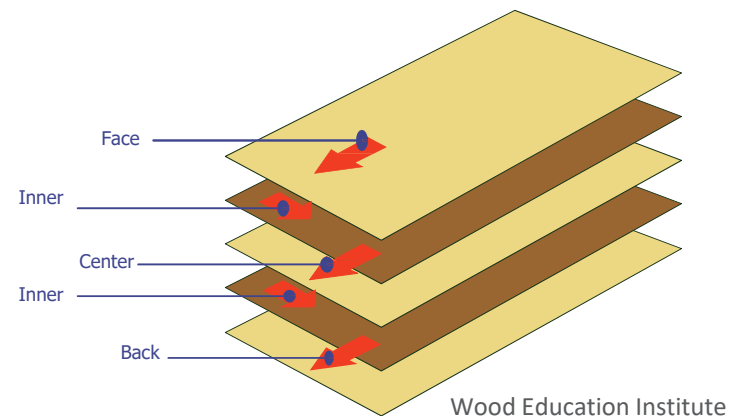
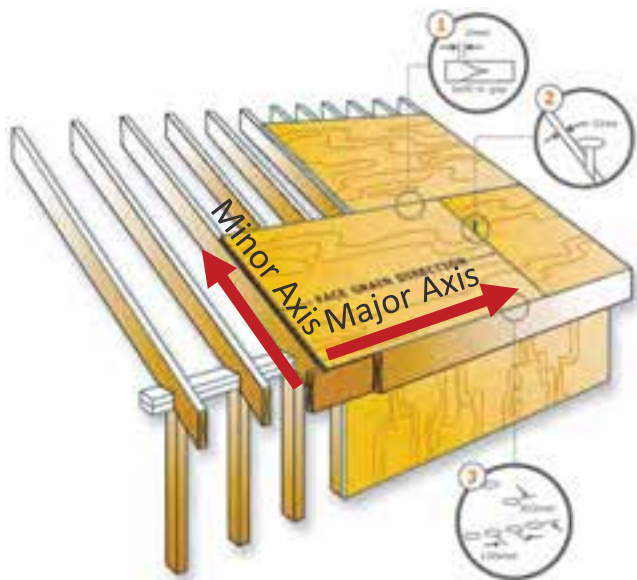
1. Discuss design considerations specific to wood framing such as moisture control and shrinkage accommodation.
2. Review design topics associated with wood trusses such as loading conditions and bracing requirements.
3. Analyze the impact that holes and notches have on structural wood members such as floor joists and wall studs and determine best locations for minimizing their impact on structural design properties.
4. Explore the variety of available wood building products and discuss how to efficiently utilize each.

Outline

- Framing: Members & Layout
- Load Path Continuity
- Localized Stresses: Notches & Holes
- Blocking & Bracing
- Trusses
- Connections
- Moisture Best Practices

Member Orientation: Panels

- Due to multi-layer composition, most panel products have a major and minor axis
- Direction of grain in face plies/veneers generally determine direction of major axis



Panel Options

Wood Structural Panels

Oriented Strand Board (OSB) and Plywood

Solid Sawn Decking

2x, 3x and 4x dimensional lumber

Mechanically Laminated or Tongue & Groove

Panel Products:

Mass timber panels (CLT, NLT, DLT, GLT, MPP)

Structural Insulated Panels (SIPS)

Wood Structural Panels (WSP)

- Plywood - the original engineered wood panel (>100 years)
- Oriented Strand Board (OSB) - a second generation engineered wood panel
- Depending on thickness, loads, generally span 16" – 24"



Lumber Decking

- Code Acceptance for 2, 3 and 4 inch thick Tongue & Groove Lumber Decking per IBC 2012 2304.8
- Depending on thickness, loads, can span 8 ft+



3x6 Double T&G Pine
Photo Southern Wood Specialties

Mass Timber Panels

Nail Laminated Timber (NLT)



Glue Laminated Timber (GLT)



Mass Plywood Panel (MPP)



Cross Laminated Timber (CLT)



Dowel Laminated Timber (DLT)

Member Orientation: Framing

- Most framing members have a major and minor axis
- Most framing members are rectangular; larger dimension generally determines direction of major axis



Member Orientation: Framing



Framing Options

Solid Sawn Lumber

Lumber (2"-4" thickness) and Timbers (5"x5" & larger)

Glulam

I Joists

Structural Composite Lumber

LVL, PSL, LSL, OSL



Image Source: APA

Solid Sawn Framing

Design Routes:

IBC 2012 2308 – Conventional Light-Frame Construction

AWC NDS 2012/2015 – Engineered Design

AWC Wood Frame Construction Manual 2012/2015

- Design tools based upon NDS

Single or Multi-ply members can be used for floor, roof joists, beams, purlins, wall studs, plates, blocking, bracing



Solid Sawn Roof Framing Spans

IBC 2015 Table 2308.7.2(2) Rafter Spans for Common Lumber Species

TABLE 2308.7.2(2)—continued
RAFTER SPANS FOR COMMON LUMBER SPECIES
 (Roof Live Load = 20 psf, Ceiling Attached to Rafters, $L/\Delta = 240$)

RAFTER SPACING (inches)	SPECIES AND GRADE		DEAD LOAD = 10 psf					DEAD LOAD = 20 psf				
			2 × 4	2 × 6	2 × 8	2 × 10	2 × 12	2 × 4	2 × 6	2 × 8	2 × 10	2 × 12
			Maximum rafter spans*									
			(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)
24	Douglas Fir-Larch	SS	8-3	13-0	17-2	21-10	Note b	8-3	13-0	16-7	20-3	23-5
	Douglas Fir-Larch	#1	8-0	12-6	15-10	19-5	22-6	7-5	10-10	13-9	16-9	19-6
	Douglas Fir-Larch	#2	7-10	11-9	14-10	18-2	21-0	6-11	10-2	12-10	15-8	18-3
	Douglas Fir-Larch	#3	6-1	8-10	11-3	13-8	15-11	5-3	7-8	9-9	11-10	13-9
	Hem-Fir	SS	7-10	12-3	16-2	20-8	25-1	7-10	12-3	16-2	19-10	23-0
	Hem-Fir	#1	7-8	12-0	15-6	18-11	21-11	7-3	10-7	13-5	16-4	19-0
	Hem-Fir	#2	7-3	11-5	14-8	17-10	20-9	6-10	10-0	12-8	15-6	17-11
	Hem-Fir	#3	6-1	8-10	11-3	13-8	15-11	5-3	7-8	9-9	11-10	13-9
	Southern Pine	SS	8-1	12-9	16-10	21-6	Note b	8-1	12-9	16-10	20-10	24-8
	Southern Pine	#1	7-10	12-3	16-2	18-11	22-6	7-5	11-1	14-0	16-5	19-6
	Southern Pine	#2	7-4	11-0	13-11	16-6	19-6	6-4	9-6	12-1	14-4	16-10
	Southern Pine	#3	5-8	8-4	10-6	12-9	15-1	4-11	7-3	9-1	11-0	13-1
	Spruce-Pine-Fir	SS	7-8	12-0	15-10	20-2	24-7	7-8	12-0	15-4	18-9	21-9
	Spruce-Pine-Fir	#1	7-6	11-9	14-10	18-2	21-0	6-11	10-2	12-10	15-8	18-3
	Spruce-Pine-Fir	#2	7-6	11-9	14-10	18-2	21-0	6-11	10-2	12-10	15-8	18-3
	Spruce-Pine-Fir	#3	6-1	8-10	11-3	13-8	15-11	5-3	7-8	9-9	11-10	13-9

Assumes Roof Live Load = 20 psf, $\Delta = L/240$

Solid Sawn Roof Framing Spans

Rafter Spacing	Species and Grade	Size	Maximum Rafter Span
16"	Douglas-Fir Larch #2	2x8	18'-2"
		2x10	22'-3"
		2x12	25'-9"
24"	Spruce-Pine- Fir #2	2x8	14'-10"
		2x10	18'-2"
		2x12	21'-0"

Assumes Roof Live Load = 20 psf, Roof Dead Load = 10 psf, $\Delta = L/240$

Glulam

Glulam = a structural composite of lumber and adhesives

Recognized in IBC 2303.1.3 using ANSI/AITC A 190.1 and ASTM D 3737

Can be used for floor, roof purlins, beams, arches, columns

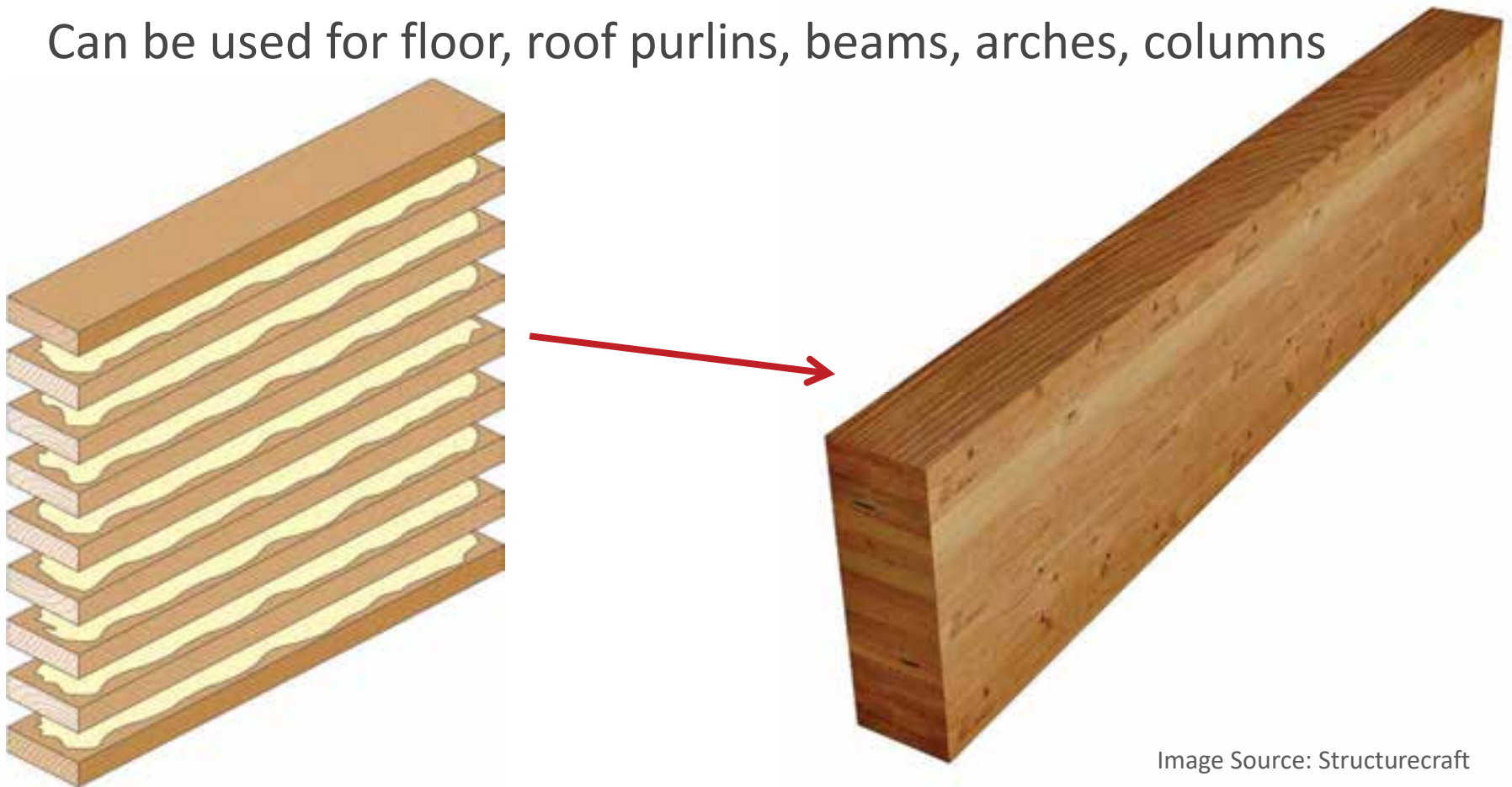


Image Source: Structurecraft

Glulam Sizes & Options

Typical Widths:

3-1/8", 3-1/2", 5-1/8", 5-1/2", 6-3/4", 8-3/4", 10-3/4", 12-1/4"

Typical Depths:

Increments of 1½" from 6" to 60"+

Note: Some glulam laminations are 1-3/8"

Typical Species

Douglas-Fir, Southern Pine, Spruce

Can be PT, or FRT, varies by manufacturer & treater

Can be cambered

Different Appearance Grades available

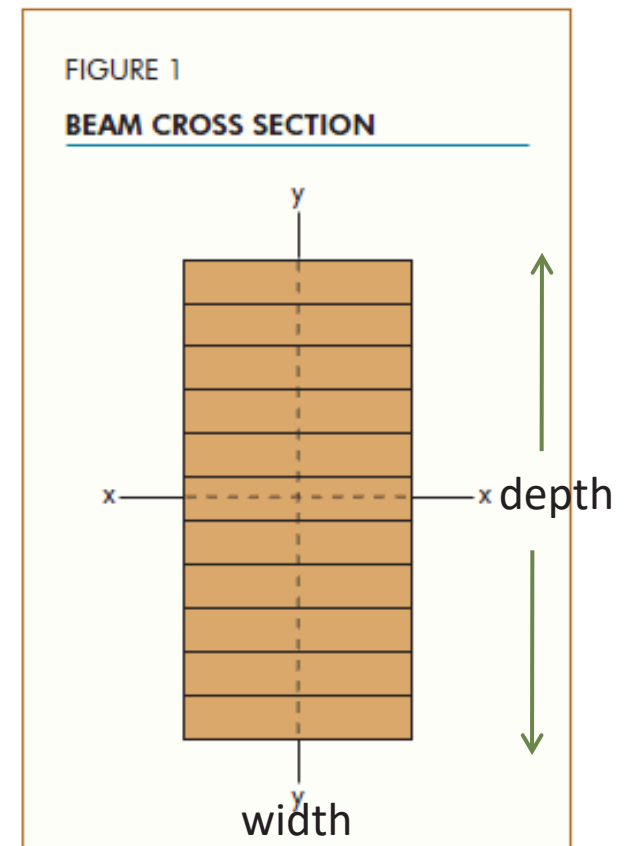


Image: APA Glulam Product Guide

Glulam Layup – Balanced & Unbalanced

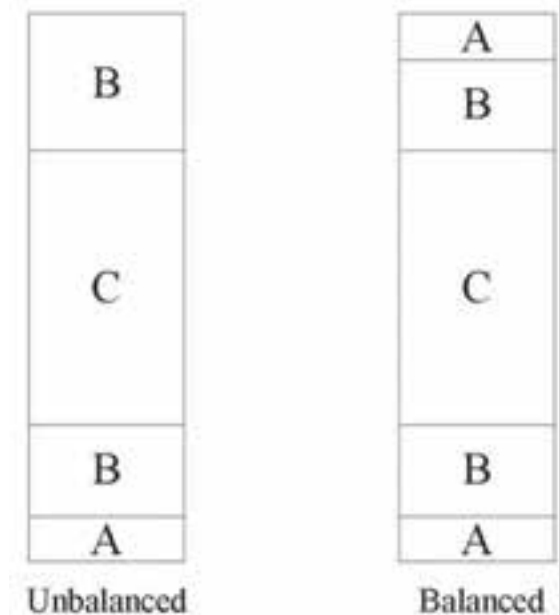
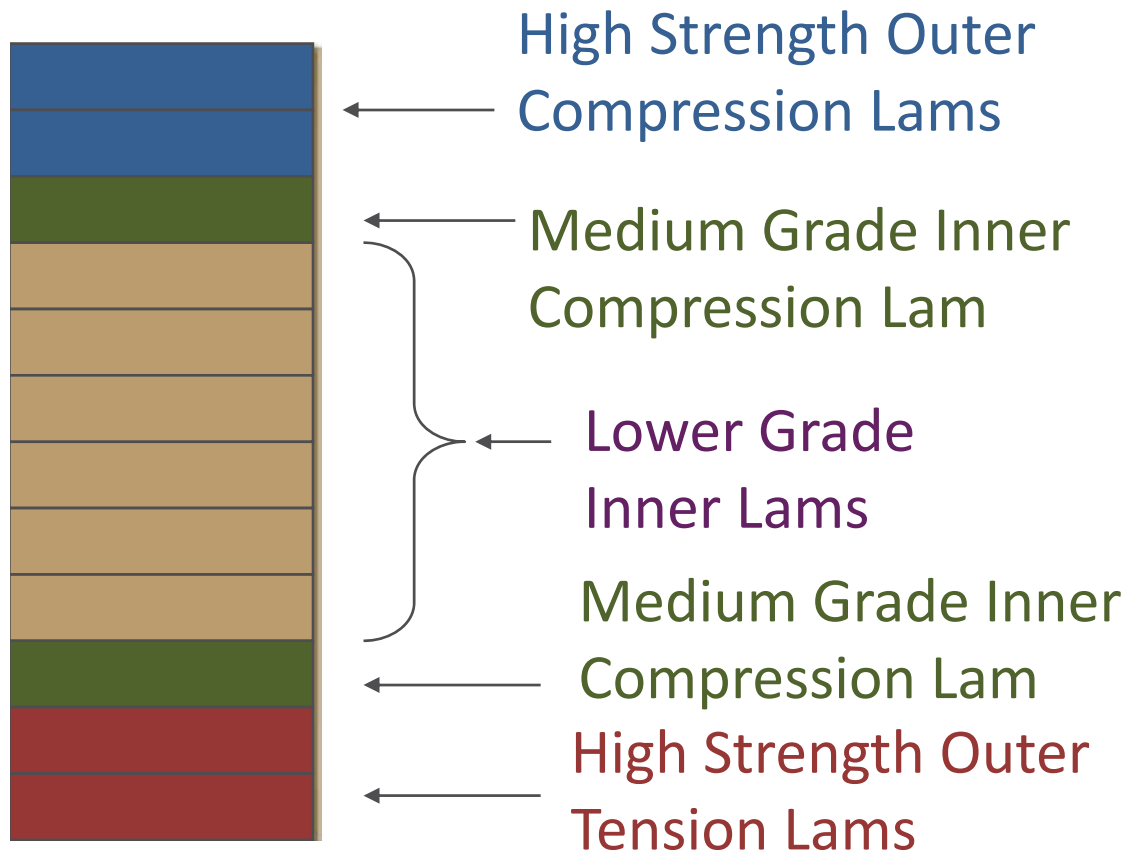
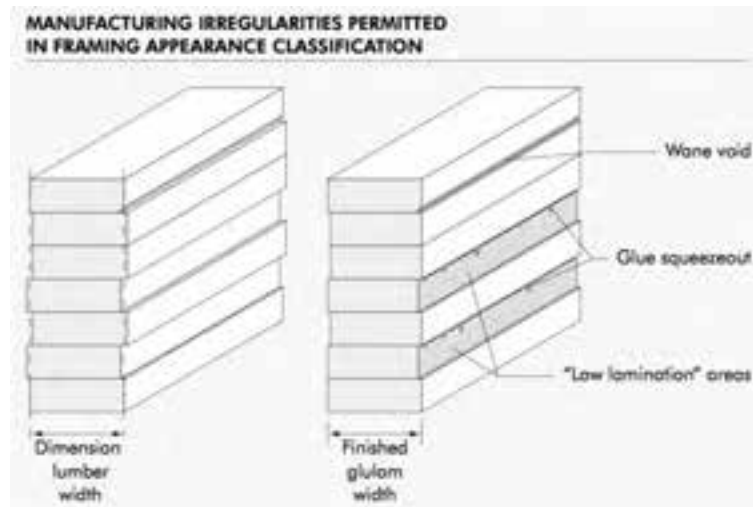


Image: AITC

Glulam Appearance Grades – APA EWS Y110



Framing Grade



Industrial Grade



Architectural Grade



Premium Grade

Images: American Laminators

Specifying Glulam Camber

- Glulam can be manufactured with camber to offset the anticipated dead load deflection
- Very important for long span members
- Glulam industry recommends camber = 1.5 times calculated dead load deflection (roof beams); 1.0 times (floor)

FIGURE 2

BEAM CAMBER PARAMETERS

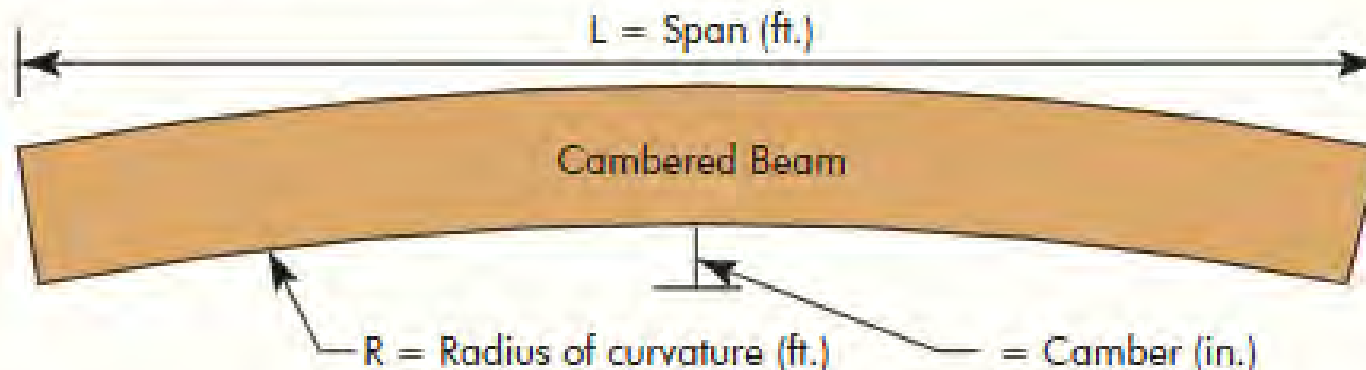
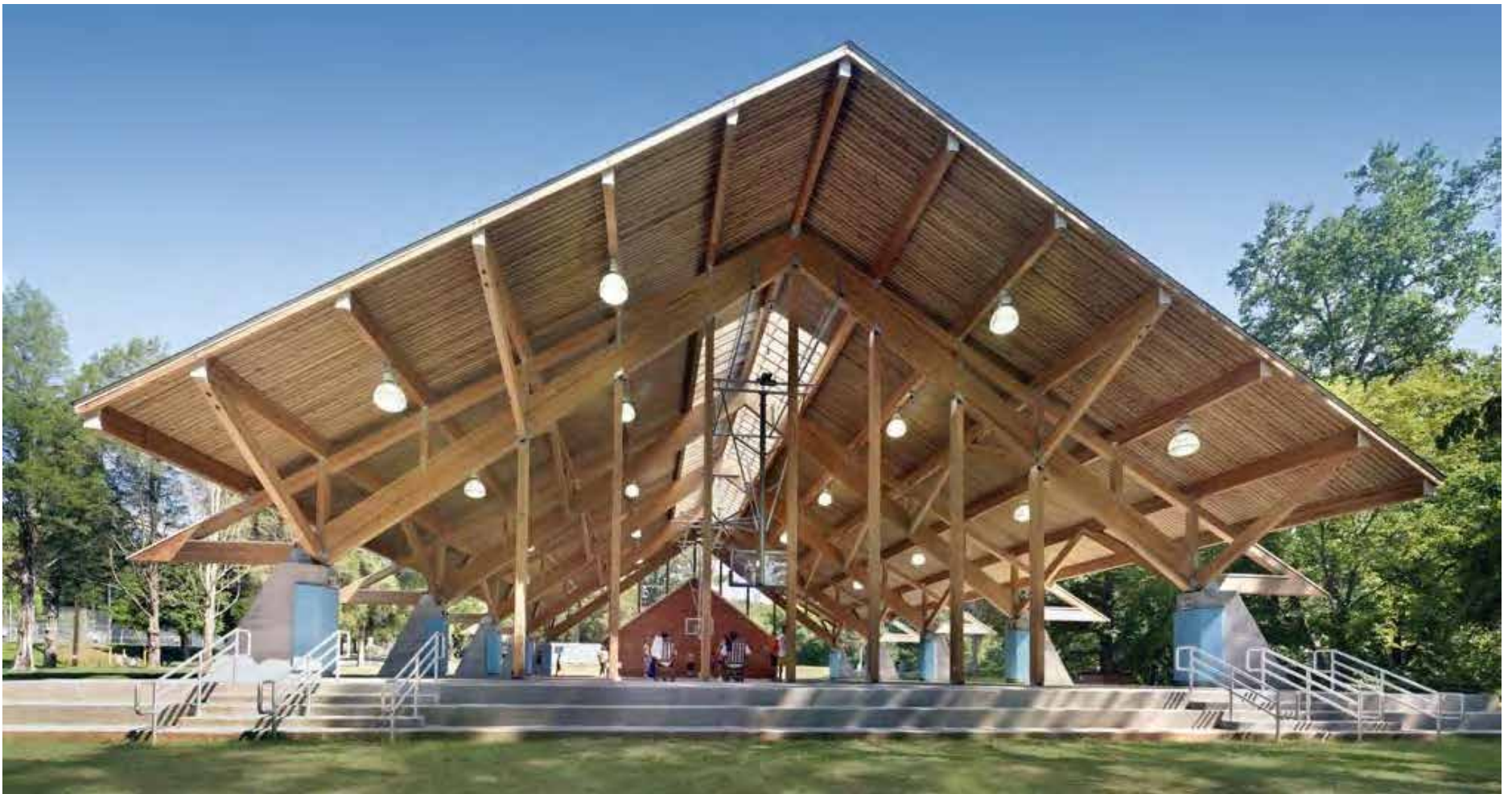


Image: APA Glulam Product Guide

Spans of 100 feet or greater achievable



Flexibility of Shapes and Spans



YMCA Camp Thunderbird Duke Energy Pavilion, Lake Wylie, South Carolina
Photo: Stanley Capps

Flexibility of Shapes and Spans



Richmond Olympic Oval, Richmond, BC Photo: naturallywood.com

Flexibility of Shapes and Spans



The Cathedral of Christ The Light, Oakland, CA
Photo: Timothy Hursley, Cesar Rubio, and John Blaustein,

Flexibility of Shapes and Spans



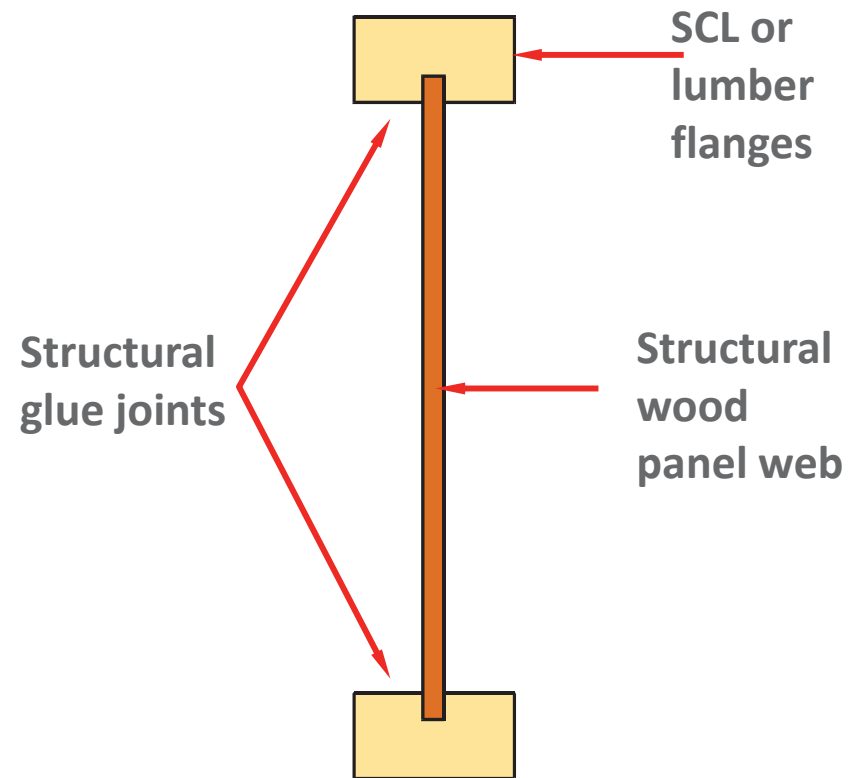
Lemay Museum, Tacoma, WA
Photo: Western Wood Structures

Pre Fabricated Wood I-Joists

Recognized in IBC 2303.1.2 using ASTM D 5055

Proprietary Products with Evaluation Reports

Can be used for floor, roof joists



Structural Composite Lumber (SCL)

Recognized in IBC 2012 2303.1.9 / IBC 2015 2303.1.10 using ASTM D 5456

Proprietary Products with Evaluation Reports

Can be used for floor, roof beams & columns, tall wall studs



Laminated Veneer Lumber



Parallel
Strand
Lumber

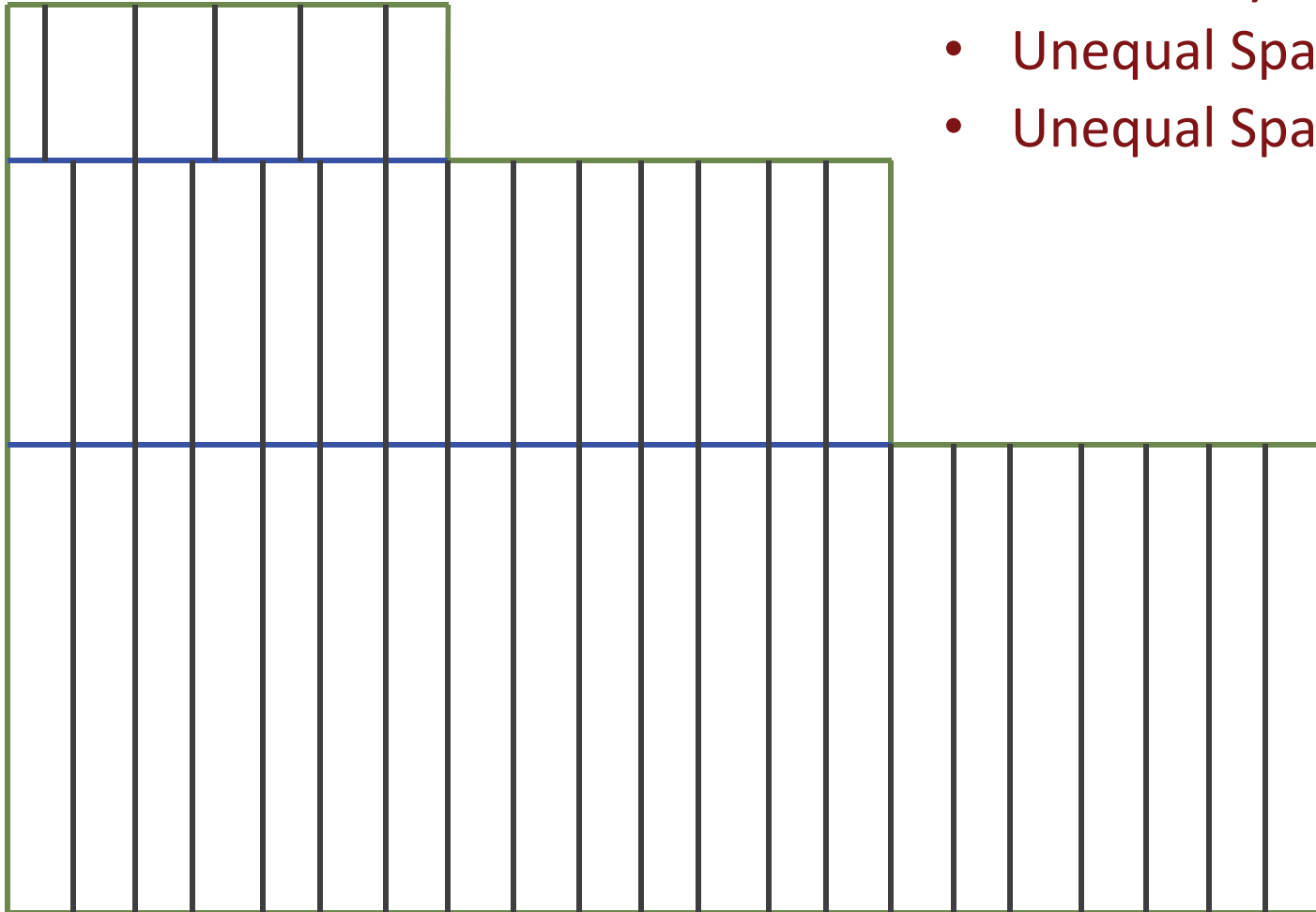


Laminated
Strand
Lumber



Oriented
Strand
Lumber

Framing: Layout, Spacing, Spans



Less Ideal Layout

- Unequal Spans
- Unequal Spacing

[illegible]

Ideal Layout

- Equal Spans
- Equal Spacings
- Avoid Stiff-Soft Areas
- Repetition
- Efficiency

Load Path Continuity

- Structural loads (gravity and lateral) must follow a continuous load path from roof to foundation
- In multi-story construction, to work around building features, may require numerous load path “steps” in lieu of a single vertical load path



Load Path Continuity: Results & Impacts

- In many situations, general building layout is complete before structural design begins
- Understanding impacts of wall, column, opening layout on structural system efficiency will aid in providing more cost effective solutions



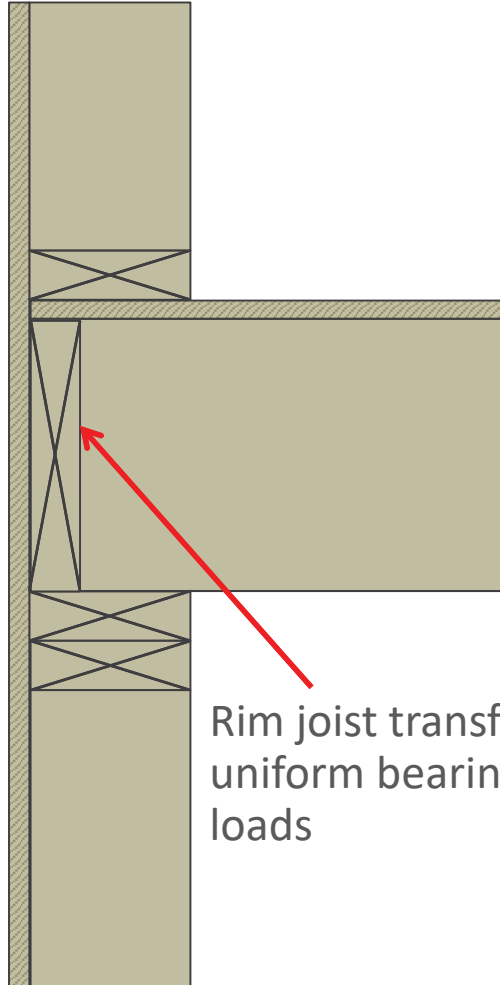
Library Square, British Columbia, Canada
JM Architects



Wood Cube Apartments
Photo: IBA Hamburg

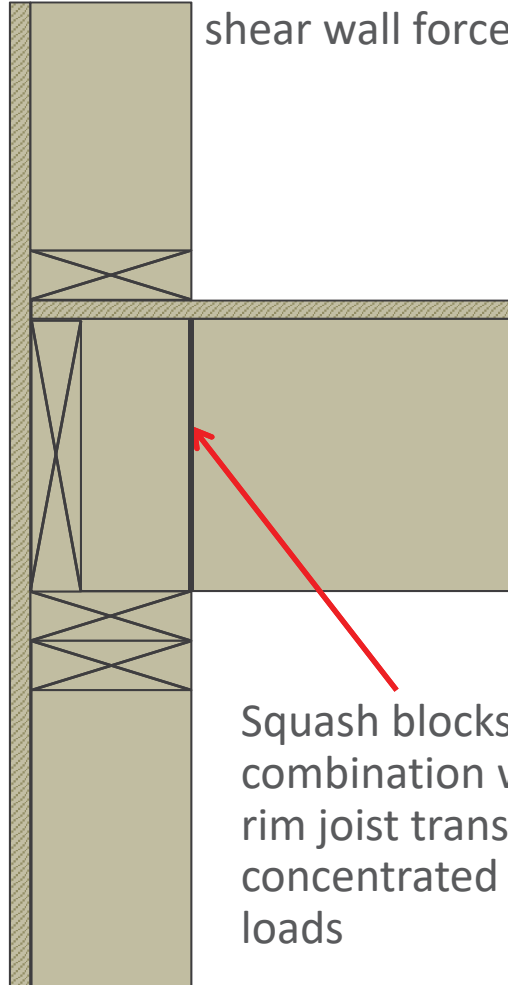
Load Path Continuity: Wall Loads

Uniform wall load



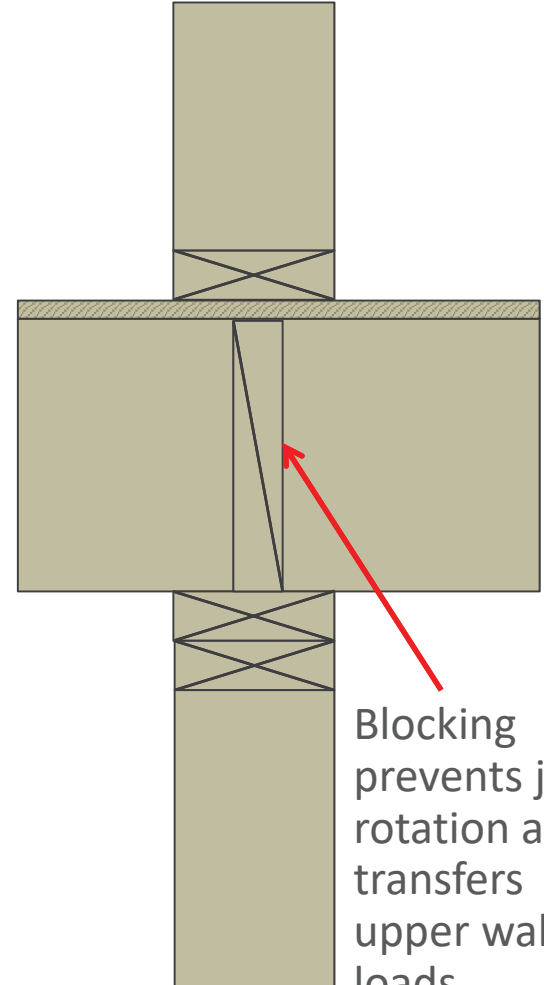
Rim joist transfers
uniform bearing wall
loads

Concentrated post
load (gravity or
shear wall forces)



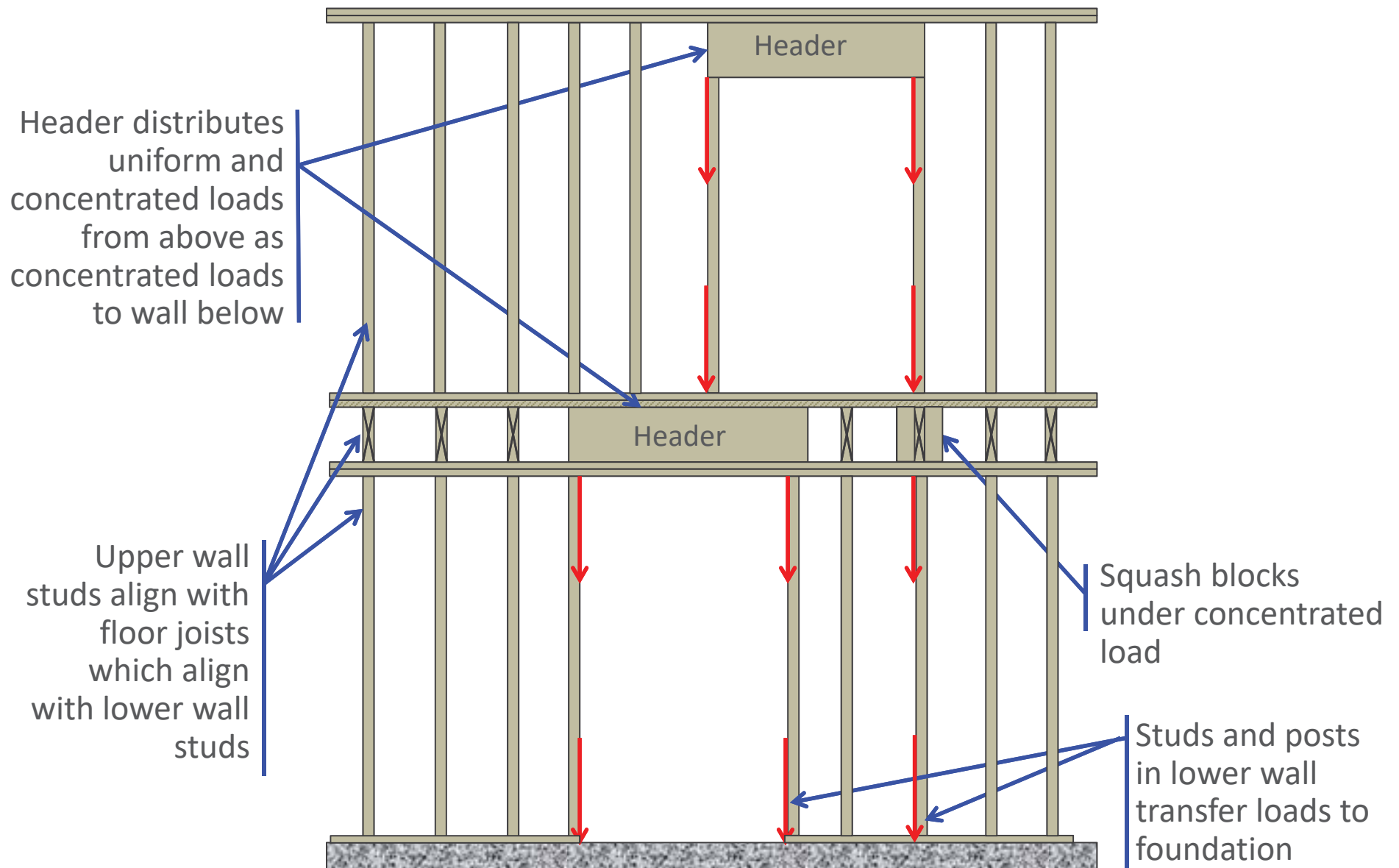
Squash blocks in
combination with
rim joist transfer
concentrated post
loads

Uniform wall load

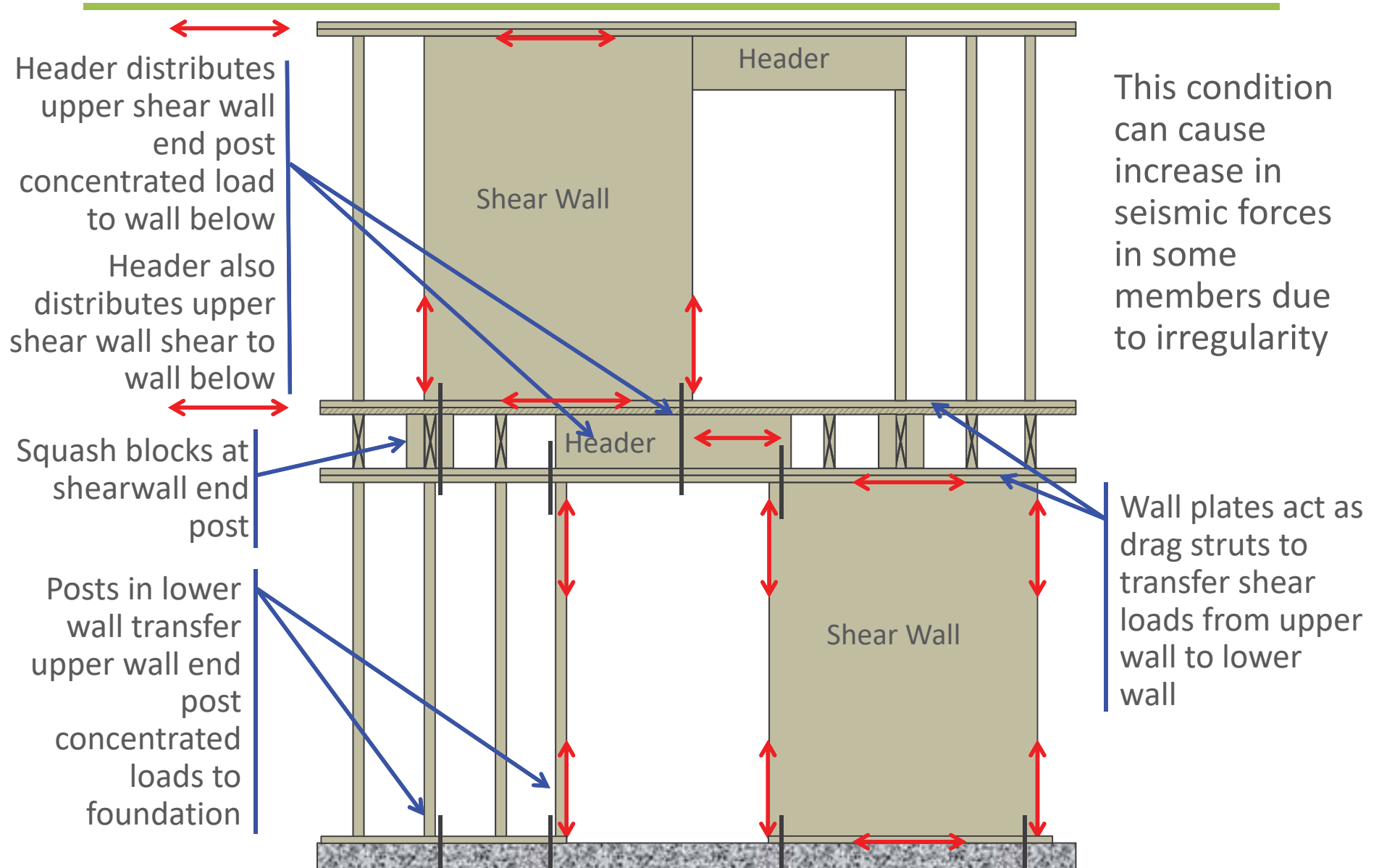


Blocking
prevents joist
rotation and
transfers
upper wall
loads

Gravity Load Path Continuity: Wall Elevation



Lateral Load Path Continuity: Wall Elevation



Spanning Members: Localized Stress

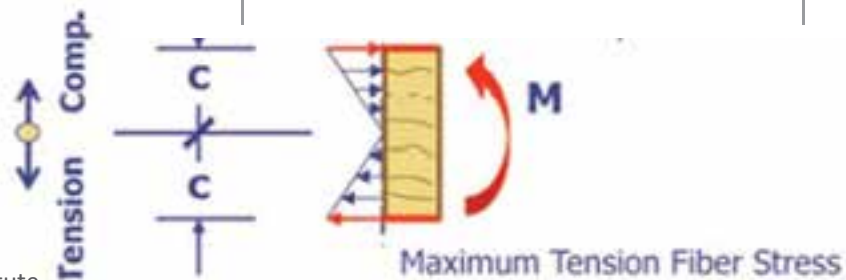
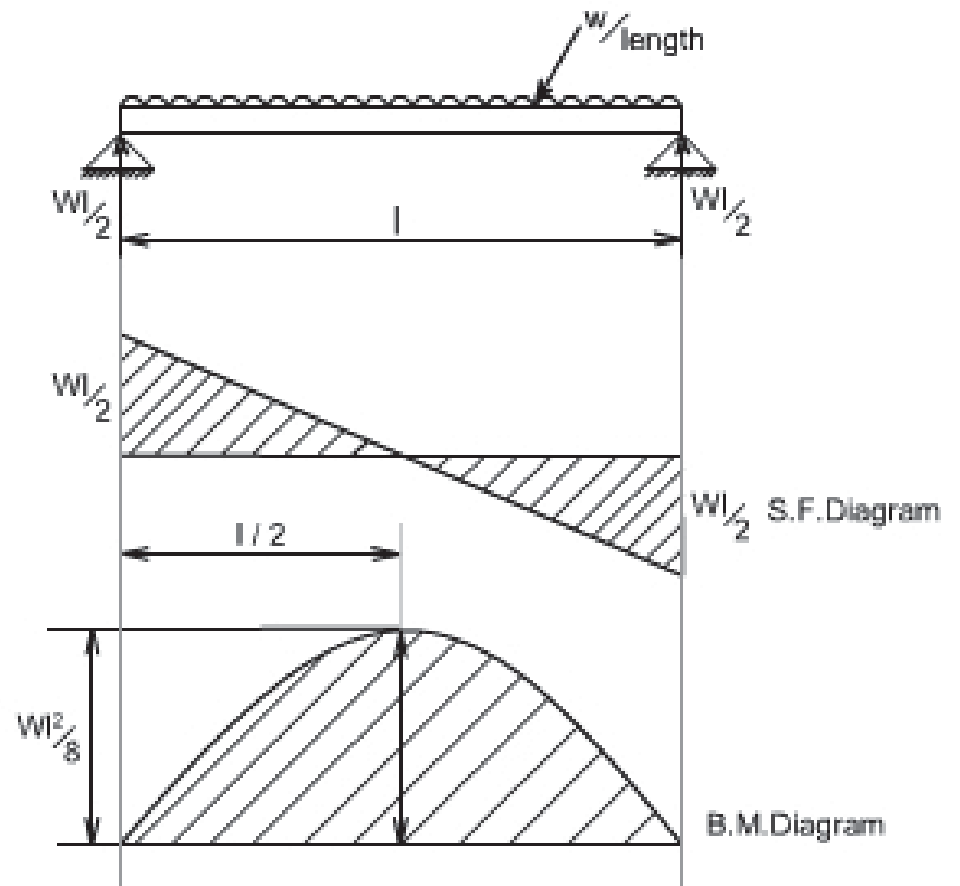
Under common, simple span, uniform loading conditions:

Moment: max. in cross section at outer fibers, max. in span at center

- Avoid notches and holes in middle of beam span, top and bottom of beam depth

Shear: max. in cross section at center, max. in span at ends

- Avoid large notches and holes in ends of beam span

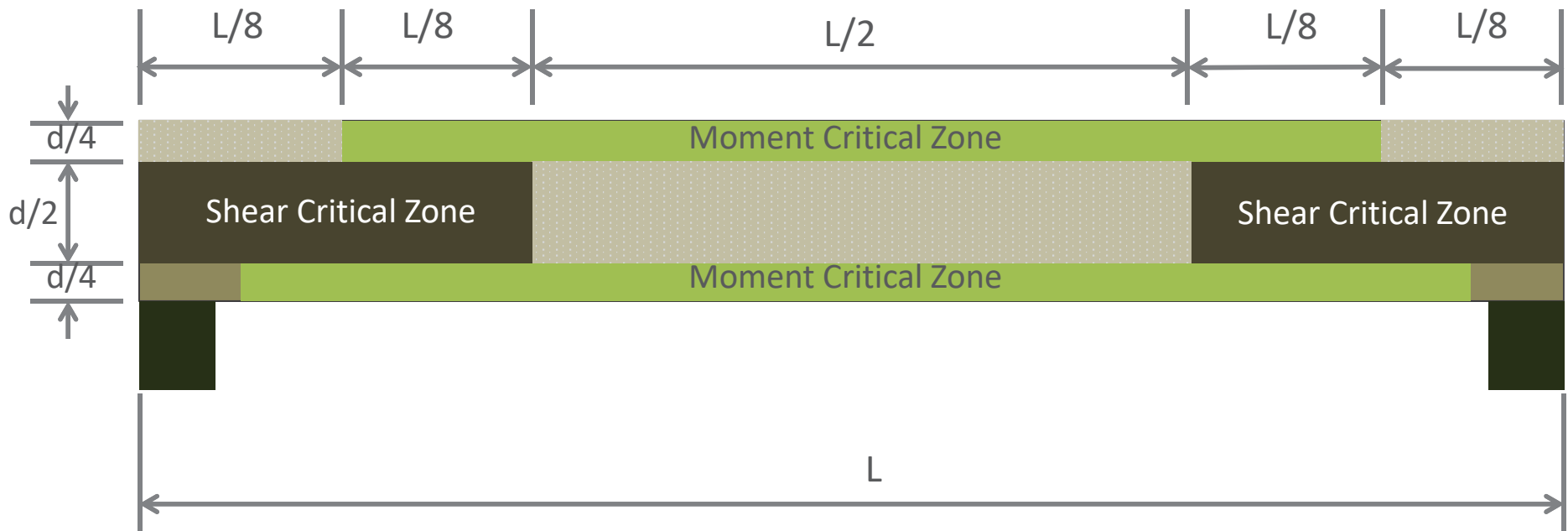


Spanning Members: Localized Stress

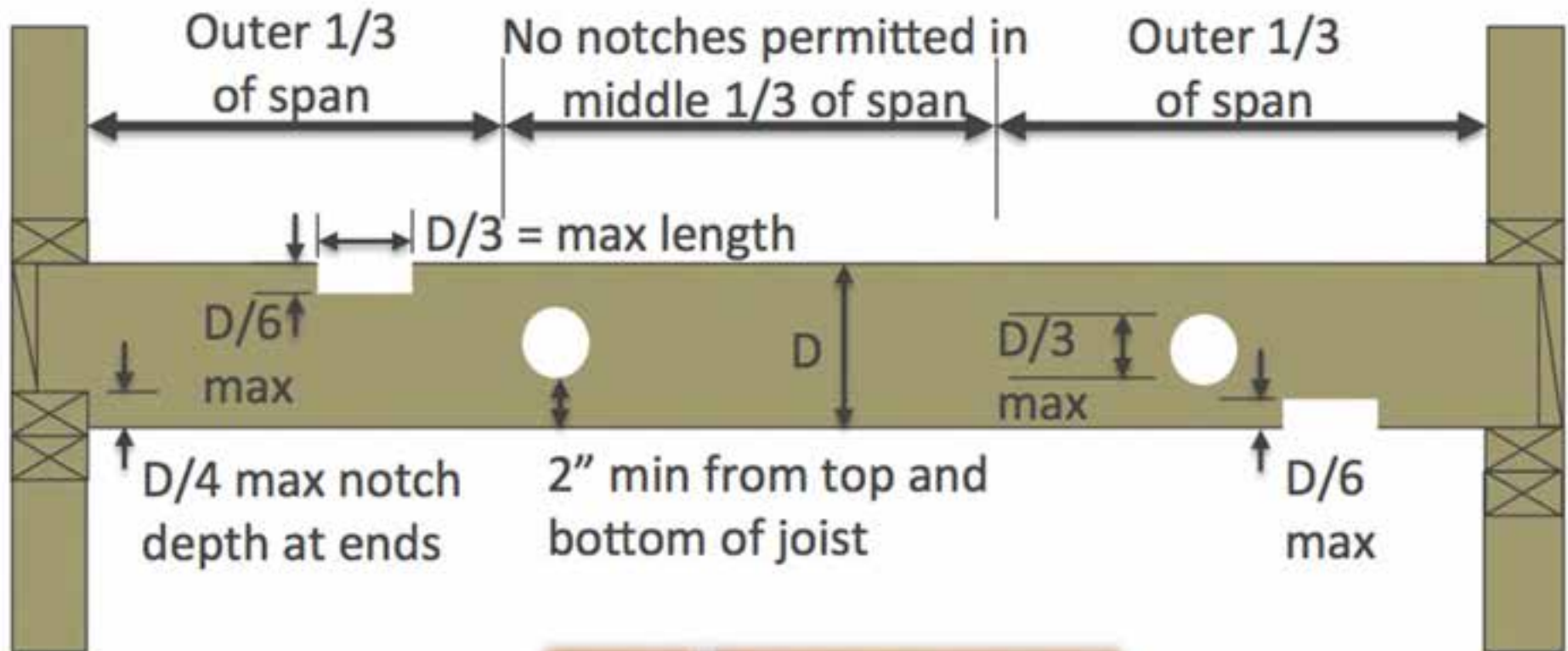
Moment critical zones: locations of maximum localized bending stresses which should not be affected with holes and notches

Shear: locations of maximum localized shear stresses which should not be affected with holes and notches

 Generally acceptable locations of holes/notches



Allowable Floor Holes & Notches – IBC 2308



Per IBC 2012 2308.8.2
or IBC 2015 2308.4.2.4

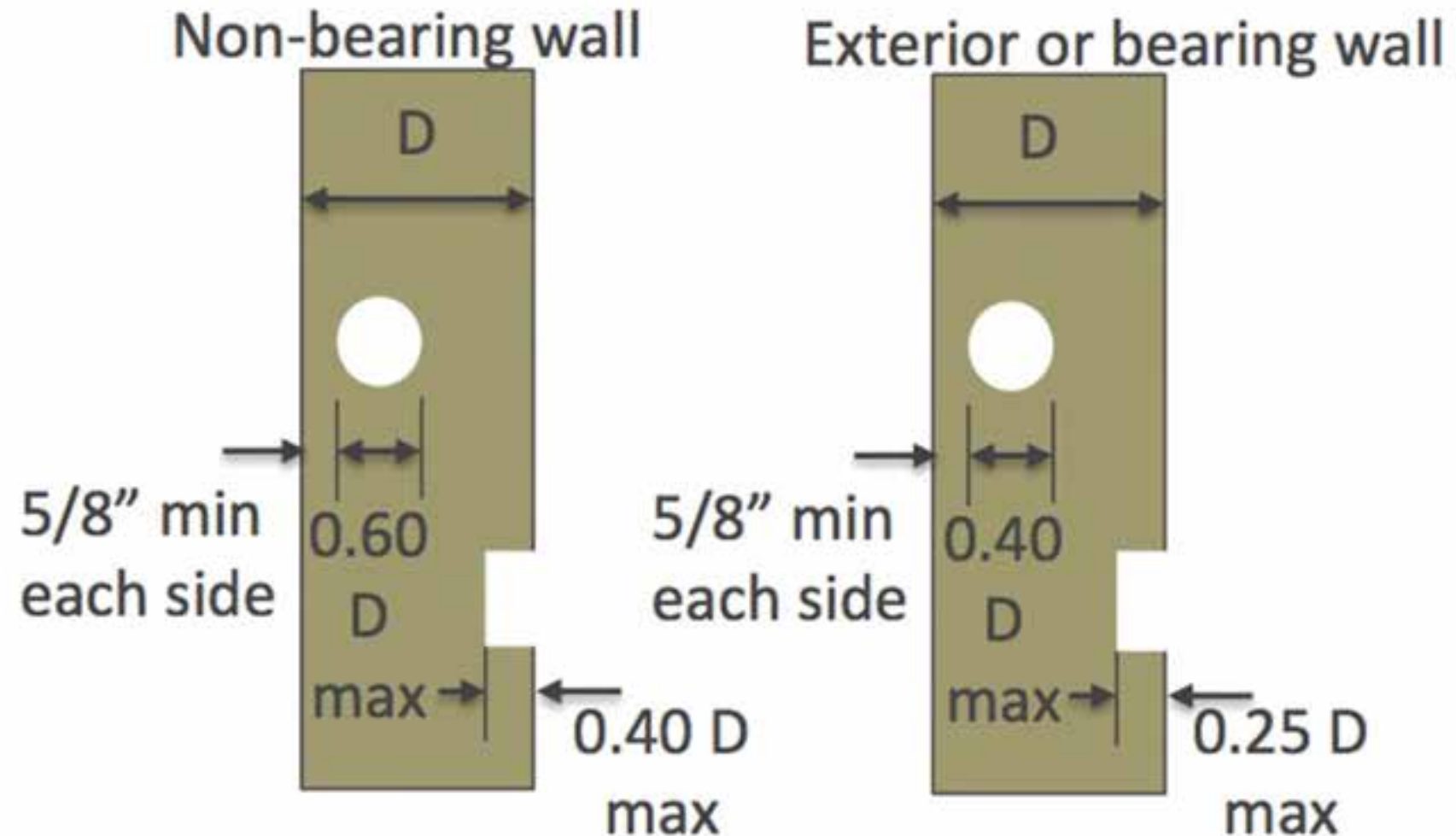


Allowable Floor Holes & Notches – IBC 2308

Allowable Notches & Holes in Solid Sawn Floor

Joist Size	Max Hole	Max Notch Depth	Max End Notch
2x6	1-3/4"	7/8"	1-3/8"
2x8	2-3/8"	1-1/8"	1-7/8"
2x10	3"	1-1/2"	2-1/4"
2x12	3-3/4"	1-7/8"	2-7/8'

Allowable Wall Holes & Notches – IBC 2308



Per IBC 2012 2308.9.10 & 2308.9.11

Per IBC 2015 2308.5.9 & 2308.5.10

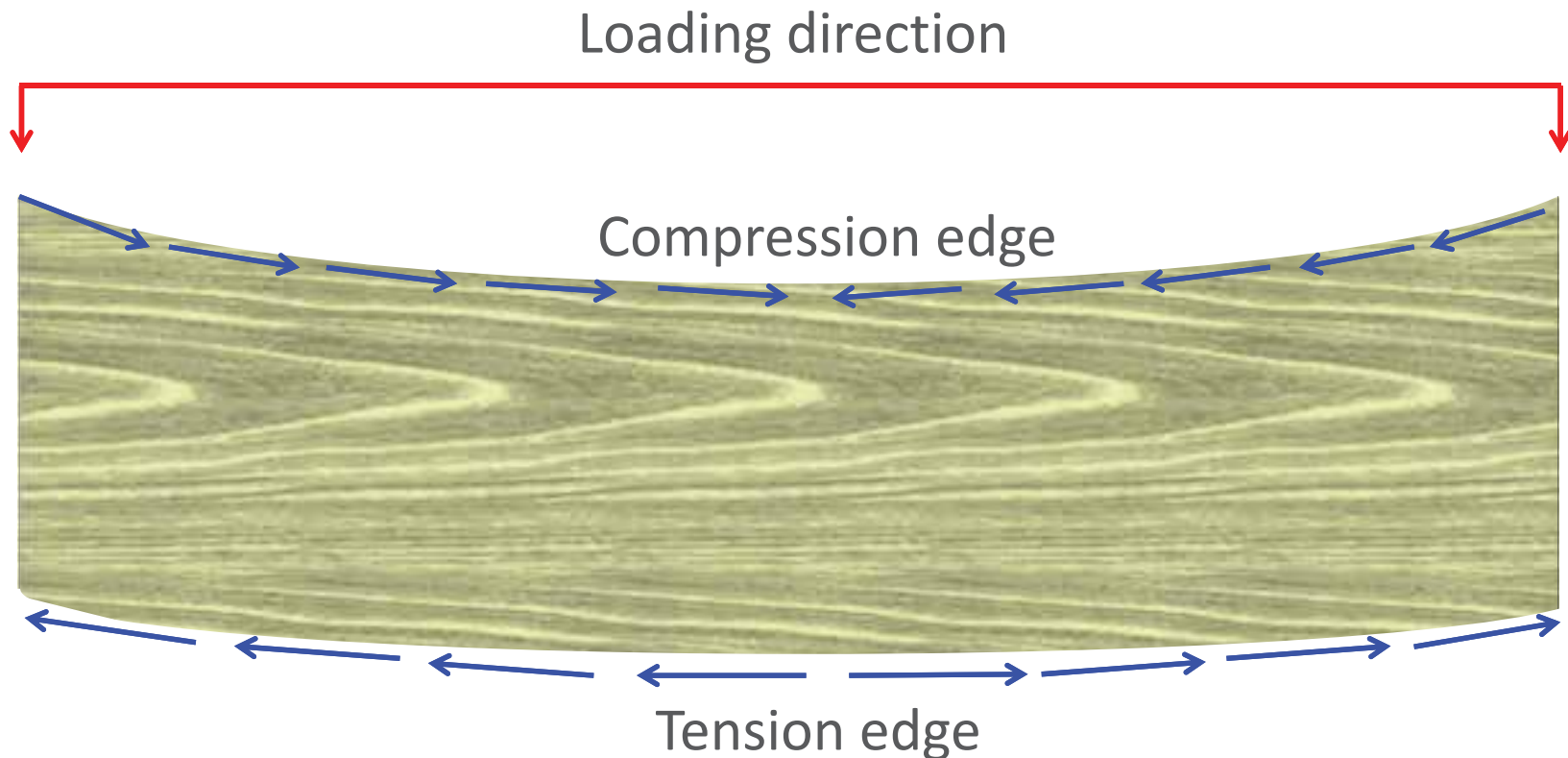
Allowable Wall Holes & Notches – IBC 2308

Allowable Notches & Holes in Solid Sawn IBC 2012 2308.9.10 & 2308.9.11 / IBC 2015 2308

Stud Size	Non-Bearing Wall		Exterior & Bearing Walls	
	Max Hole	Max Notch	Max Hole	Max Notch
2x4	2"	1-3/8"	1-3/8"	7/8"
2x6	3-1/4"	2-1/8"	2-1/8"	1-3/8"
2x8	4-1/4"	2-7/8"	2-7/8"	1-7/8"

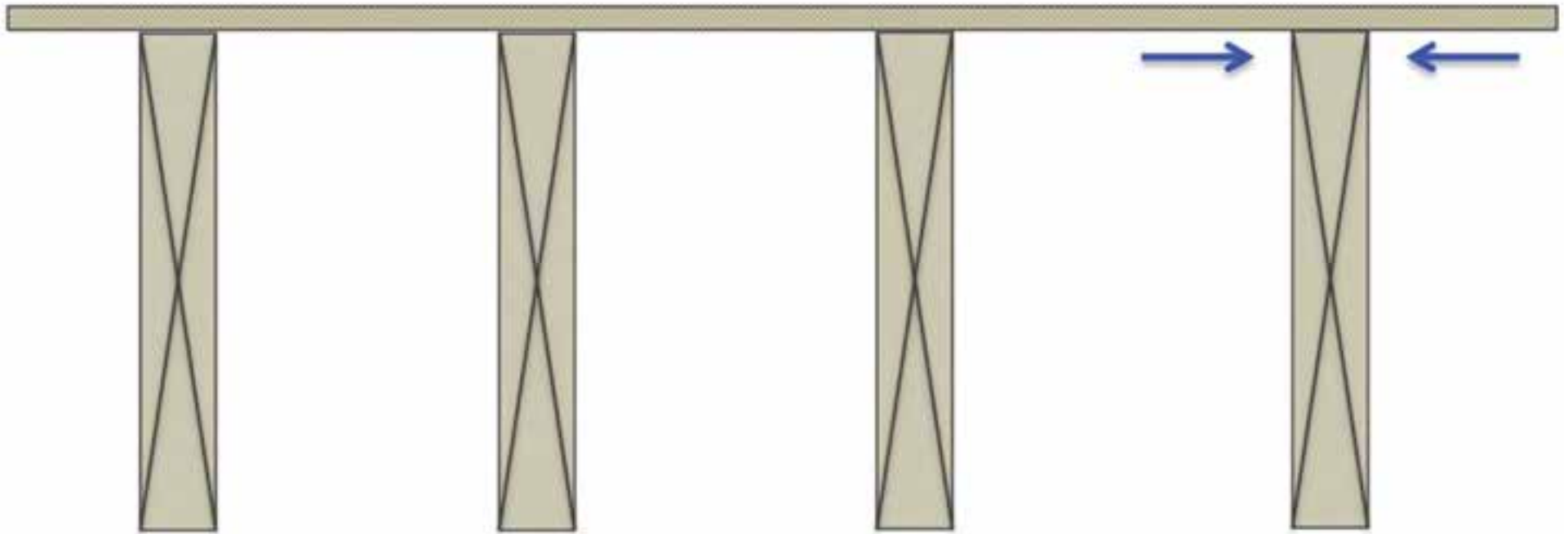
Floor Framing: Compression Edge Bracing

- Bending causes compression in one edge of member
- Compression can cause buckling; compression edge bracing resists buckling



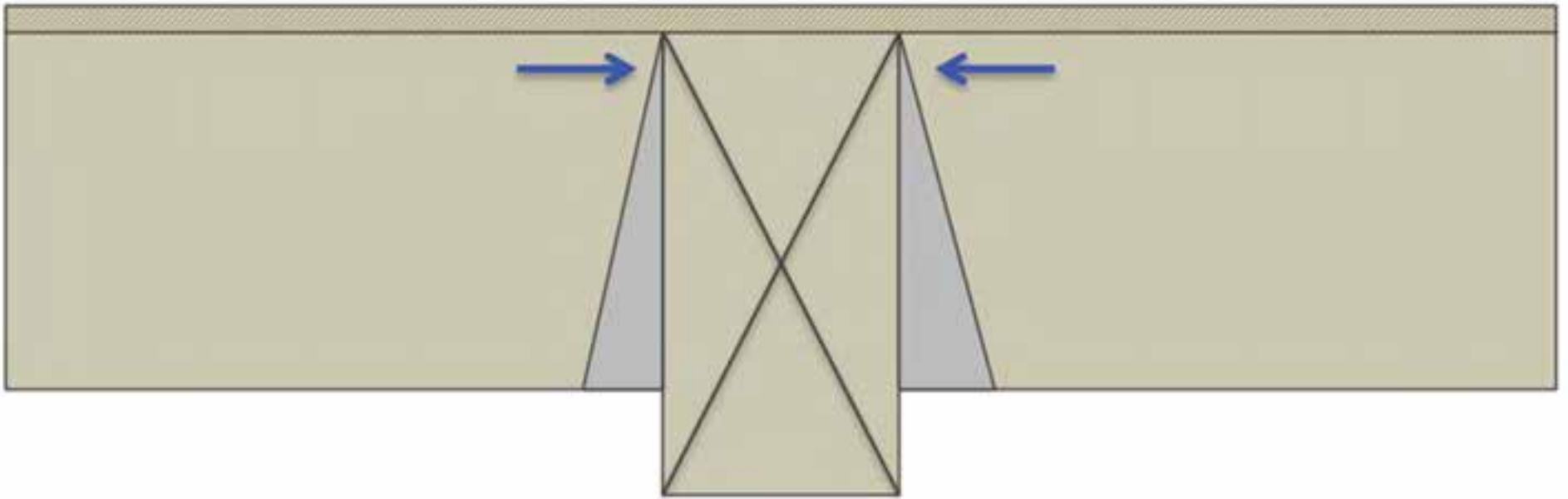
Floor Framing: Compression Edge Bracing

- Floor sheathing braces compression flange of floor joists
- Specify sheathing to framing fasteners sized to resist compression force



Floor Framing: Compression Edge Bracing

- Floor joists brace compression flange of floor beam
- Floor joists installed tight to beam resist beam compression force



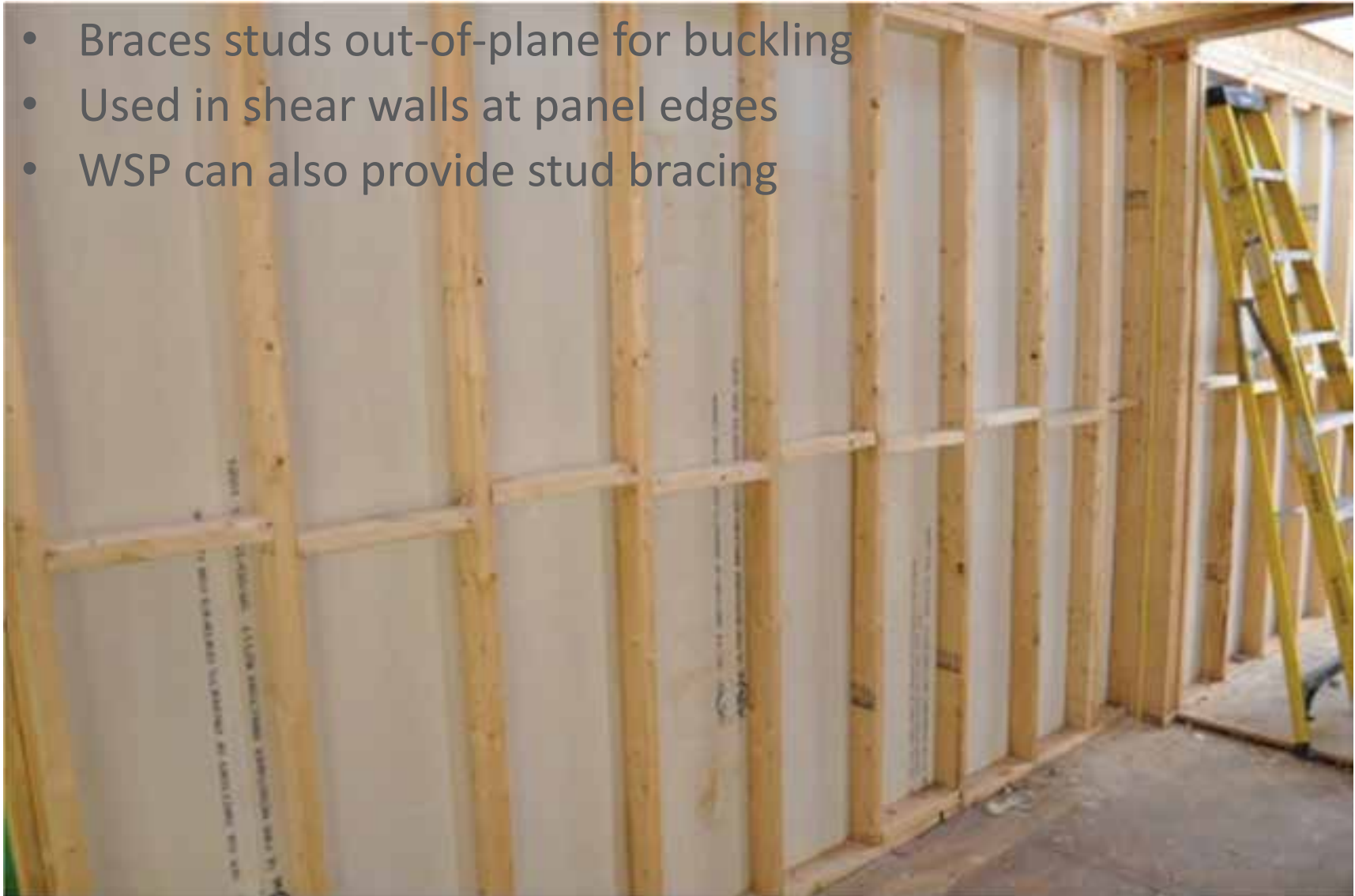
Floor Framing: Blocking & Bracing

- Stiffens floor & prevents joist rotation and out-of-plane buckling
- Required at joist ends and at each support per IBC 2012 2308.8.2 / IBC 2015 2308.4.2.3
- Best practice is to install at 8'-10' o.c. and at concentrated loads
- Solid blocking, wood cross bracing, light gauge steel cross bracing



Wall Framing: Blocking & Bracing

- Braces studs out-of-plane for buckling
- Used in shear walls at panel edges
- WSP can also provide stud bracing



Trusses: Blocking & Bracing

- Trusses may require global and local bracing
- Global bracing provides overall roof/floor structure stability (unique to trusses often due to their size)
- Bottom chord bracing, sway bracing



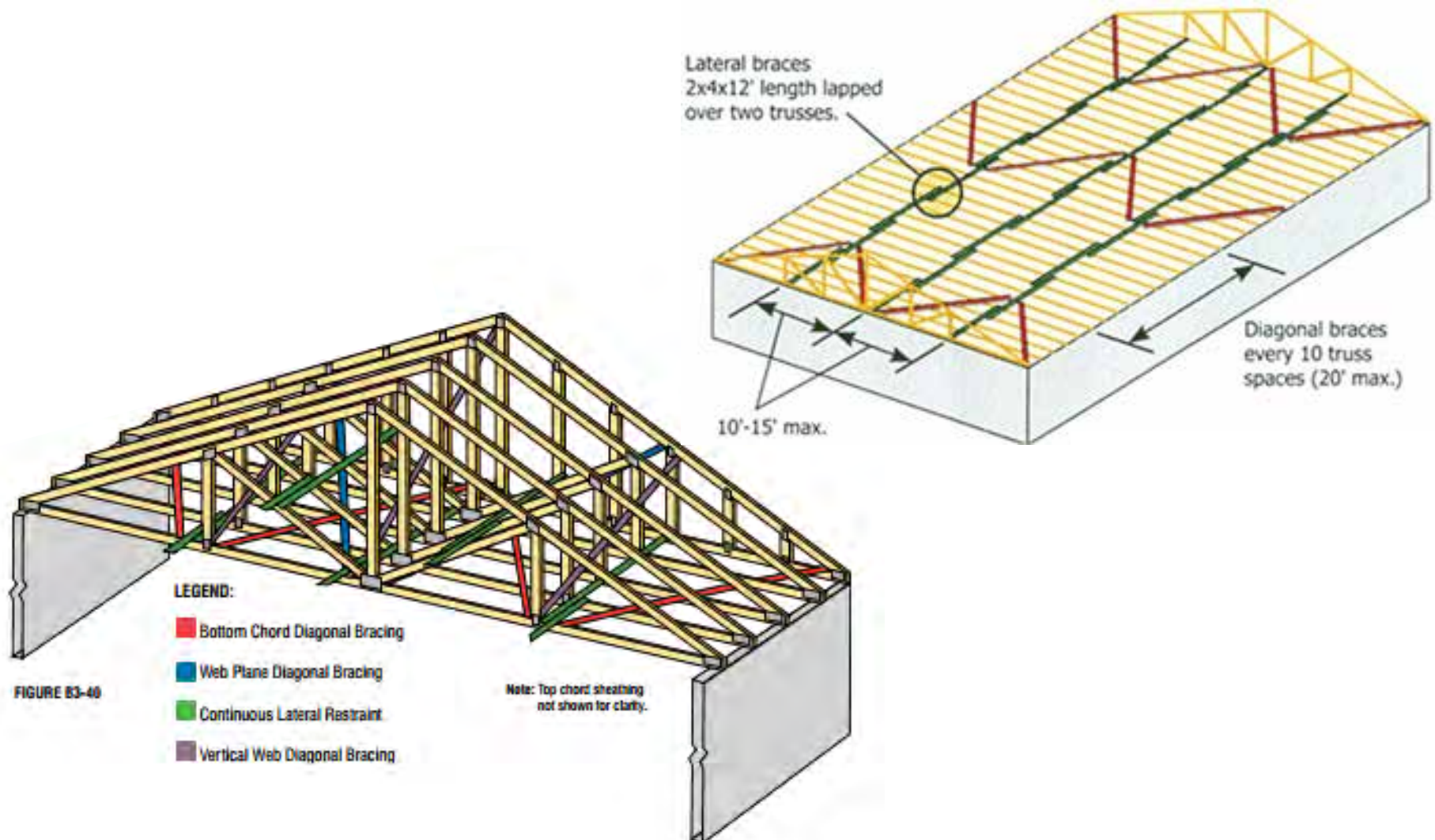
Sway Bracing

Bottom
Chord
Bracing



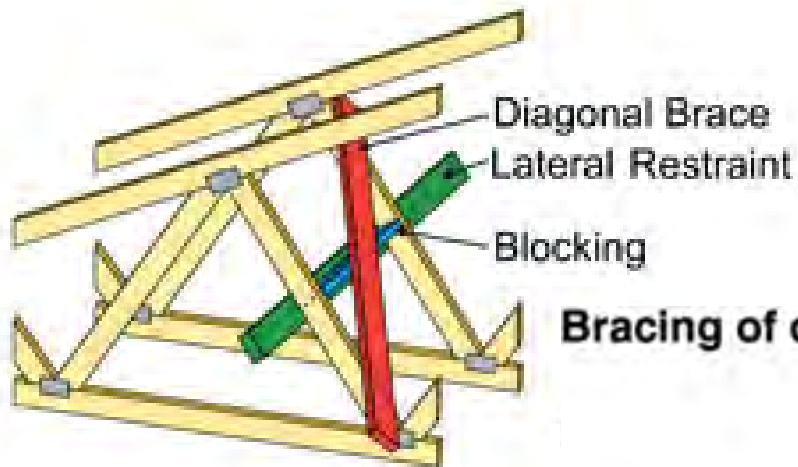
Trusses: Blocking & Bracing

- Local bracing addresses webs in compression

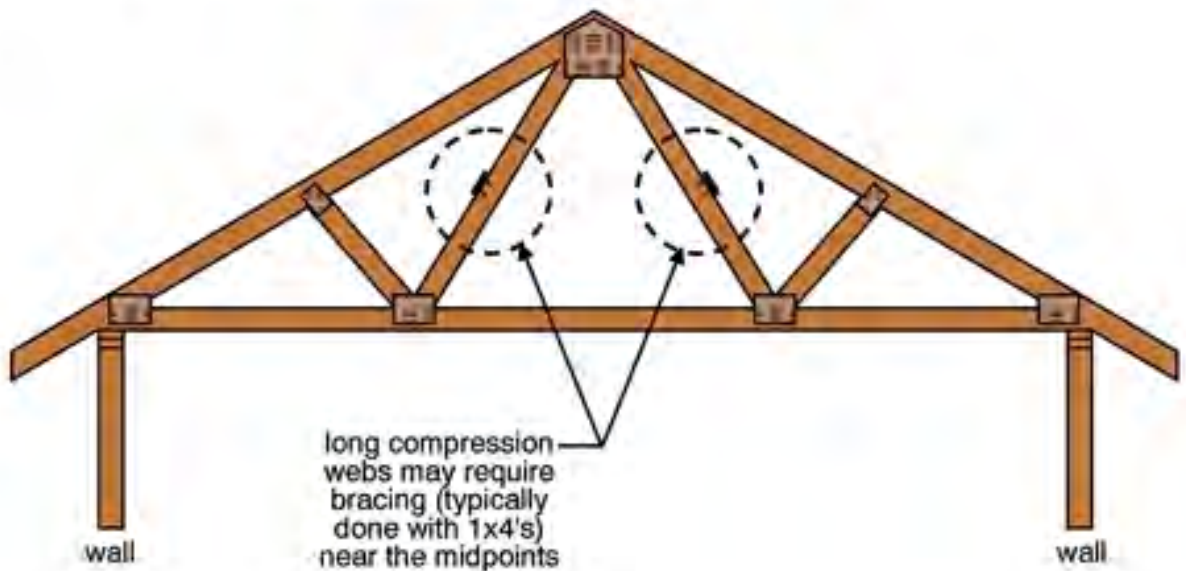


Trusses: Blocking & Bracing

- Additional diagonal bracing for web braces may be required

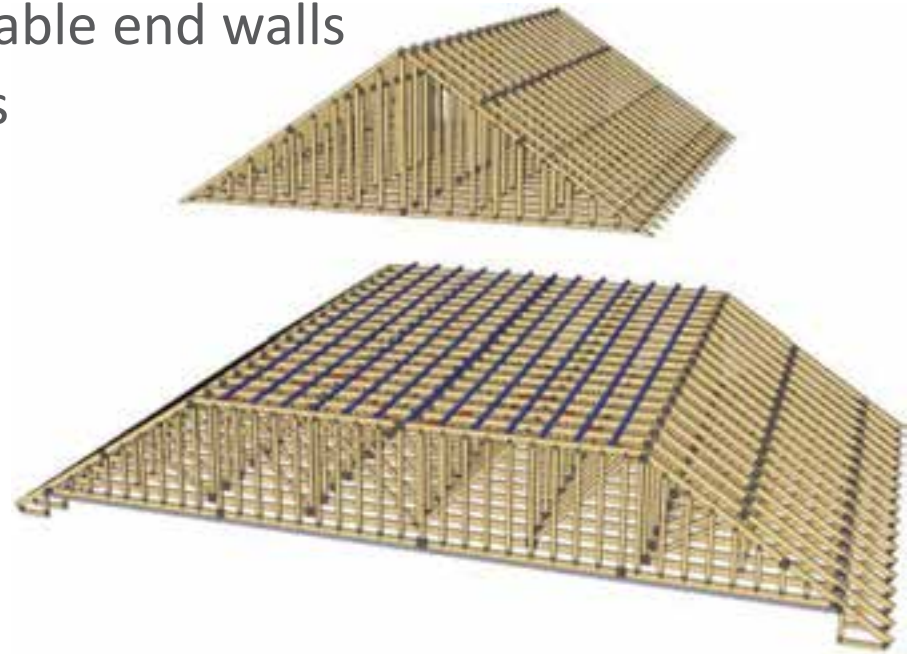


Bracing of compression webs



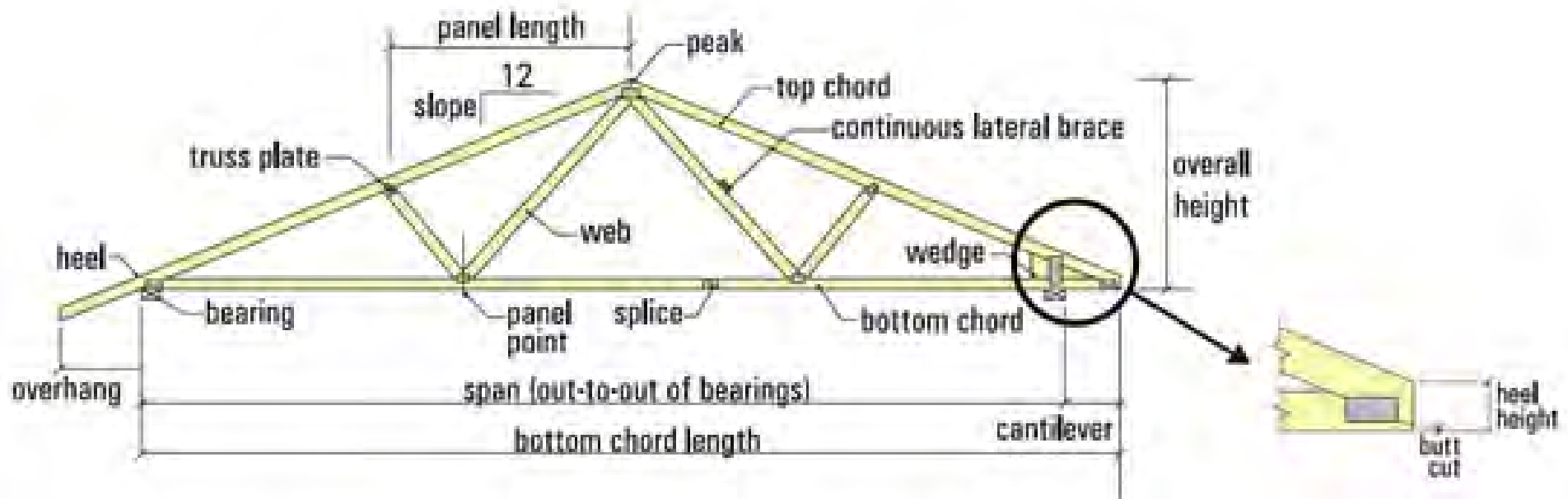
Trusses: Additional Bracing Considerations

- Piggyback trusses: typically required for trusses over 10' – 12' tall (transportation limitation)
- Gable end diagonal bracing at gable end walls
- Parallel chord truss strong backs
- Temporary construction bracing



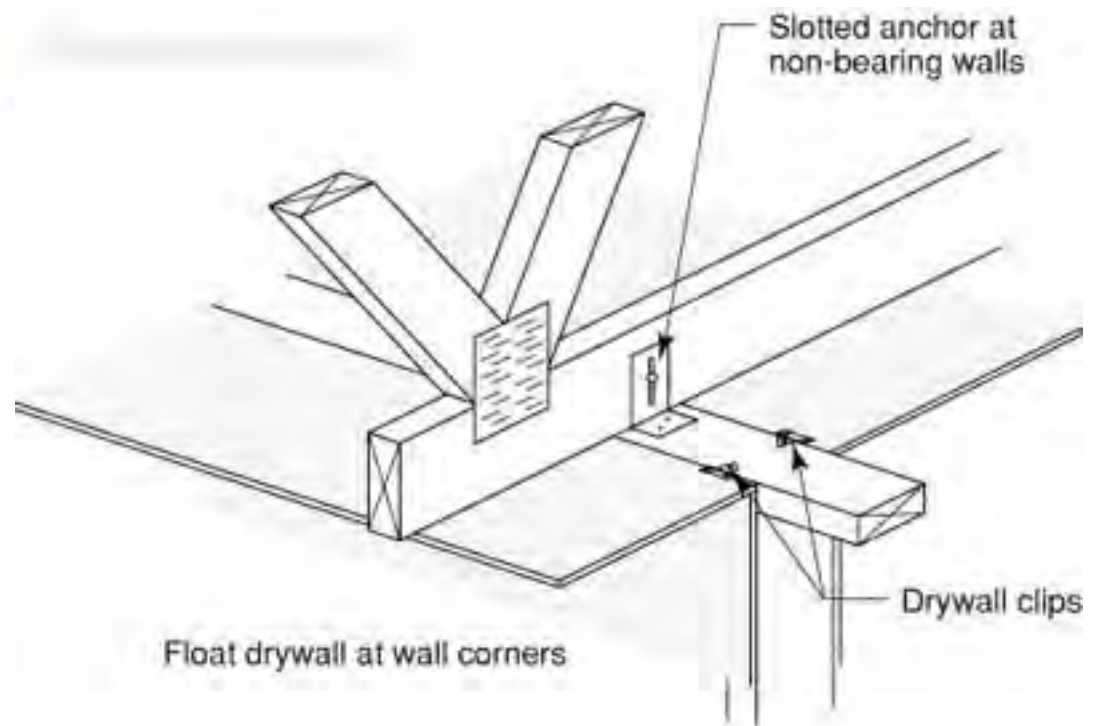
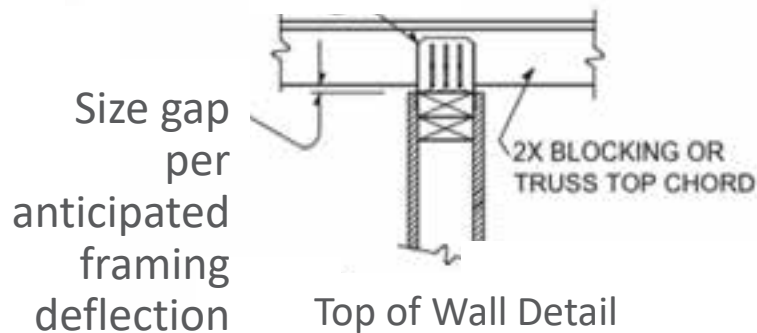
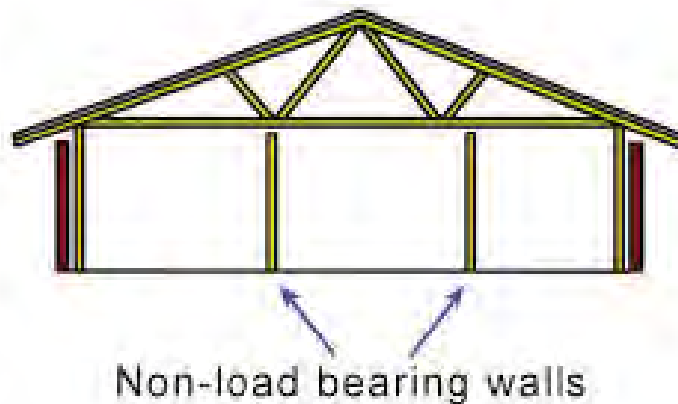
Trusses: Diagrams, Components

- Drawings should indicate truss dimensions (span, overhang, height), spacing, slope, bearing conditions, required web layout, loads, codes
- Three main components: chords, webs, & plates
- Applied components: sheathing, purlins, bracing, uplift connectors



Non-Load Bearing Partitions

- Important to verify that “idealized” conditions are achieved
- Partitions remain non-load bearing
- Provide slip connections; limit finish damage, undesired structural load redistribution

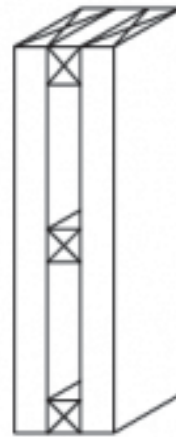


Built-Up Members

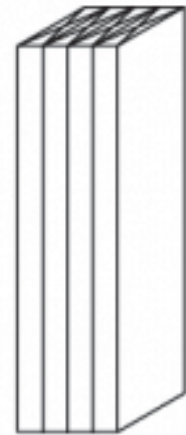
- Direction of load (perp. or parallel to multi-ply interfaces) determines fastener design
- Bending axis parallel to interface axes requires shear flow analysis
- Many ways to optimize multi-ply members: built-up, t-members, I-shape



SINGLE MEMBER
COLUMN



SPACED COLUMN



BUILT-UP COLUMN
(WITH CONTINUOUS MEMBERS)

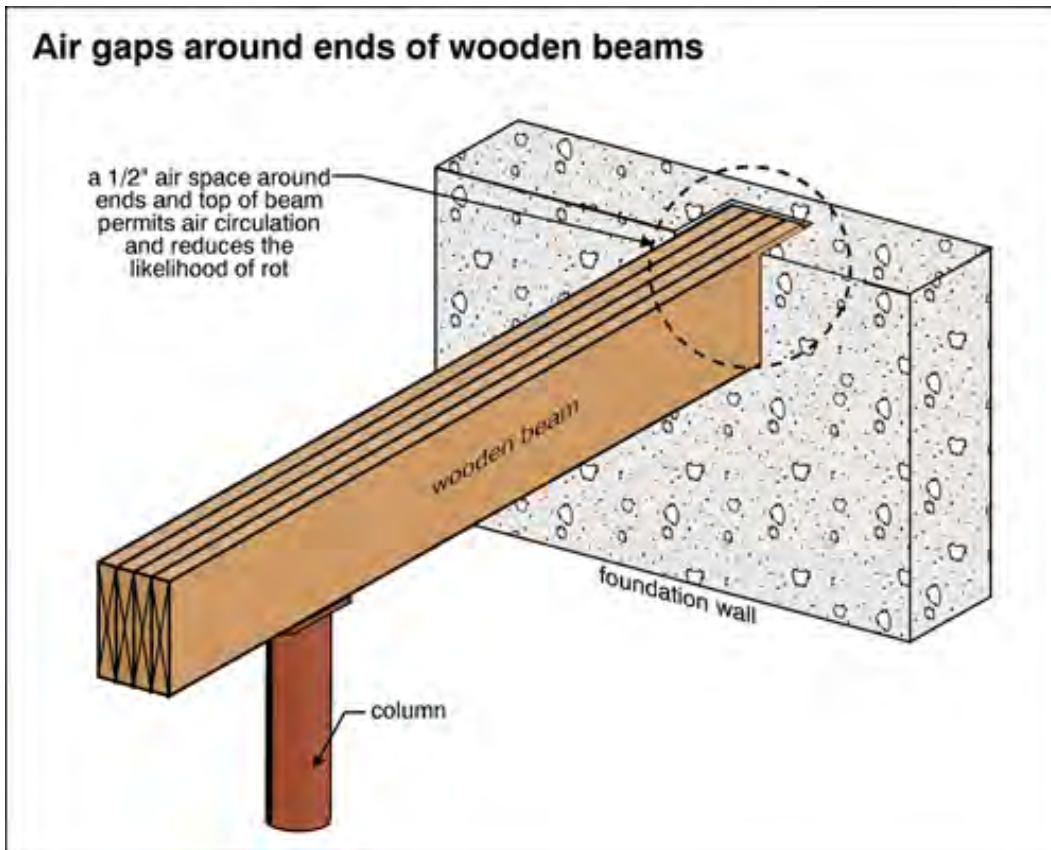
Wood in Contact with Other Materials

- IBC 2012 2304.11 / IBC 2015 2304.12 requires preservative treated or naturally durable wood when in contact with foundation walls



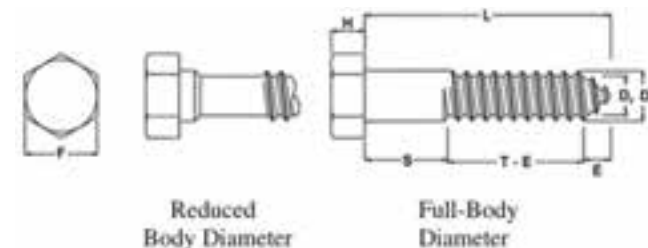
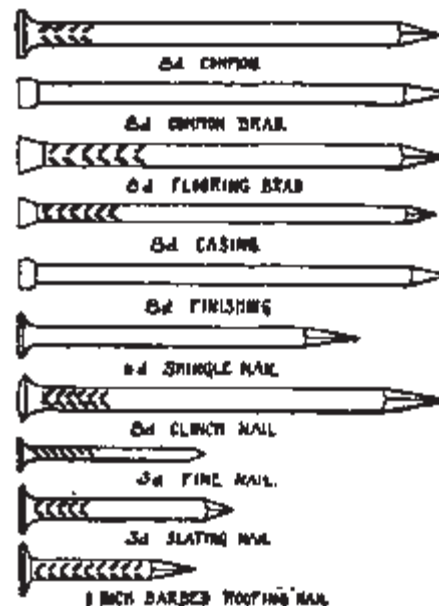
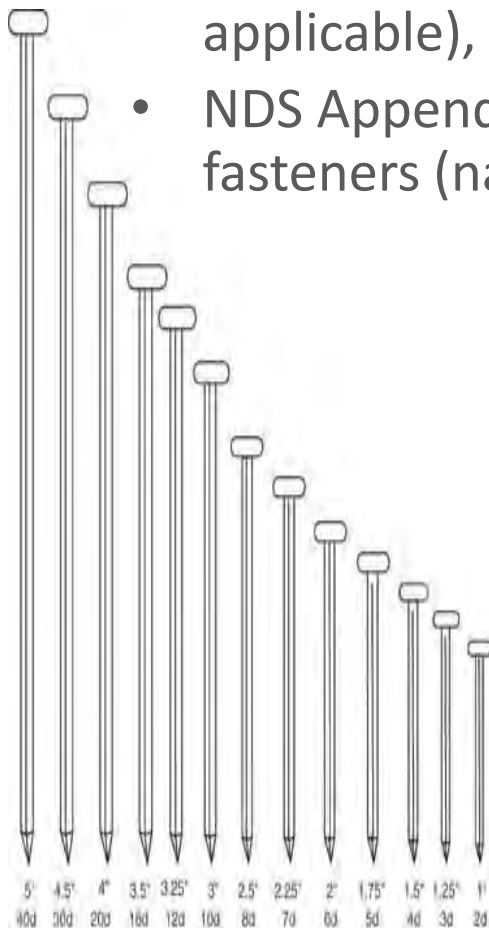
Wood in Contact with Other Materials

- Detail to allow drying, venting, differential movement
- Avoid moisture traps



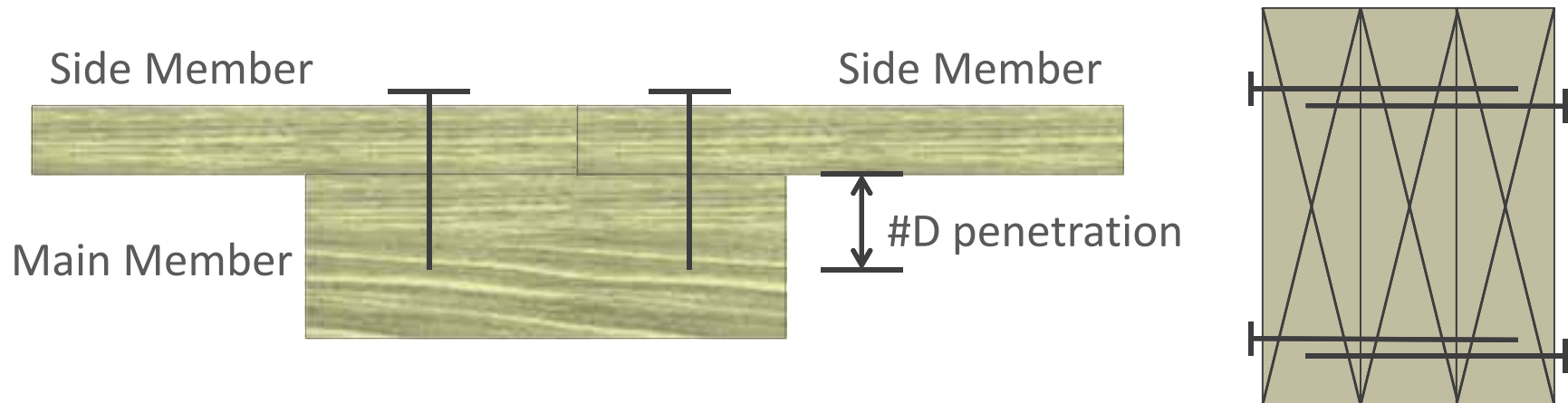
Connections: Specification

- Fastener specification should include all variables, not just a size (10d) as there can be many different options in a size group
- Fastener length, diameter, finish, material, thread length (if applicable), and thread pitch (if applicable) are all variables
- NDS Appendix L contains dimensions for typical dowel type fasteners (nails, wood screws, lag screws, bolts)



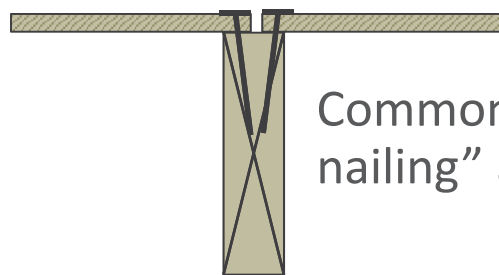
Connections: Length Specification

- Penetration measured as distance fastener extends in *Main Member*
- Minimum penetration generally 4D (lag screws) to 6D (nails, wood screws)
- Minimum penetration for no capacity reduction generally 8D (lag screws) to 10D (nails, wood screws)
- Consider effects of fasteners on both faces of multi-ply members – stagger (vertically, horizontally, or both)

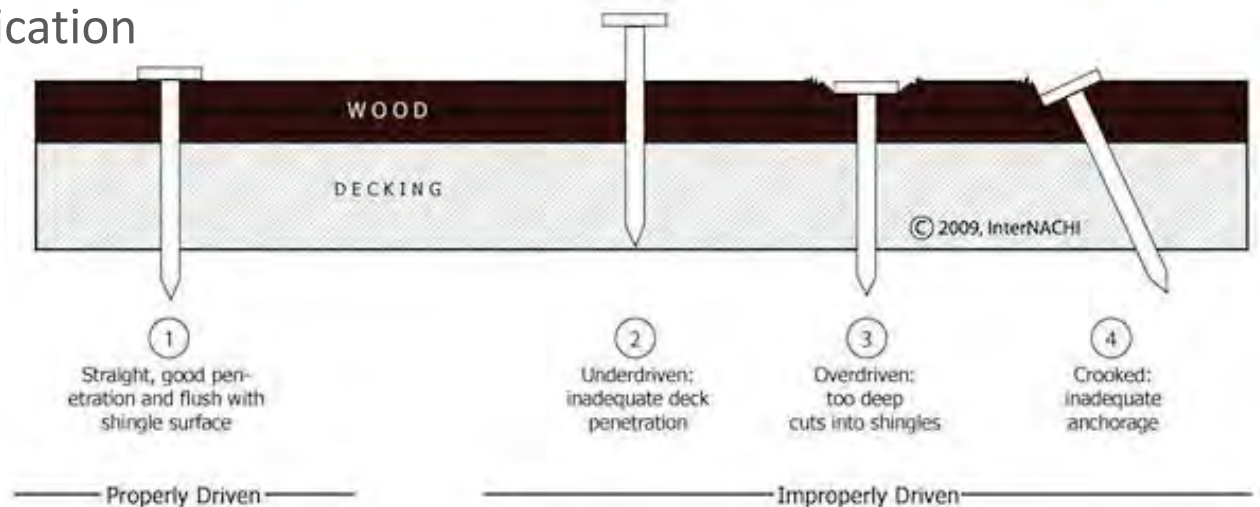


Connections: Proper Installation

- Fastener alignment and head seating are key to achieving full fastener capacity
- Detail connections to allow adequate edge, end distances, avoid wood splitting
- Per Teco, if 50% of fasteners are overdriven to 1/8", over 8% reduction in shear capacity results

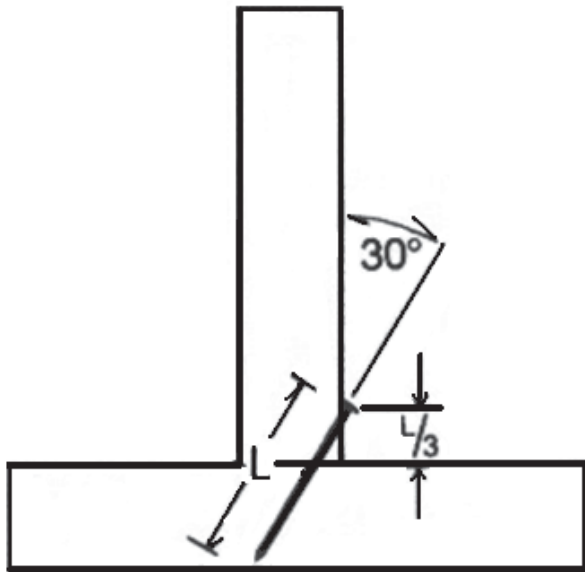


Common “slant nailing” application



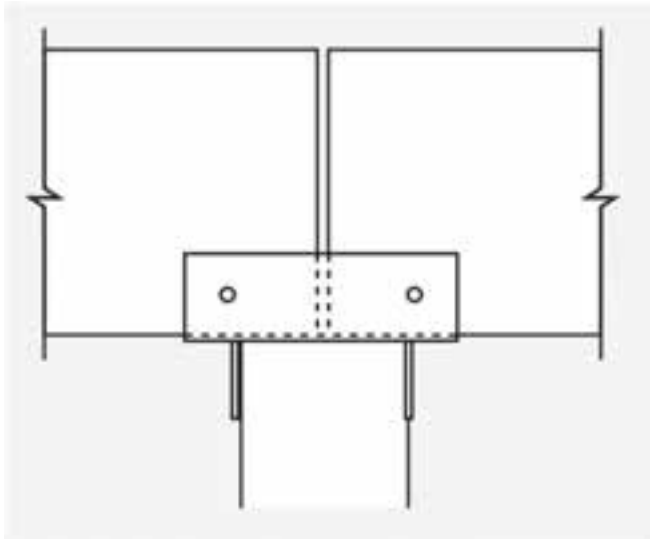
Connections: Toe Nails

- Recommended install 30° from side member longitudinal direction
- Nail penetrates side member a distance $1/3$ its length from the joint
- Toe nail reduction factor, $C_{tn} = 0.67$ for withdrawal, 0.83 for shear (NDS 2012 11.5.4 / NDS 2015 12.5.4)



Connections: Shrinkage

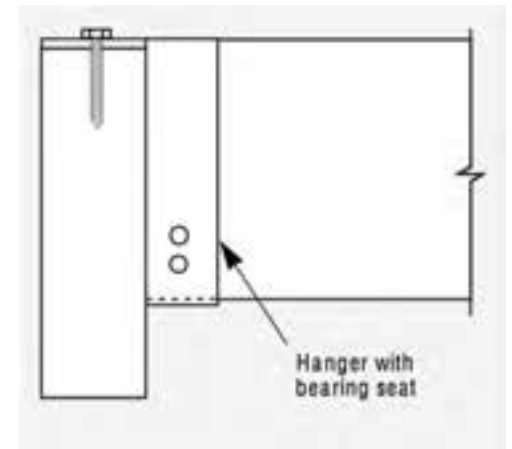
- Consider shrinkage in connected members
- Avoid restraining shrinkage in connection – place connections on one edge of member rather than on both edges
- Best practice is to connect to lower half of beams, use of slotted holes aids in avoiding shrinkage cracks
- Single row of fasteners (vs. multiple rows) in non-dried lumber recommended



Source: APA



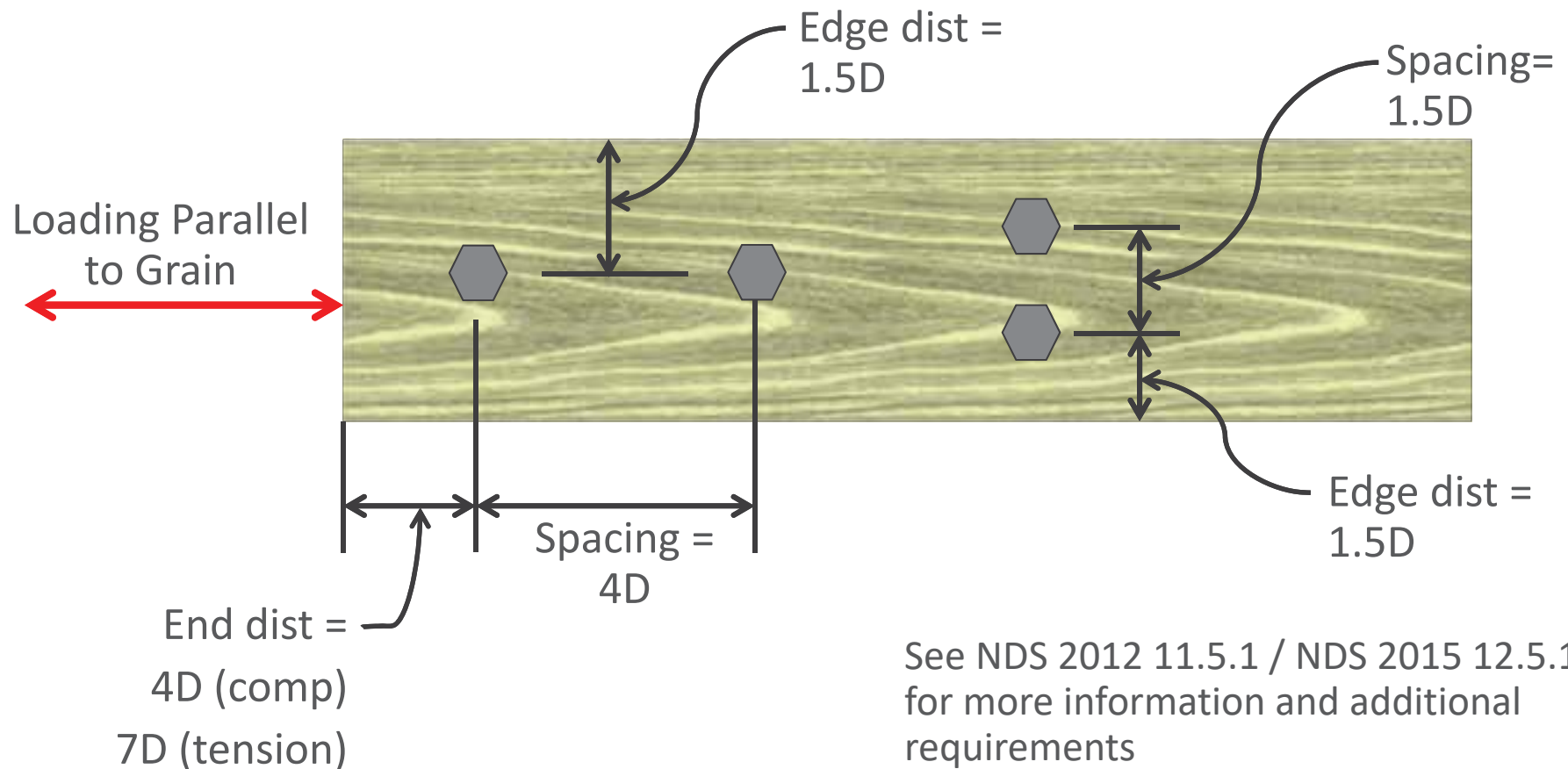
Source: Strongtie.com



Source: APA

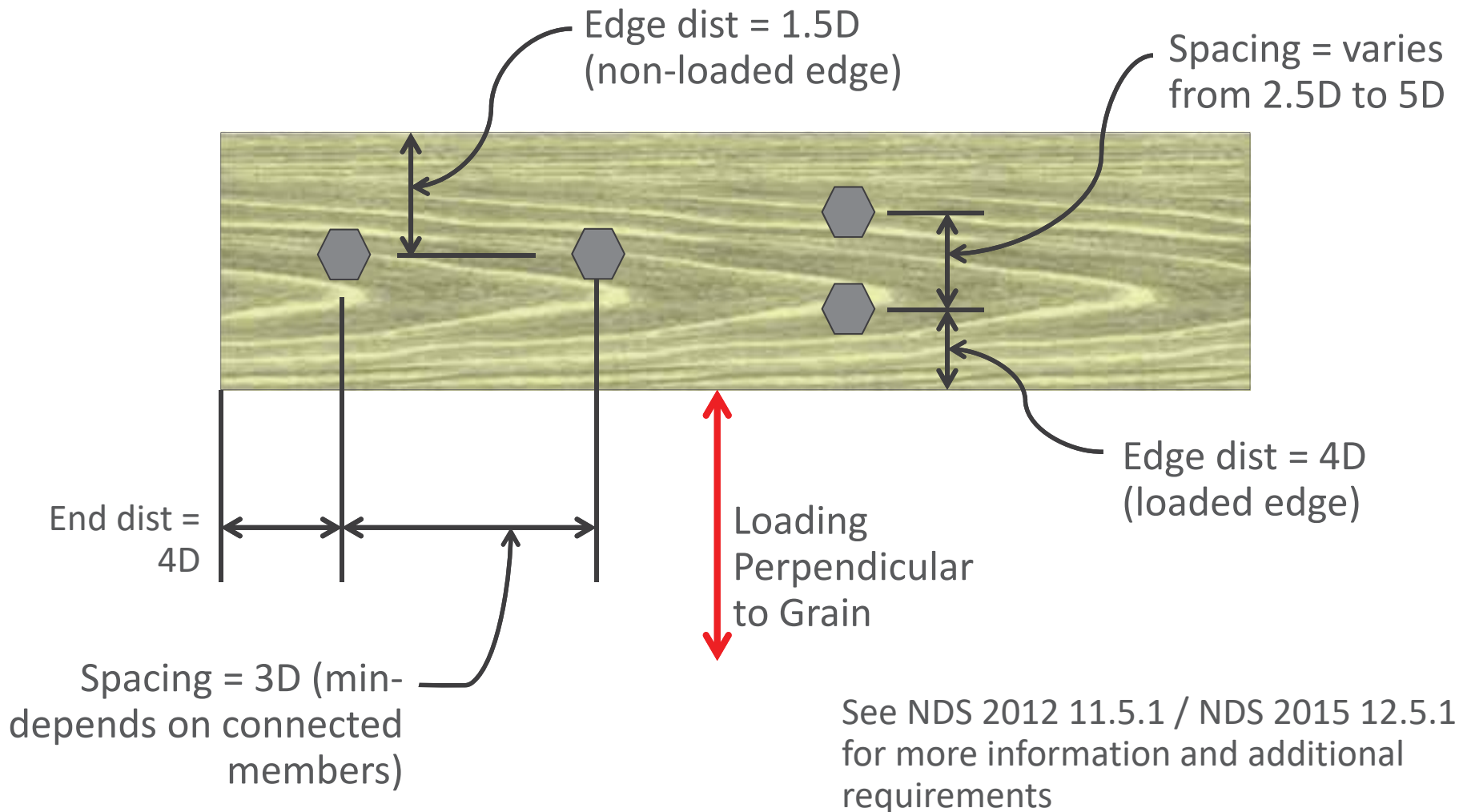
Dowel Type Connections: Location

- Three main location considerations: spacing, edge distance, end distance
- Spacing considerations between fasteners in a row and between rows of fasteners



Dowel Type Connections: Location

- Loading perpendicular to grain fastener location considerations



Moisture Control Considerations

- Detail connections to avoid moisture traps
- Avoid end grain exposure to moisture by use of hold off past bases, beam end caps



Moisture Control Considerations

- Minimize storage of material on site where rain and standing water can increase moisture content
- Keep unused framing material covered
- Immediately remove any standing water from floor framing after rain showers



> Questions?

This concludes The
American Institute of
Architects Continuing
Education Systems
Course

Anthony Harvey, PE
Regional Director
anthony.harvey@woodworks.org

Visit www.woodworks.org for more educational materials,
case studies, design examples, a project gallery, and more



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Introducing Cross Laminated Timber

New Opportunities for
Timber Construction

Anthony Harvey, PE





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Questions related to specific materials, methods, and services will be addressed at the conclusion of this presentation.



Course Description

Cross laminated timber (CLT) is an engineered wood building system designed to complement light- and heavy-timber framing options. Because of its high strength and dimensional stability, it can be used as an alternative to concrete, masonry and steel in many building types. This presentation will introduce CLT with a series of project examples that demonstrate its use and associated benefits in a range of applications. Information on manufacturing, specification and code-related considerations will also be discussed.



Learning Objectives

1. Review completed CLT projects that demonstrate a range of applications and system configurations.
2. Discover how CLT can be used under current and future building codes and standards.
3. Discuss benefits of using CLT in place of concrete and steel, including structural versatility, prefabrication, lighter carbon footprint and reduced labor costs.
4. Discuss the fire characteristics of CLT, including the benefits of charring, current seismic approaches that can be used for CLT buildings, and how the acoustic and moisture performance of CLT assemblies can inform the design of a project.

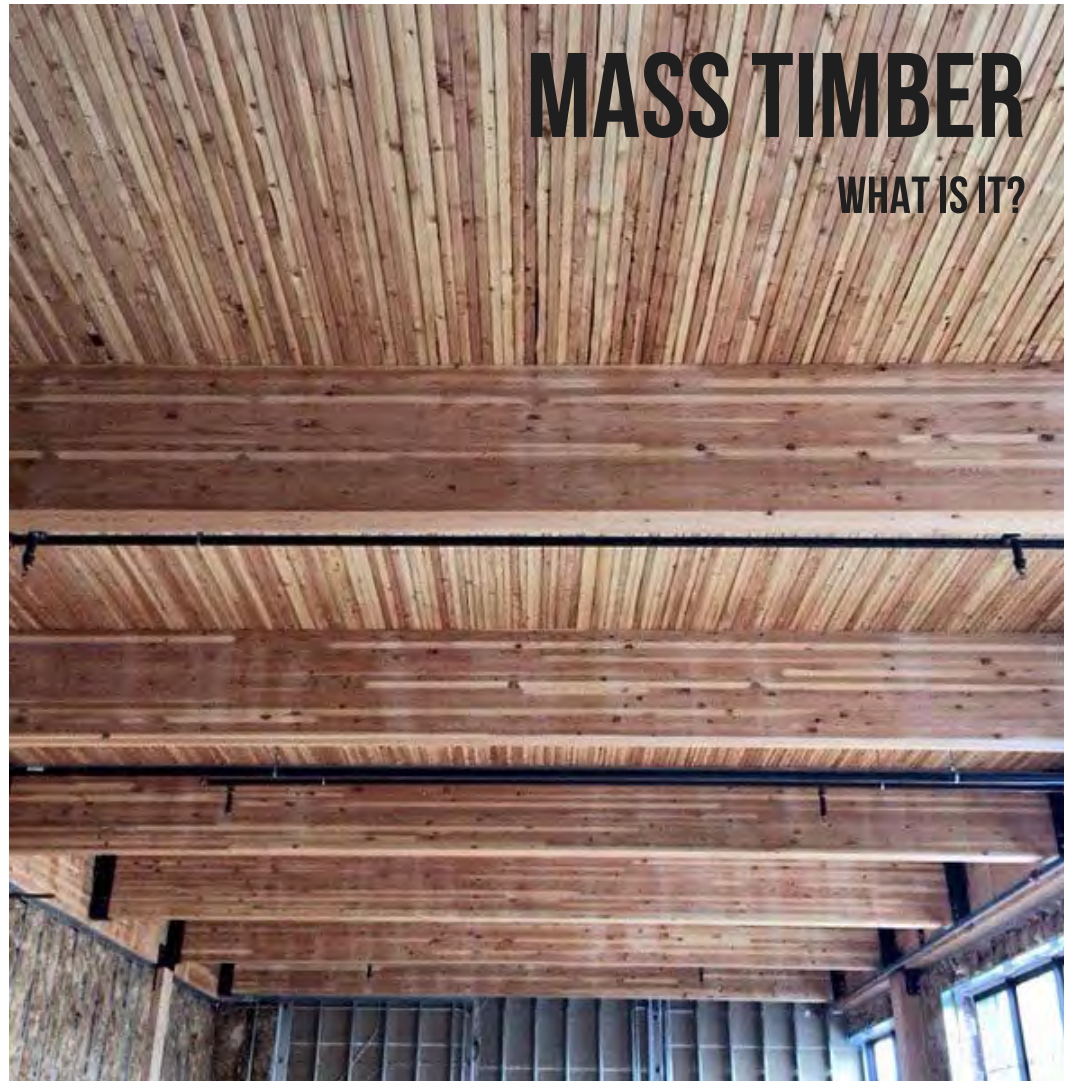
Outline

- What is CLT?
 - Mass Timber
 - The Appeal
 - History
 - Availability
- Using CLT
 - Project Examples
 - Best applications
 - Cost effective design
 - Building Codes and Standards

Outline

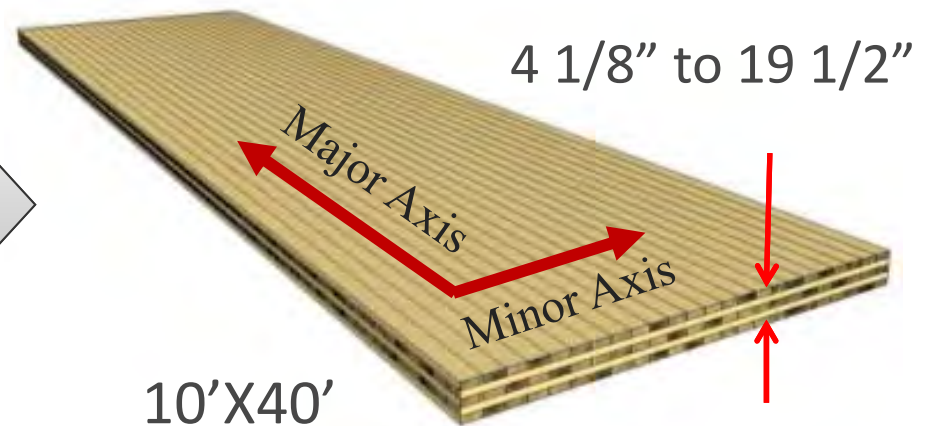
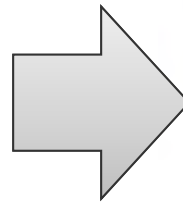
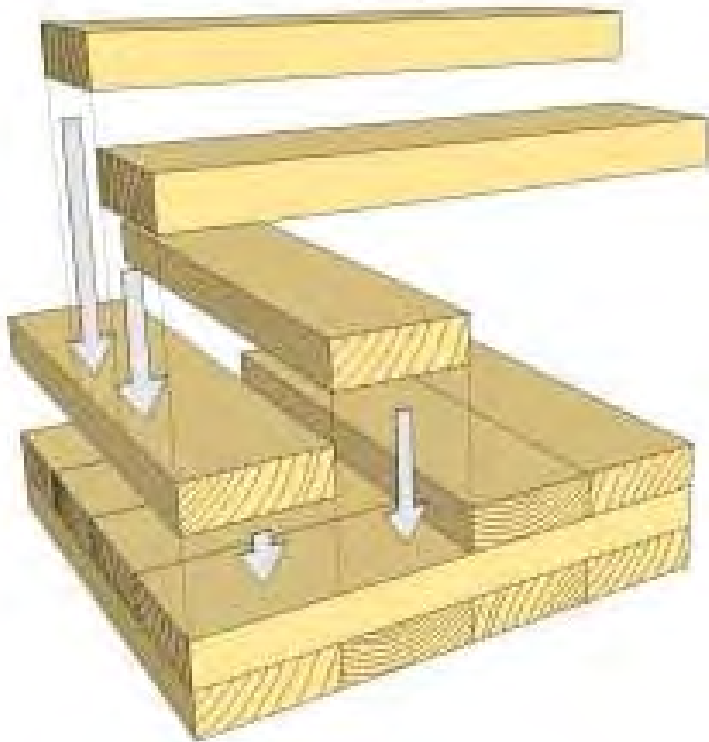
- What is CLT?
 - Mass Timber
 - The Appeal
 - History
 - Availability
- Using CLT
 - Project Examples
 - Best applications
 - Cost effective design
 - Building Codes and Standards

**MASS TIMBER IS A
CATEGORY OF FRAMING
STYLES OFTEN USING SMALL
WOOD MEMBERS FORMED
INTO LARGE PANELIZED
SOLID WOOD CONSTRUCTION
INCLUDING CLT, NLT OR
GLULAM PANELS FOR FLOOR,
ROOF AND WALL FRAMING**



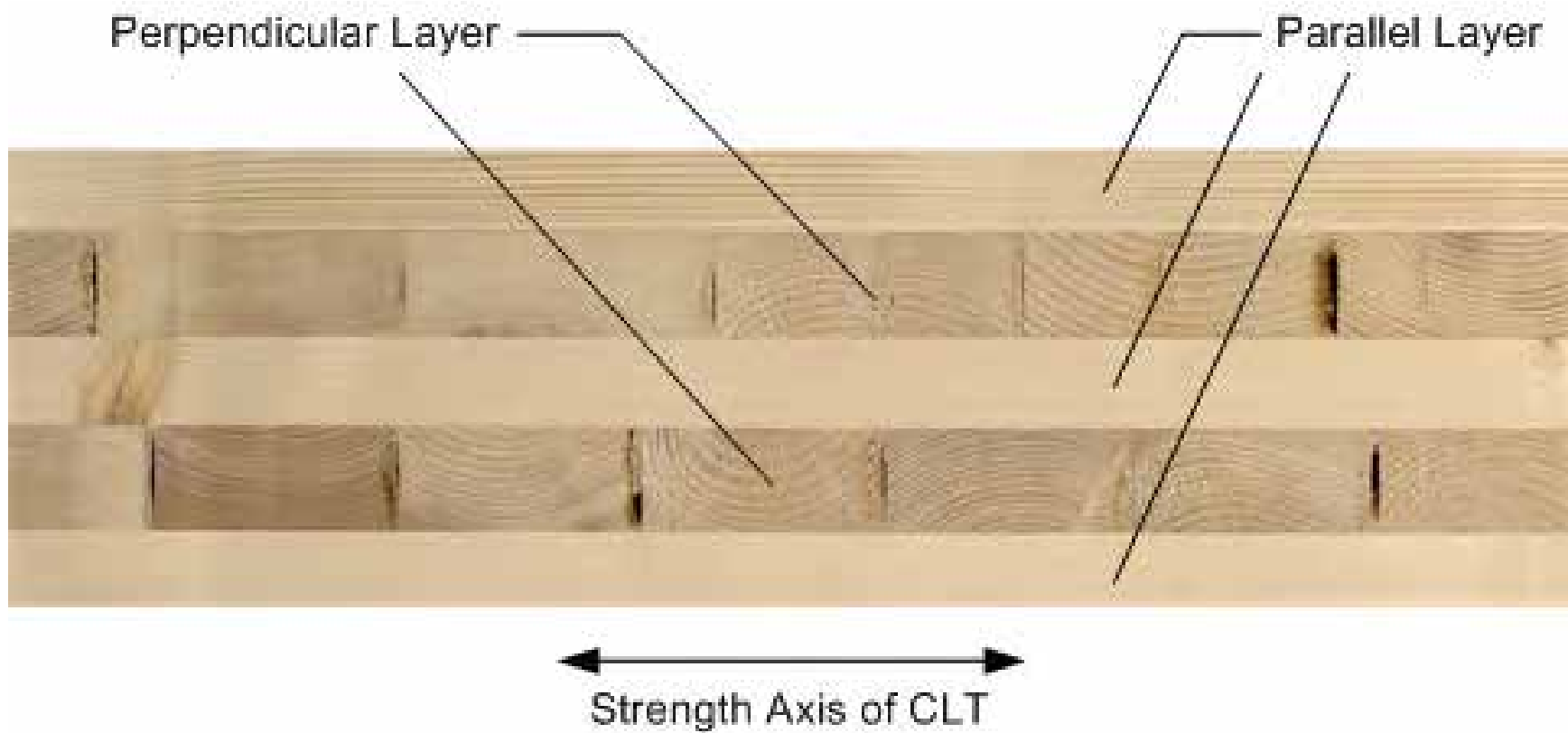
What is Cross Laminated Timber (CLT)?

- Solid wood panel
- 3 layers min. of solid sawn lams
- 90 deg. cross-lams
- Similar to plywood sheathing



10'X40'
8'X64'

CLT Composition

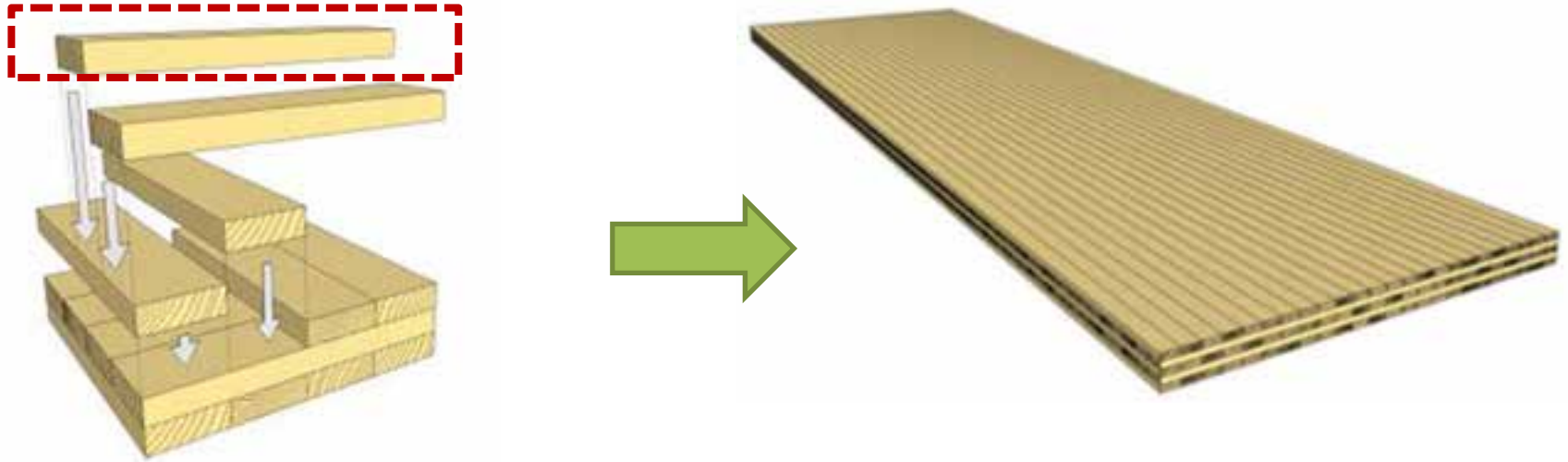


CROSS LAMINATED TIMBER

LUMBER IN CLT IS FINGER JOINTED
TYPICALLY NOT EDGE GLUED



Structural Composition of CLT



Laminations: (Per PRG 320-2012)

5/8" to 2" thick

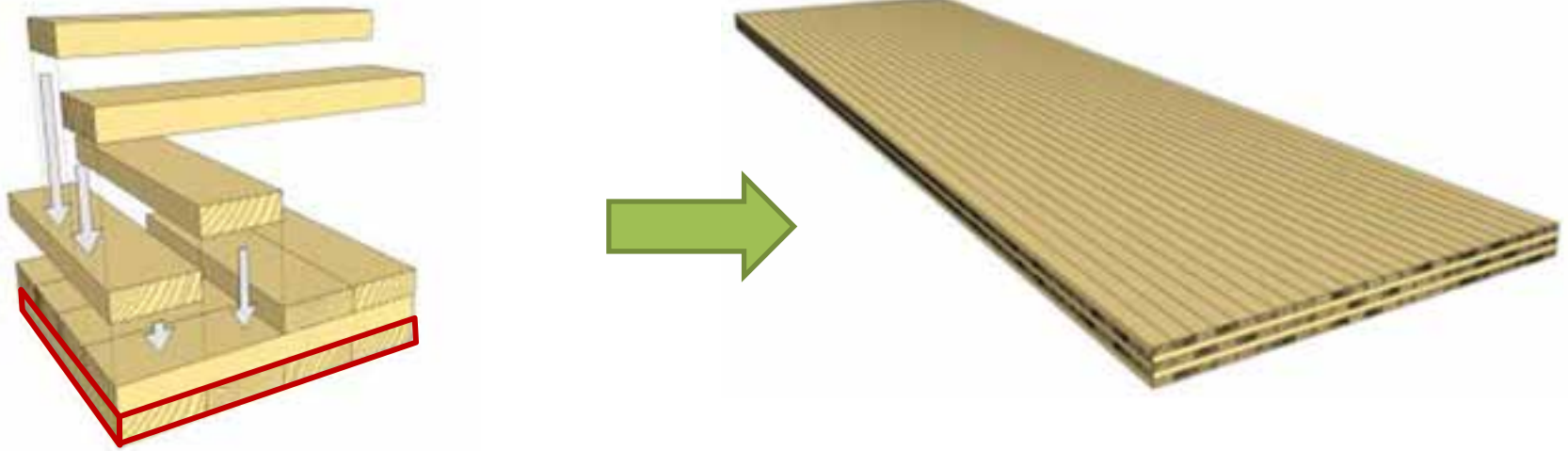
2.4" - 9.5" wide

Machine Stress Rated or Visually Graded Dimensional Lumber or SCL
Dried to 12% Moisture Content before layup.

A common NA thickness is 1 3/8" (planed 2x stock)

PRG 320 provides thickness to width requirements of laminations

Structural Composition of CLT



Layers: (Per PRG 320-2012)

Oriented in orthogonal arrangement

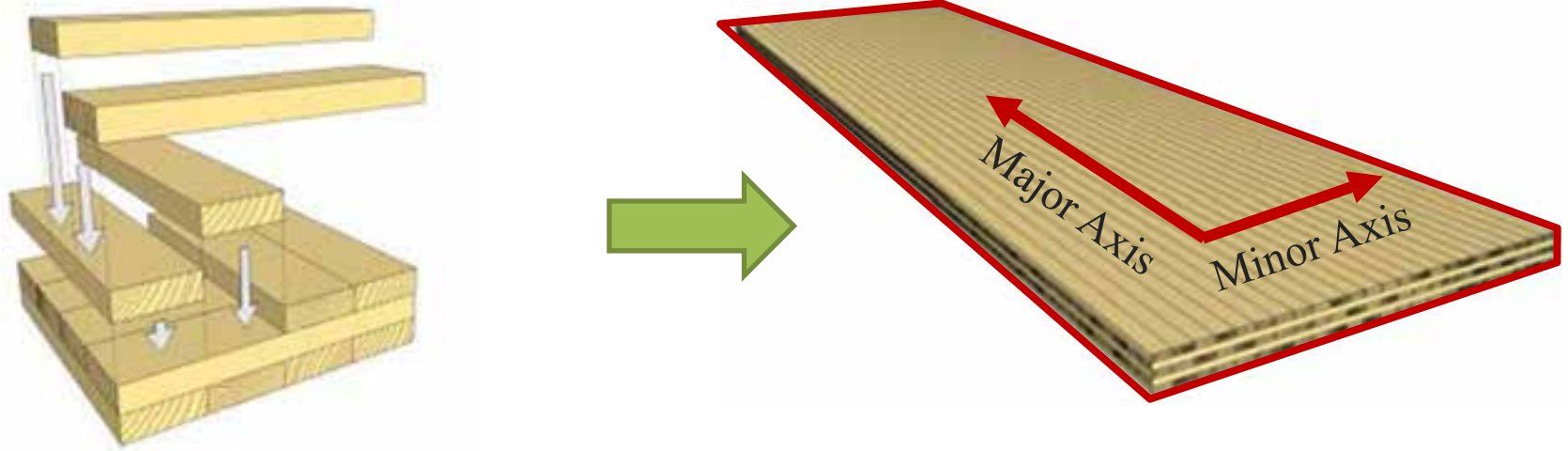
Odd number of symmetric layers most common

Double parallel exterior layers permitted

Unbalanced layup permitted

Reference glu-lam adhesive standard (AITC 405)

Structural Composition of CLT



Panels, also known as Billets.

20 inch max thickness in PRG 320

Up to 8 ft or more wide per manufacturer and shipping

Up to 40 ft or more long per manufacturer and shipping

Major axis: stronger, stiffer, usually long direction

Minor axis: less strong and stiff, usually short direction

MASS TIMBER PRODUCTS

CROSS-LAMINATED TIMBER (CLT)

COMMON CLT LAYUPS

3-PLY 3-LAYER



5-PLY 5-LAYER



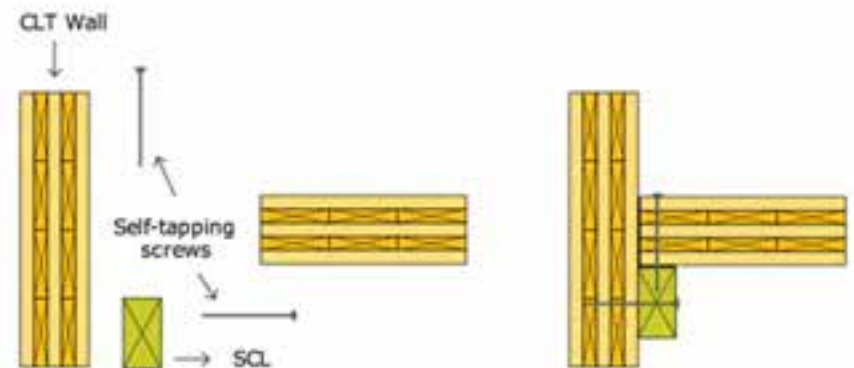
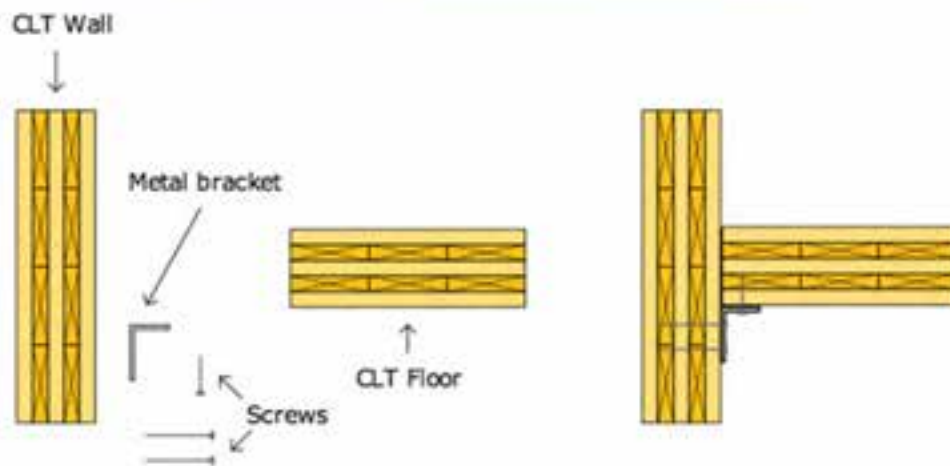
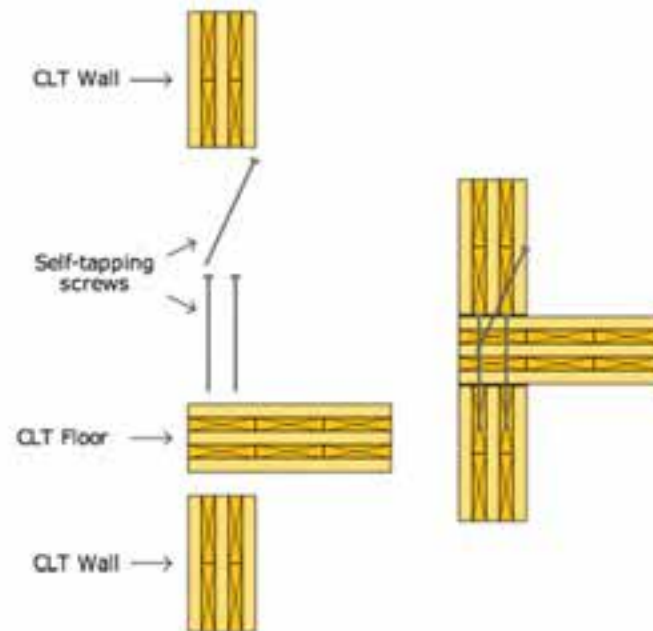
7-PLY 7-LAYER



9-PLY 9-LAYER



How to use CLT - Assembly



Source: US CLT Handbook

What is the appeal of CLT?

Sustainability

- Embodied Carbon

Performance

Construction
Efficiency



Reduced Embodied Carbon

Volume of wood used	950 m ³
Carbon sequestered and stored (CO ₂ e)	760 metric tons
Avoided greenhouse gases (CO ₂ e)	320 metric tons
Total potential carbon benefit (CO ₂ e)	1,080 metric tons

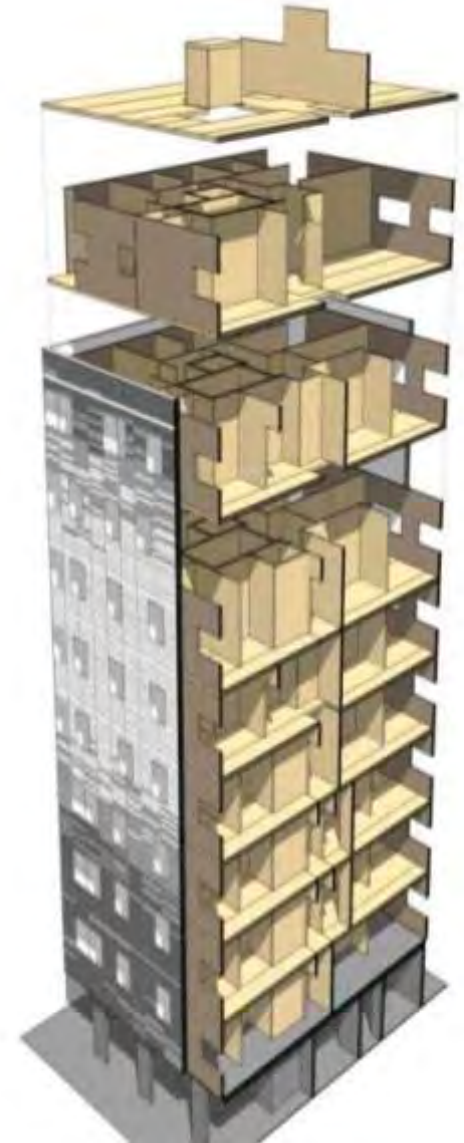
Carbon savings from the choice of wood in this one building are equivalent to:



1,615 passenger vehicles off the road for a year



Enough energy to operate a home for 803 years



Stadhaus, London, UK

Architect: Waugh Thistleton Architects

Photo credit: Waugh Thistleton Architects

BULLITT CENTER

SEATTLE, WA

PHOTO CREDIT: BULLITT CENTER





**BUILDING INFO:
OFFICE BUILDING**

4 STORIES MASS TIMBER OVER 2 STORIES CONCRETE

52,000 SF

NET ZERO

LIVING BUILDING CHALLENGE CERTIFIED

TYPE IV CONSTRUCTION

250 YR DESIGN LIFE

COMPLETED 2013

BULLITT CENTER

SEATTLE, WA



250 YEAR STRUCTURE

HEAVY TIMBER, CONCRETE & STEEL

PHOTO CREDIT: MILLER HULL PARTNERSHIP

BULLITT CENTER

SEATTLE, WA

**NAIL-LAMINATED TIMBER DECKS PROVIDE:
MAXIMIZED SPANS, REDUCED NUMBER OF COLUMNS, MORE OPEN SPACE
FLEXIBILITY, MINIMIZED STRUCTURE DEPTH**

PHOTO CREDIT: JOHN STAMETS

What is the appeal of CLT?

Sustainability

- Reduced Embodied Carbon
- Minimal waste production

Performance

Construction Efficiency



Minimal Waste



Why are designers drawn to CLT?

Sustainability

- Reduced Embodied Carbon
- Minimal waste production
- Highly Energy Efficient

Performance

Construction Efficiency



Energy Efficient

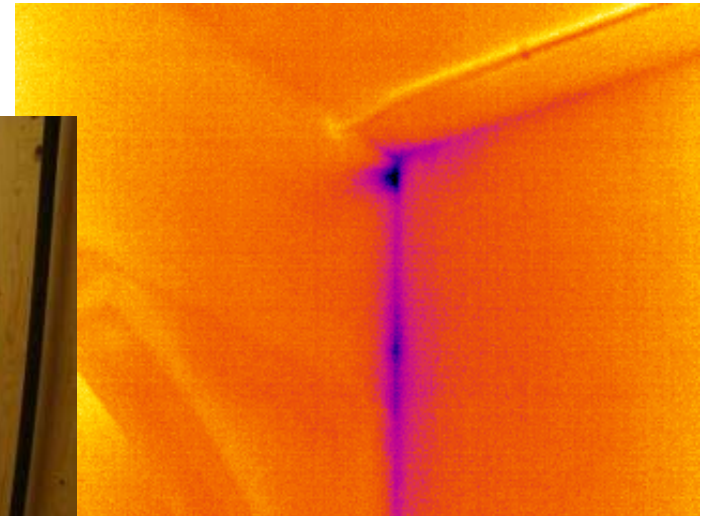


Table 2

Thermal resistance of typical softwood at various thicknesses and 12% moisture content

Thickness	1 in. (25 mm)	4 in. (100 mm)	6 in. (150 mm)	8 in. (200 mm)
R-value ($\text{h}\cdot\text{ft}^2\cdot^\circ\text{F}\cdot\text{Btu}^{-1}$)	1.25	5.00	7.50	10.00
RSI ($\text{m}^2\cdot\text{K}\cdot\text{W}^{-1}$)	0.22	0.88	1.30	1.80

CLT has an R-value of approximately 1.25 per inch of thickness.

Source: US CLT Handbook

What is the appeal of CLT?

Sustainability

- Reduced Embodied Carbon
- Minimal waste production
- Highly Energy Efficient

Performance

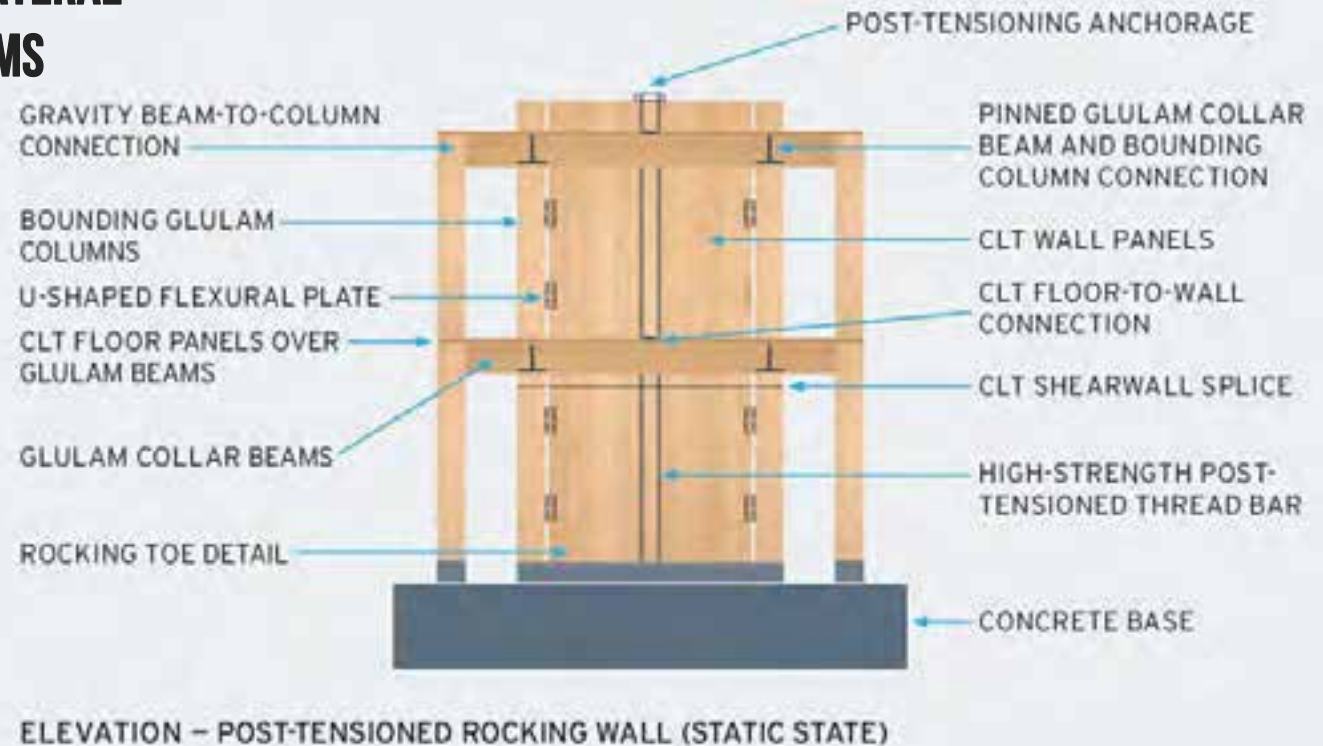
- Disaster Resilient

Construction Efficiency

MASS TIMBER APPEAL

DISASTER RESILIENT

INNOVATIVE MASS TIMBER LATERAL FORCE RESISTING SYSTEMS



MASS TIMBER SHAKE TABLE TEST AT UCSD

CLT ROCKING SHEAR WALL CONCEPT

SOURCE: KPFF

MASS TIMBER APPEAL

DISASTER RESILIENT



CANDLEWOOD SUITES

REDSTONE ARSENAL, AL



IMAGE CREDIT: IHG® Army Hotels,
Lendlease

CANDLEWOOD SUITES

REDSTONE ARSENAL, AL



IMAGE CREDIT: LEND LEASE



- **62,600 SF, 4 STORY HOTEL, 92 PRIVATE ROOMS**
- **CLT UTILIZED FOR WALLS, ROOF PANELS, AND FLOOR PANELS**
- **1,557 CLT PANELS; TYPICAL FLOOR PANEL IS 8'X50' & WEIGHS 8,000 LBS**
- **COMPLETED LATE 2015**

IMAGE CREDIT: LEND LEASE & SCHAEFER

CANDLEWOOD SUITES

REDSTONE ARSENAL, AL



What is the appeal of CLT?

Sustainability

- Reduced Embodied Carbon
- Minimal waste production
- Energy Efficient

Performance

- Disaster Resilient
- Fire Resistant

Construction Efficiency



Fire Test Results

- ASTM E119 Fire Endurance Test
 - 5-Ply CLT (6-7/8" thick)
 - 5/8" Type X GWB each side
 - 2 hour target
 - Actual 3 hours 6 minutes
- 2015 NDS Chapter 16 includes char rates for CLT to achieve up to 2 hour fire rating



CLT is Defined – 2015 IBC

SECTION 202 DEFINITIONS

CROSS-LAMINATED TIMBER. A prefabricated engineered wood product consisting of at least three layers of solid-sawn lumber or *structural composite lumber* where the adjacent layers are cross-oriented and bonded with structural adhesive to form a solid wood element.

Add new text as follows:

2303.1.4 Structural glued cross-laminated timber. Cross-laminated timbers shall be manufactured and identified as required in ANSI/APA PRG 320-2011.

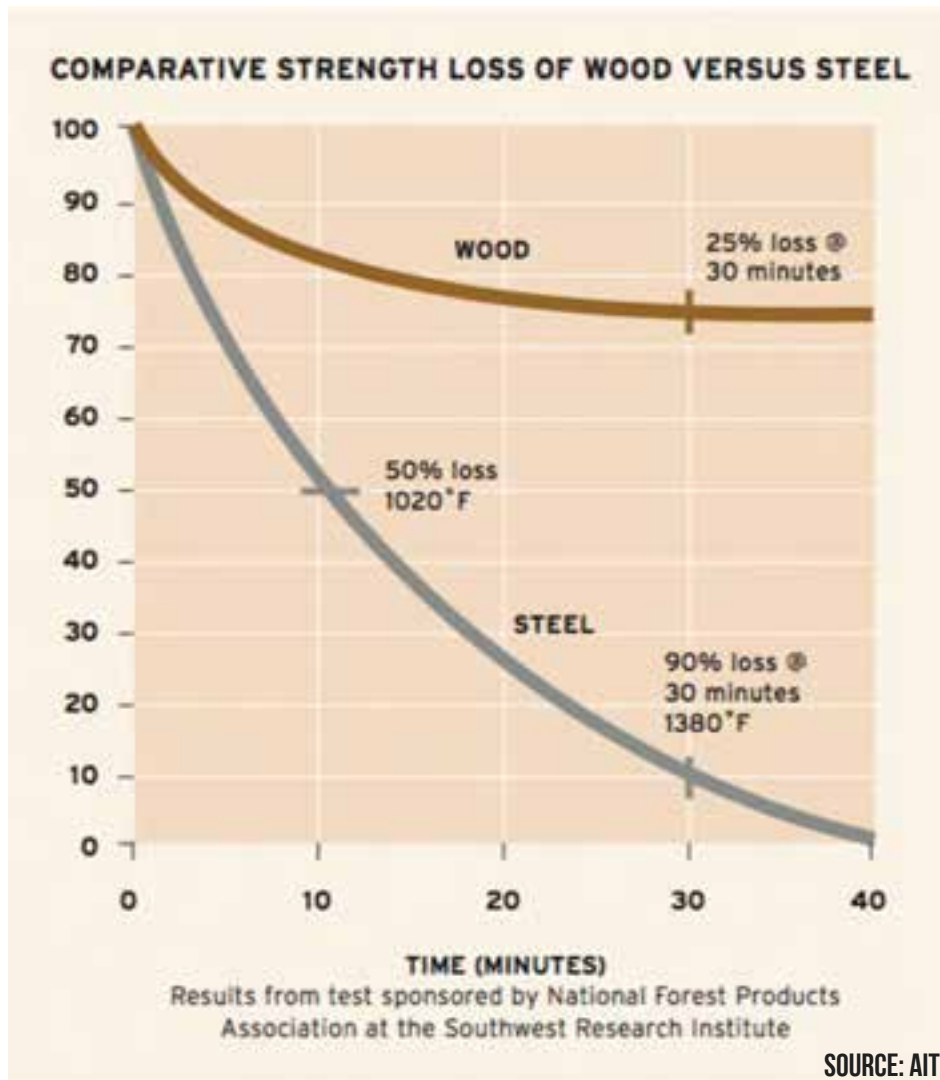
Add new standard to Chapter 35 as follows:

ANSI

ANSI/APA PRG 320-2011 Standard for Performance-Rated Cross-Laminated Timber

MASS TIMBER DESIGN

FIRE RESISTANCE



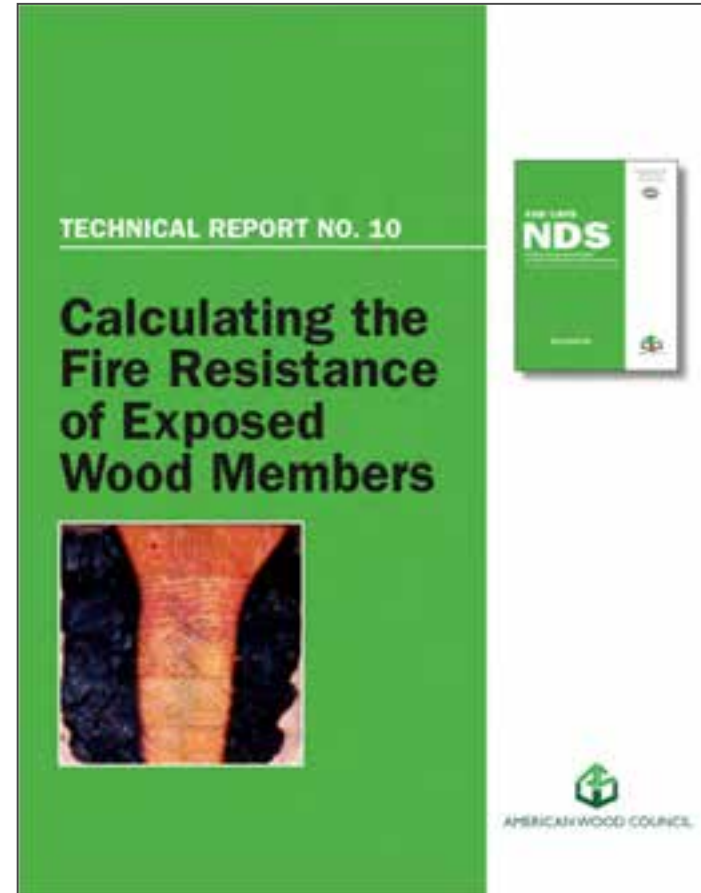
Achieving One Hour Equivalency for Protected Construction

NDS Chapter 16

Fire Design of Wood Members

NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION	
FIRE DESIGN OF WOOD MEMBERS	
16.1 General	150
16.2 Design Procedures for Exposed Wood Members	150
16.3 Wood Connections	151
Table 16.2.1 Effective Char Rates and Char Layer Thicknesses (for $\rho_c = 1.5 \text{ g/cc}$)	150
Table 16.2.2 Adjustment Factors for Fire Design	151

OR →



TR 10

Available from AWC website

CONSTRUCTION TYPES

IBC 602

ALL WOOD FRAMED BUILDING OPTIONS:

TYPE III

EXTERIOR WALLS NON-COMBUSTIBLE (MAY BE FRTW)

INTERIOR ELEMENTS ANY ALLOWED BY CODE, INCLUDING MASS TIMBER

TYPE V

ALL BUILDING ELEMENTS ARE ANY ALLOWED BY CODE, INCLUDING MASS TIMBER

TYPES III AND V ARE SUBDIVIDED TO A (PROTECTED) AND B (UNPROTECTED)

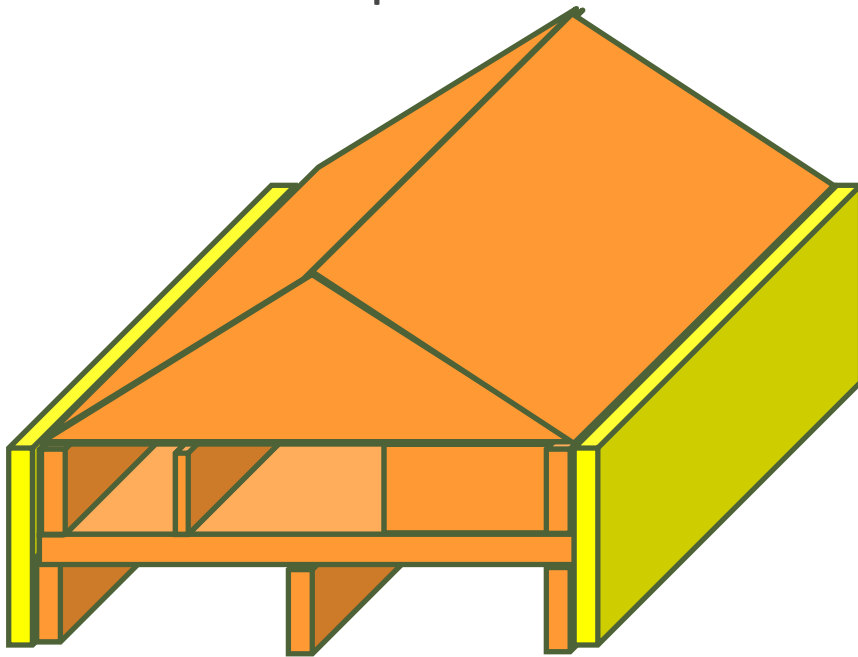
TYPE IV (HEAVY TIMBER)

EXTERIOR WALLS NON-COMBUSTIBLE (MAY BE FRTW OR CLT)

INTERIOR ELEMENTS QUALIFY AS HEAVY TIMBER (MIN. SIZES, NO CONCEALED SPACES)

Type IV Construction – IBC 602.4

Exterior walls are of noncombustible materials and interior building elements are of solid or laminated wood without concealed spaces. FRT wood or Cross Laminated Timber*- 2015IBC is permitted in exterior walls, where 2hr fire rating or less is required



*Exterior surface of CLT is protected by FRT sheathing, ½" gypsum, or other non-combustible materials

- Non combustible Exterior walls
- Interior walls-solid without concealed spaces

- Fire Retardant Treated exterior walls or Cross laminated Timber (CLT)-2015 IBC are allowed if fire rating is 2hr or less

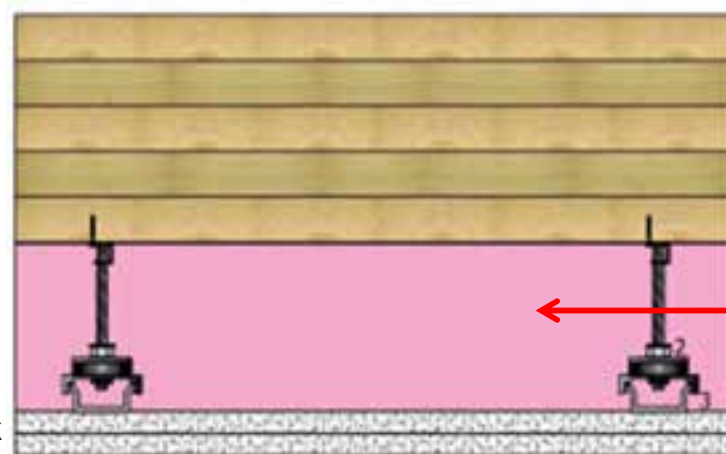
- Heavy Timber

Concealed Space Limitations on HT

Type IV Construction requires that interior elements be without concealed spaces:

- Concealed spaces include dropped ceilings, attics, chases, others

Concealed space requirement does not apply to any other construction type. If using heavy timber elements in non type IV construction, concealed spaces are permitted but may be required to be sprinklered



Example of concealed space created by dropped ceiling

What is the appeal of CLT?

Sustainability

- Reduced Embodied Carbon
- Minimal waste production
- Highly Energy Efficient

Performance

- Disaster Resilient
- Good Fire Resistance
- High performing Acoustics

Construction Efficiency

CLT Acoustics

Sound Insulation of Bare CLT Floors and Walls

Number of layers	Thickness (in.)	Assembly type	STC	IIC
3	3-3/4 to 4-1/2	Wall	32-34	N.A.
5	5-1/3	Floor	39	23
5	5-3/4	Floor	39	24
Measured on field bare CLT wall and floor				
Number of layers	Thickness in.	Assembly type	FSTC	FIIC
3	4-1/8	Wall	28	N.A.
7	8-1/5	Floor	N.A	25-30

Source: US CLT Handbook

MASS TIMBER DESIGN

ACOUSTICS

Common mass timber floor assembly:

- Finish floor (if applicable)
- Underlayment (if finish floor)
- 1.5'' to 3'' thick concrete/gypcrete topping
- Acoustical mat
- WSP (if applicable)
- Mass timber floor panels



Image credit: AcoustiTECH

Why are designers drawn to CLT?

Sustainability

- Reduced Embodied Carbon
- Minimal waste production
- Highly Energy Efficient

Performance

- Disaster Resilient
- Good Fire Resistance
- High performing Acoustics
- Structural Flexibility

Construction Efficiency

Structural Flexibility



Photo Credit: APA

What is the appeal of CLT?

Sustainability

- Reduced Embodied Carbon
- Minimal waste production
- Highly Energy Efficient

Performance

- Disaster Resilient
- Good Fire Resistance
- High performing Acoustics
- Structural Flexibility

Construction Efficiency

- ~75% lighter than concrete



75% Lighter Weight Than Concrete



What is the appeal of CLT?

Sustainability

- Reduced Embodied Carbon
- Minimal waste production
- Highly Energy Efficient

Performance

- Disaster Resilient
- Good Fire Resistance
- High performing Acoustics
- Structural Flexibility

Construction Efficiency

- ~75% lighter than concrete
- Reduced construction time



Reduced Construction Time



Murray Grove, London UK

- 8 stories of CLT over 1 story concrete podium
- 8 stories built in 27 days (~1/2 the time of precast concrete)



Franklin Elementary School, Franklin, WV

- 45,200 ft² 2 story elementary school
- 8 weeks to construct

FRANKLIN ELEMENTARY SCHOOL

FRANKLIN, WV



PHOTO CREDIT: PAM WEAN, MSES ARCHITECTS



45,200 sf, 2 story school

CLT utilized for walls, roof panels, and floor panels

CLT chosen for its construction schedule benefits

Completed January 2015

Photo Credit: Pam Wean, MSES Architects

What is the appeal of CLT?

Sustainability

- Reduced Embodied Carbon
- Minimal waste production
- Highly Energy Efficient

Performance

- Disaster Resilient
- Good Fire Resistance
- High performing Acoustics
- Structural Flexibility

Construction Efficiency

- ~75% lighter than concrete
- Reduced construction time
- Pre-fabricated and Precise

CLT: A Prefabricated Material

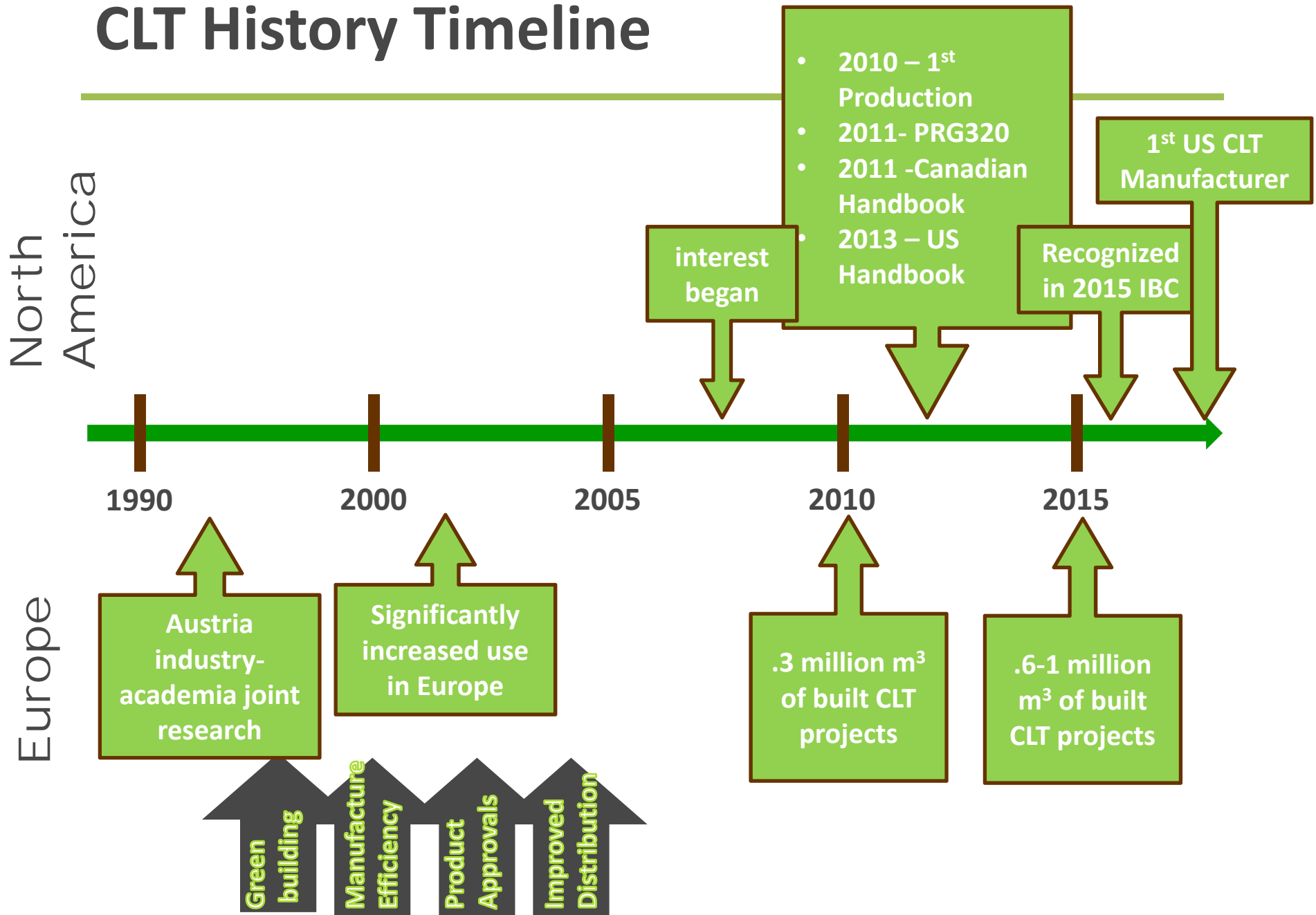


- Custom engineered for material efficiency.
- Custom designed for project.
- Each panel numbered, delivered & installed in predetermined sequence

- Finished panels are planed, sanded, cut to size. Then openings are cut with precise CNC routers.
- Third party inspection at factory.



CLT History Timeline





Product Availability

Outline

- What is CLT?
 - Mass Timber
 - The Appeal
 - History
 - Availability
- Using CLT
 - Project Examples
 - Best applications
 - Cost effective design
 - Building Codes and Standards

Mass Timber Building Options





Promega Feynman Center, The Crossroads, Madison, Wisconsin
Photos: Aitor Sanchez/EwingCole

New Stanford Heat Recovery Center Stanford, CA



Image Credit: ZGF Architects

Stanford Heat Recovery Center
Photo: Structurlam

CLT Roof Panels used over
entrance/walkway

Part of 125,000 sf building
that is expected to reduce
campus carbon emissions
by 50% and save an
estimated \$300M over the
next 35 years



Chicago Horizon Pavilion

Chicago, IL



Photo Credit: Tom Harris

56' square kiosk

2 Layers of 3-ply, 4-1/8" CLT roof panels in opposite directions, each panel 8' x 56', creating 2 way spanning plate



Chicago Horizon Pavilion
Photos: Tom Harris



Chicago Horizon Pavilion
Photos: Aaron Forrest

Total roof structure
thickness 8-1/4"

Spans up to 30 feet between
columns at points





4 STORIES
16,000 SF
GREEN ROOF

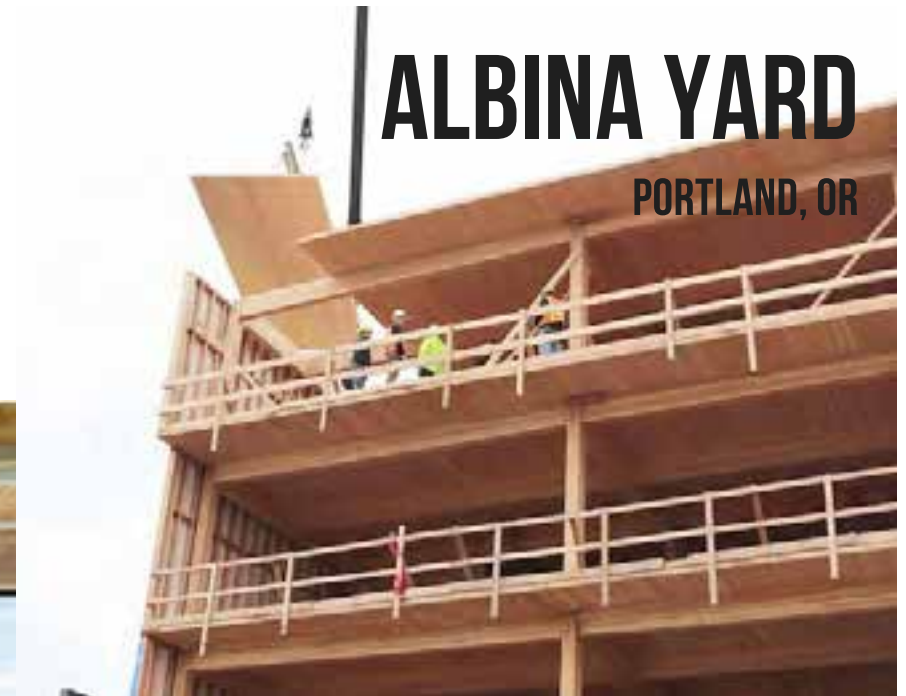
ALBINA YARD

PORTLAND, OR



ARCHITECT: LEVER ARCHITECTURE
IMAGE CREDIT: LEVER ARCHITECTURE

- **20'X20' GRID, 12' FLOOR TO FLOOR**
- **3-PLY CLT FLOOR PANELS WITH ELECTRICAL CONDUIT
POURED INTO 1" LIGHT WEIGH GYPSUM TOPPING**
- **WOOD SHEARWALL CORE WITH OPEN FRONT DESIGN
FOR GLAZING WALL**



**CLT PANELS FOR AN ENTIRE FLOOR
INSTALLED IN LESS THAN 4 HOURS**

SOURCE: LEVER ARCHITECTURE¹

FIRST TECH CREDIT UNION

HILLSBORO, OR



5 STORIES
156,000 SF



ARCHITECT: HACKER
IMAGE CREDIT: STRUCTURLAM

FIRST TECH CREDIT UNION

HILLSBORO, OR

ARCHITECT: HACKER
IMAGE CREDIT: STRUCTURLAM

COMPLETED 2017 — 156,000 SF
626 PANELS & 988 GLULAMS



UMASS DESIGN BUILDING

AMHERST, MA

IMAGE CREDIT: ALEX SCHREYER



UMASS DESIGN BUILDING

AMHERST, MA

4 STORY, 87,500 SF FACILITY WITH: CLASSROOMS, LOUNGES, MEETING ROOMS, MATERIALS-TESTING LAB, GREEN-BUILDING LAB, WOOD SHOP, DIGITAL FABRICATION LAB, CAFE, EXHIBIT SPACE, AND LIBRARY

IMAGE: ALEX SCHREYER

UMASS DESIGN BUILDING

AMHERST, MA

COMPLETED JAN 2017

PHOTO CREDIT: ALEX SCHREYER





PHOTO CREDIT: ALEX SCHREYER



UMASS DESIGN BUILDING

AMHERST, MA

U OF ARKANSAS STUDENT DORMS

FAYETVILLE, AR

IMAGE CREDIT: MODUS STUDIO





**(2) - 5 STORY BUILDINGS
TOTAL OVER 200,000 SF
368 RESIDENTIAL ROOMS**



IMAGE CREDIT: MODUS STUDIO/LEERS WEINZAPFEL ARCHITECTS

Wood Innovation Design Center

Prince George, British Columbia

8 Levels/6 Stories

97 feet tall

Completed Fall 2014

Architect: Michael Green Architecture
Structural Engineer: Equilibrium Consulting
Contractor: PCL Constructors Westcoast
Photos: Ema Peter Photography





WOOD INNOVATION DESIGN CENTER

PHOTO CREDIT: ED WHITE

PRINCE GEORGE, BC



PHOTO CREDIT: EMA PETER

WIDC MEP ACCOMMODATION



- Completed in 2012
- 10 stories
- ~ 105 ft. tall, > 18.6 K sqft.
- 3 million in R&D
- Poor soils required a much lighter building

Forte', Victoria Harbor, Melbourne, Australia

Architect: Lend Lease

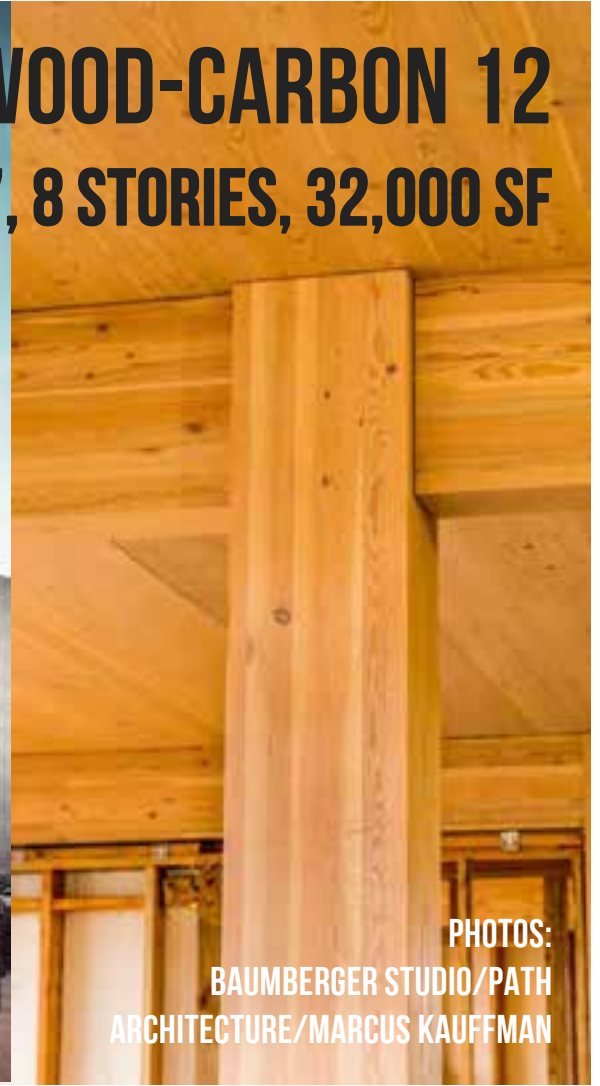


Forte', Victoria Harbor, Melbourne, Australia
Architect: Lend Lease



MODERN TALL WOOD-CARBON 12

2017, 8 STORIES, 32,000 SF



PORTLAND, OR

PHOTOS:
BAUMBERGER STUDIO/PATH
ARCHITECTURE/MARCUS KAUFFMAN



BROCK COMMONS

VANCOUVER, BC

18 STORIES

174 FT

156K SQ.FT.

© Acton Ostry Architects & University of British Columbia via CTBUH



**5 PLY CLT PANELS, 2-WAY SPAN
~9'X13' GRID OF COLUMNS**



BROCK COMMONS

VANCOUVER, BC

IMAGES: ACTON OSTRY ARCHITECTS



17 STORIES OF TIMBER INSTALLATION
STARTED JUNE 6, 2016
FINISHED AUGUST 10, 2016

BROCK COMMONS

VANCOUVER, BC



BROCK COMMONS

VANCOUVER, BC



CLT as an alternate to Concrete/Masonry





U.S. BUILDING CODE STATUS



TALL WOOD IN THE U.S.

©2011 NATTAROL PORNALNUWAY
WWW.FIVECLOCKSTUDIO.COM

U.S. BUILDING CODES

Tall Wood Ad Hoc Committee

Balanced Committee: 2016-2018

Development of code change proposals for prescriptive code allowances of tall wood buildings.



Mass Timber Fire Testing at ATF Lab



Mass Timber Shake Table Test at UCSD

New Building Types



16 STORIES
BUILDING HEIGHT 170' 0"
ALLOWABLE BUILDING AREA 972,000 SF
AVERAGE AREA PER STORY 60,750 SF

TYPE IV-A



13 STORIES
BUILDING HEIGHT 136' 0"
ALLOWABLE BUILDING AREA 648,000 SF
AVERAGE AREA PER STORY 49,846 SF

TYPE IV-B



8 STORIES
BUILDING HEIGHT 84' 0"
ALLOWABLE BUILDING AREA 405,000 SF
AVERAGE AREA PER STORY 50,625 SF

TYPE IV-C

IBC 2021



324,000 SF
ALLOWABLE BUILDING AREA
54,000 SF
AVERAGE AREA PER STORY

8 STORIES MAXIMUM
30' 0" MAXIMUM BUILDING HEIGHT
324,000 SF MAXIMUM AREA

TYPE IV- HT

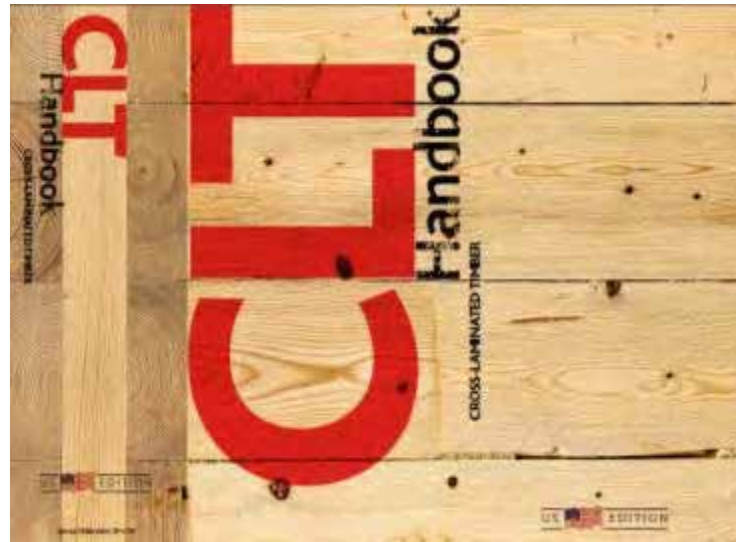
IBC 2015

BUSINESS OCCUPANCY [GROUP B]

*BUILDING FLOOR-TO-FLOOR HEIGHTS ARE SHOWN AT 12'-0" FOR ALL EXAMPLES FOR CLARITY IN COMPARISON BETWEEN 2015 TO 2021 IBC CODES

US CLT Handbook

- | | |
|------------------|-------------------|
| 1. Introduction | 10. Enclosure |
| 2. Manufacturing | 11. Environmental |
| 3. Structural | 12. Lifting |
| 4. Lateral | |
| 5. Connections | |
| 6. DOL and Creep | |
| 7. Vibration | |
| 8. Fire | |
| 9. Sound | |



Mass Timber Projects In Design and Constructed in the US (December 2018)

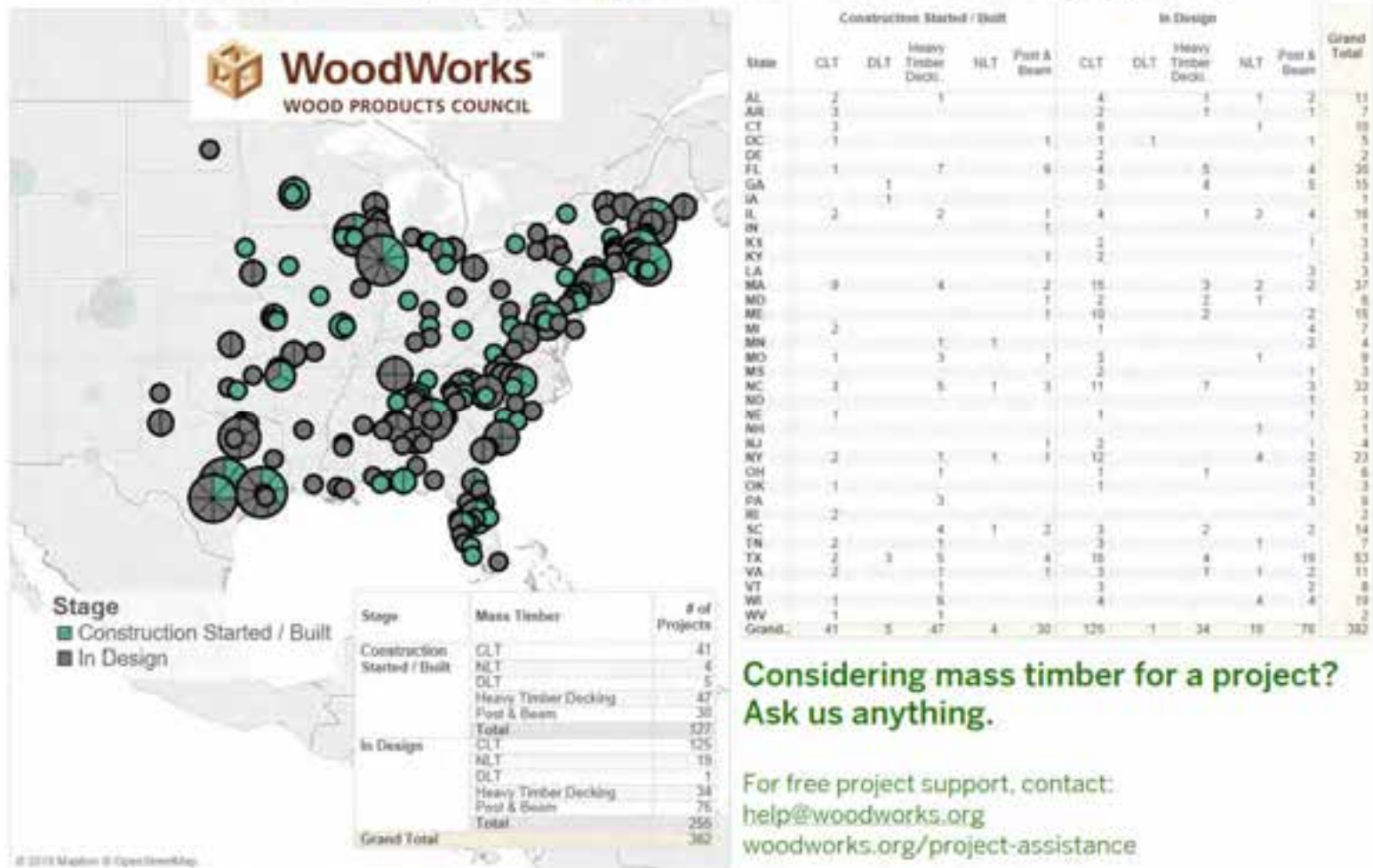
**AS OF DECEMBER 2018, 487 MASS TIMBER PROJECTS
DESIGNED, UNDER CONSTRUCTION OR BUILT**



**Considering mass timber for a project?
Ask us anything.**

For free project support, contact:
help@woodworks.org
woodworks.org/project-assistance

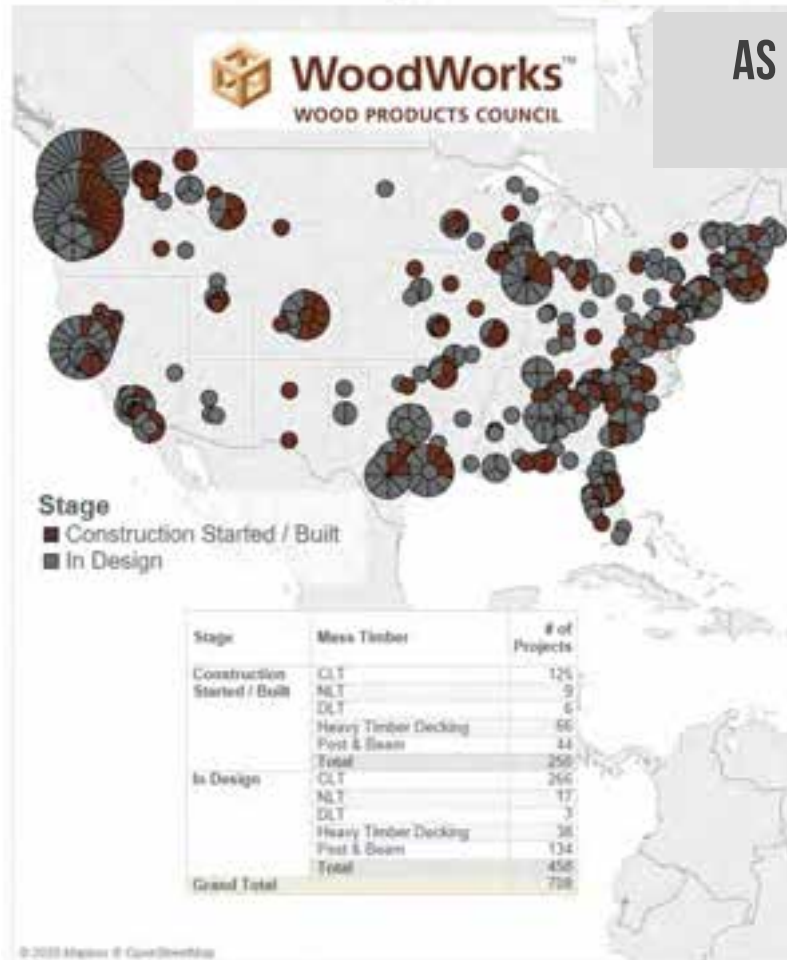
Mass Timber Projects In Design and Constructed in the US (July 2019)



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Mass Timber Projects In Design and Constructed in the US (December 2019)



AS OF DECEMBER 2019, 708 MASS TIMBER PROJECTS DESIGNED, UNDER CONSTRUCTION OR BUILT

CA	In Design	11	UT	In Design	1
CT	Construction Started / Built	3	NJ	Construction Started / Built	1
DC	In Design	8	NY	In Design	6
DE	Construction Started / Built	1	NM	Construction Started / Built	1
FL	In Design	5	NV	In Design	2
GA	Construction Started / Built	10	NY	Construction Started / Built	8
HI	In Design	18	OH	In Design	24
IA	Construction Started / Built	4	OK	Construction Started / Built	1
ID	In Design	13	OK	In Design	1
IL	Construction Started / Built	3	OR	Construction Started / Built	2
IN	In Design	1	OR	In Design	25
KS	Construction Started / Built	1	PA	In Design	23
KY	In Design	3	PA	Construction Started / Built	3
LA	Construction Started / Built	5	RI	In Design	5
MA	In Design	11	RI	Construction Started / Built	2
MD	Construction Started / Built	1	SC	In Design	1
ME	In Design	1	SC	Construction Started / Built	6
MH	Construction Started / Built	2	TN	In Design	11
MN	In Design	1	TN	Construction Started / Built	3
MO	Construction Started / Built	2	TX	In Design	4
MT	In Design	1	TX	Construction Started / Built	17
NE	Construction Started / Built	5	UT	In Design	37
NH	Construction Started / Built	13	VA	Construction Started / Built	3
NI	In Design	25	VA	In Design	6
ND	Construction Started / Built	1	VT	Construction Started / Built	7
NE	In Design	14	VT	In Design	8
NM	Construction Started / Built	1	WA	Construction Started / Built	8
NV	Construction Started / Built	3	WA	In Design	26
NY	In Design	6	WV	Construction Started / Built	44
NC	Construction Started / Built	2	WV	In Design	8
ND	In Design	4	WY	Construction Started / Built	13
NE	Construction Started / Built	3	WY	In Design	1
NH	In Design	6			

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> Questions?

This concludes The
American Institute of
Architects Continuing
Education Systems
Course

Anthony Harvey PE

Regional Director

anthony.harvey@woodworks.org